

# Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1



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Prepared for:  
2609216 Ontario Inc.

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CAMBIUM INC.

866.217.7900

[cambium-inc.com](http://cambium-inc.com)



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## 1.0 Introduction

Cambium Inc. (Cambium) was retained by 2609216 Ontario Inc (the Client) to complete a hydrogeological investigation in support of the development of a proposed residential subdivision at 152 & 156 Miller Drive, City of Barrie, Ontario (herein referred to as Site).

The property is currently a predominantly vacant, “T” shaped Site, with sloping topography, generally falling towards the northwest and southeast property extents, and is approximately 1.44 hectares (ha) or 3.56 acres in size. At the time of the investigation, the eastern portion of the property contained two residential homes, with the remainder of the Site being undeveloped and consisting of a mixture of open field and medium mixed forest. Wet areas without any standing water were noted at the northwest and southeast property extents with deposits of organic soils.

Cambium understands that the proposed residential development consists of four detached single-family dwellings, and ten blocks of attached townhomes consisting of fifty-four total units. It is assumed that each dwelling will have an individual basement level. The development also includes internal roadways and parking areas.

Hydrogeological investigation is required as part of the application process of Draft Subdivision Plan and Site Plan Approval for the proposed residential development.

### 1.1 Scope of Work

This hydrogeological investigation was conducted with the following tasks:

- **Review of available background information:** a review of available geological and hydrogeological information for the Site and surrounding areas and the previous investigation reports completed for the Site, was conducted to provide background information to allow for characterization of the Site’s soil and groundwater conditions.
- **Detailed site inspection:** an inspection of the Site was completed to review existing site conditions including identification of any hydrogeological features such as significant areas of potential groundwater recharge or areas of groundwater discharge.



- **Measurement of groundwater levels:** groundwater levels were measured in the existing monitoring wells to establish and/or confirm the general groundwater flow condition and to assess the groundwater elevations.
- **Soil Infiltration Assessment:** Results of surficial soil testing completed during the geotechnical investigation across the Site were utilized to characterize the infiltration rate of the shallow surficial soils (i.e., within approximately 0.5 mbeg). The data will support the design and placement of Low Impact Development (LID) measures, as needed at the Site.
- **Single Well Response Test (SWRT):** two existing monitoring wells were hydraulically tested to calculate the hydraulic conductivity of the native soils to estimate potential dewatering rates.
- **Water Balance (Preliminary):** a preliminary water balance study was completed for the proposed development using the Thornthwaite-Mather approach and the climatic data obtained from Environment Canada.
- **Source Water Protection:** an assessment of source water protection issues identified at the Site was reviewed and appropriate recommendations provided.

Source Protection Policies (SPP) including RLU-1 (Notice 59 to proceed from Risk Management Official), DNAPL-1 (policy related to drinking water quality threat) and Salt (ICA)-2 (related to handling and storage of road salt) will apply to the Site. As such, SPP policies are addressed in the report (Section 8.0).

It should be noted that a geotechnical investigation was completed at the Site by Cambium in 2019 (Cambium, 2019). The data or information obtained in this former investigation has been incorporated into this hydrogeological investigation report.

The proposed development lots will have water and wastewater services provided by the City of Barrie.

The proposed development constitutes a major development as defined by the Lake Simcoe Protection Plan (LSPP) and is subjected to provide a detailed water balance and hydrogeological report. The proposed development plan is included in Appendix A.



## 1.2 Site Description

The Site is bound to the east by residential subdivision and by Miller Drive and surrounded by open lands with treed areas to the north, west and south.

The regional location of the Site is outlined on Figure 1, the property and surrounding areas outlined on Figure 2.



## 2.0 Environmental Features

To assess environmental features, various databases maintained by the Ministry of Natural Resources and Forestry (MNRF), the Ministry of Environment, Conservation and Parks (MECP), and the Nottawasaga Valley Conservation Authority (NVCA) were reviewed.

The Site is situated within the City of Barrie municipal limits and falls under the authority of the NVCA. The Site falls within the Bear Creek – Nottawasaga River subwatershed within the Nottawasaga River watershed. As per NVCA mapping, the majority of the Site is located within a regulated area (O.Reg 172/06; Figure 3); as such, regulation area development restrictions apply to the proposed development..

As per MNRF Natural Heritage System database, the Site is not located in a significant natural area or area of natural and scientific interests (ANSIs). From the provincial mapping (Appendix A), an unevaluated wetland is noted within the northwestern corner of the Site. This unevaluated wetland extends off-site, immediately adjacent to the western boundary of the Site and extends down south of the Site. A provincially significant wetland (PSW) is located approximately 200 m west of the Site, adjacent to the unevaluated wetland (MNRF, 2024). Cambium personnel noted that low lying areas in the northwest and southern portions of the property had no standing water during the site visits.

As shown on the MECP Source Water Protection Atlas map, the Site is in the following areas:

- Wellhead Protection Area C (WHPA-C);
- Wellhead Protection Area Q1 and Q2 (WHPA-Q1/Q2) with low stress level;
- A Highly Vulnerable Aquifer (HVA) area with a vulnerability score of 6;
- Significant Groundwater Recharge Area (SGRA) with a score of 6; and
- Issue Contributing Area (ICA) with chloride as a contaminant.



## 3.0 Physical Setting

### 3.1 Topography and Drainage

According to the regional topographic map of the area (Appendix A), the Site slopes generally to the south-southwest, with the elevations ranging from approximately 240 meters above sea level (masl) in the northeast to about 238 masl in the south.

A topographic survey was completed by Dino Astri Surveying Ltd. in 2019 for the Site (see Appendix A). The highest elevation was measured in the eastern portion at approximately 243 masl, sloping downwards to the southwest to an elevation of approximately 236 masl.

Therefore, the total relief of the area is about 7 m.

The Site is situated within the Bear Creek – Nottawasaga River subwatershed. Surface drainage at the Site is expected to follow local topography and discharge off-site to the south, into the provincially mapped unevaluated wetland.

### 3.2 Physiography

According to Chapman and Putnam (1984), the Site is located at the boundary of the Simcoe Uplands and Simcoe Lowlands physiographic regions. The Simcoe Uplands generally consist of the topographic highs, rising up to 100 m above the adjacent Simcoe Lowlands. The Simcoe Lowlands were flooded by glacial Lake Algonquin, and as a result, are floored by sand, silt, and clay.

### 3.3 Overburden Geology

The surficial geology at the Site was characterized as ice-contact stratified deposits consisting mainly of sand and fine silt with varying amounts of gravel, with high infiltration capacity (Data Set 14 - Revised from the Ontario Geological Survey (OGS, 2000)).

### 3.4 Bedrock Geology

According to Miscellaneous Release – Data 219 from the Ontario Geological Survey (OGS, 2007), the bedrock can be characterized as being from the Paleozoic Era, belonging to the



Middle Ordovician Simcoe Group, consisting primarily of limestone, dolostone, shale, arkose, and sandstone. The Simcoe Group consists of four formations that dip gently towards the southwest; Gull River Formation, Bobcaygeon Formation, Verulam Formation, and the Lindsay Formation from oldest to youngest. Verulam Formation outcrops along the shoreline of Kempenfelt Bay and extends west of the City of Barrie and ranges in thickness from 32 to 65 m and consists of fossiliferous limestone with inter-beds of calcareous shale.



## 4.0 MECP Well Records

Water well records on file with the Ministry of the Environment, Conservation and Parks (MECP) served as a data source for this hydrogeological assessment. The well locations were provided from the MECP interactive water well record database. According to the well records, there appears to be 17 wells within a 500 m radius around the property and were installed between the years 1952 and 2010. The locations of the recorded water wells are shown on Figure 4 and the well records are included in Appendix B.

A thick layer of overburden materials is reportedly present in the study area based on the details in the well records. Overburden consists mainly of sand and clay with fine silt and gravel.

Well depths ranged from as shallow as 7.3 metres below ground surface (mbgs) to as deep as 110.3 mbgs. Shale bedrock was encountered at a depth of 108.6 mbgs in well No. 5714853, with the well installed to a total depth of 110.3 mbgs.

Water well record no. 5714853 was installed for water supply but abandoned due to insufficient supply and all the other wells were recorded as test holes, observation wells, abandoned wells or decommissioned wells.



## 5.0 Borehole and Monitoring Well Installation

During the geotechnical investigation completed at the Site between May 21 and 22, 2019 (Cambium, 2019), a total of 14 boreholes were completed. The boreholes were designated as BH101-19 through BH114-19, and each borehole was terminated at a depth of 5.0 mbgs. Caving (sloughing) was encountered in 12 of the boreholes.

The borehole locations and elevations were surveyed using a Sokkia RTK GPS system. The borehole elevations were tied to geodetic elevations referencing a known benchmark.

Two boreholes identified as BH110-19 and BH114-19 were completed as monitoring wells.

Figure 5 depicts the locations of boreholes and monitoring wells completed by Cambium. The observed soil stratigraphy and the details of monitoring wells installed by Cambium are presented in the borehole logs included in Appendix C. The subsurface conditions at the Site consist of a surficial layer of topsoil overlying brown sandy deposits with trace amounts of silt, clay, and gravel to the termination depths of 5.0 mbgs. The boreholes were terminated in native soils and bedrock was not encountered within the explored depths.

### 5.1 Physical Laboratory Testing

Physical laboratory testing, including three sieve and hydrometer analyses (LS-702, 705), was completed on selected soil samples to confirm the soil texture. The results are presented in Table 1 and are included in Appendix D.

**Table 1 Particle Size Distribution**

Sample	Depth (mbgs)	Soil	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Percolation Time (min/cm)
BH101-19	1.5 – 2.0	Sand trace Silt trace Clay	0	93	7		7
BH108-19	2.3 – 2.7	Sand trace Silt trace Gravel trace Clay	4	88	8		6
BH114-19	0.6 – 1.2	Sand some Silt trace Clay	0	83	17		8



As per the data above, the percolation times (T) ranged from 6 min/cm to 8 min/cm, with a geometric average of 7 min/cm, indicating soils with good drainage.

## 5.2 Groundwater Levels

Groundwater levels were measured in all the installed monitoring wells (BH110-19 and BH114-19) by Cambium on five occasions. The recorded groundwater levels are presented in Table 2 below.

**Table 2 Measured Groundwater Details**

Well		BH110-19	BH114-19
Top of Pipe Elevation (masl)		236.81	241.85
Ground Surface Elevation (masl)		235.73	240.94
Stick-up (m)		1.08	0.91
May 28, 2019	Water Level (mbgs)	0.10	4.49
	Groundwater Elev.(masl)	235.63	236.45
May 25, 2020	Water Level (mbgs)	0.13	4.49
	Groundwater Elev.(masl)	235.60	236.45
June 23, 2021	Water Level (mbgs)	0.86	4.49
	Groundwater Elev.(masl)	234.87	236.45
April 5, 2024	Water Level (mbgs)	0.10	4.33
	Groundwater Elev.(masl)	235.63	236.61
May 3, 2024	Water Level (mbgs)	0.11	4.29
	Groundwater Elev.(masl)	235.62	236.65

As presented above, the measured groundwater levels in the monitoring wells during the spring months (April to June), ranged in depth from as shallow as 0.10 mbgs in the southwestern part of the Site to as deep as 4.49 mbgs in the north, and the elevations ranged from 234.87 masl to 236.65 masl. The wide variation in depth to water levels can be attributed to the ground elevation differences across the Site.

It is noted that water levels did not fluctuate significantly at BH114-19 throughout any of the months, but that a larger water level fluctuation was noted for BH110-19 from April/May conditions to drier June conditions. As BH110-19 is at a lower elevation, fluctuations in



groundwater (with shallowest groundwater recorded during the spring months) is to be expected.

A long-term four-season water level monitoring is on-going and a letter report discussing the water level fluctuations and seasonal variations will be submitted after the conclusion of the program.

### **5.3 Inferred Groundwater Flow Direction**

As only two monitoring wells were installed at the Site, groundwater contours could not be accurately interpreted. Based on the water table elevation data obtained from the monitoring events and the location of the unevaluated wetland, groundwater flow within the shallow unconfined aquifer was interpreted to flow to the south, where it is interpreted to discharge to surface into the unevaluated wetland.

### **5.4 In-Situ Hydraulic Conductivity Tests**

Single well response tests (slug tests) were completed on the two on-site monitoring wells to determine the hydraulic conductivity (k-value) of the soils. The slug tests were conducted on June 23, 2021, as part of this hydrogeological study. A rising head test was performed in the existing monitoring well BH110-19; monitoring well BH114-19 did not have enough water to perform the test.

The hydraulic conductivity was estimated utilizing Aquifer Test Pro software utilizing the Hvorslev method. The field measurements of the slug test and the calculations are provided in Appendix E. The estimated hydraulic conductivity at BH110-19 was  $1.24 \times 10^{-6}$  m/sec, typical values for sand with trace silt.



## 6.0 Construction Dewatering Requirements

The proposed residential development consists of 4 detached single-family dwellings and ten blocks of attached townhomes with 54 total units. It is assumed that each dwelling will have an individual basement level. The development also includes municipal water and wastewater services; therefore, the installation of linear infrastructure also has the potential for short-term construction dewatering.

As per the grading plan provided (Appendix A) and correspondence with the Client, it is known that the depth of the basement is dependent on what model of dwelling is proposed for each block (i.e. split level, walkout, lookout, etc.); however, it is known that no full depth basements are proposed. It is also known that fill materials will be imported in select areas of the Site to raise the grade to ensure that basement level can be accommodated above the water table so no dewatering activities will be required.

Groundwater depths vary greatly across the Site due to varying surficial topography. Deeper groundwater conditions were encountered at BH114-19, with a spring maximum groundwater elevation of 236.65 masl. Based on the proposed grade elevations (Appendix A) of each dwelling in the northern, western, and central portions of the Site, short-term dewatering for the installation of the partial basements is not expected. It is known that spring groundwater elevations are close to ground surface in the southern corner of the Site, at a maximum of 235.63 masl. The dwellings in this area of the Site are walk-out basements, and the lowest elevation of each dwelling is provided on the grading plan. A review of the measured spring groundwater elevations and the walkout basement elevations, short-term dewatering is not anticipated for the installation of the dwellings in the southern corner of the Site.

Additionally, it is known that linear infrastructure is required to be installed at the Site to provide water and wastewater services. A detailed servicing plan was not provided at the time of this report's preparation; however, based on the surface grade of the internal roadways along the sanitary and sewer alignment and the presumed depth of linear infrastructure, short-term dewatering is not anticipated for the installation of linear infrastructure.



Cambium recommends planning the construction schedules during peak summer or early autumn seasons during the seasonal drought period so as to minimize any potential dewatering activity at the Site.



## 7.0 Water Balance Assessment

Following the Thornthwaite and Mather methodology (1957), a water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from ground or be transpired by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage ( $\Delta S$ ) in a steady-state system.

The annual water budget of a site can be expressed as:

$$P = ET + R + I + \Delta S$$

Where:

P = Precipitation (mm/year)

ET = Evapotranspiration (mm/year)

R = Run-off (mm/year)

I = Infiltration (mm/year)

$\Delta S$  = Change in groundwater storage (taken as zero) (mm/year)

The calculations presented here compare the pre- and post-development water balance changes within the Site boundaries as a result of the proposed development. It is noted that the water balance described herein does not account for catchment areas that extend off-site.

At the time of the investigation, the eastern portion of the property contained two residential homes, the remainder of the site was undeveloped and consisted of a mixture of open field and medium mixed forest. The proposed residential development consists of four detached single-family dwellings, and ten blocks of attached townhomes with fifty-four total units. The development also includes internal roadways and parking areas. The pre- and post-development plans are shown in Figure 6 and Figure 7, respectively.

Based on the available design information, the pre- and post-development Site coverage can be generally categorized into three types: paved areas, roof areas, and landscaped areas. A detailed landscape plan was not available at the time of this report. However, based on the



Site statistics provided by the client, a summary of the surface areas of the development is listed in Table 3:

**Table 3 Pre- and Post-Development Statistics**

Type of Land Coverage	Pre-Developments Areas (m <sup>2</sup> )	Post Development Areas (m <sup>2</sup> )
Paved Area	394	4,219
Roof Area	423	5,093
Landscaped Area	13,615	5,120
<b>Total (m<sup>2</sup>)</b>	<b>14,432</b>	<b>14,432</b>

## 7.1 Water Surplus

Water surplus is calculated by determining the difference between precipitation and evapotranspiration at a Site over the course of a year (changes in soil water storage were assumed to be negligible). The volume of water surplus is further sub-divided into portions that either infiltrate the on-site soils or directed off-site as runoff.

The climatic data, including monthly average temperature and precipitation from 1991 to 2020, were obtained from Environment Canada for Barrie WPCC (Climate ID: 6110557), located about 4 km distance from the Site. Accordingly, the average annual evapotranspiration was estimated to be about 535 mm/year using the USGS Thornthwaite Monthly Water Balance methodology (Appendix F), and the average annual precipitation was recorded to be 933 mm/year. Therefore, the water surplus of the Site was calculated to be 398 mm/yr.

Transpiration does not occur from structures, paved areas, or hard gravel surfaces. It was assumed that 10% of precipitation falling on these surfaces is lost directly to evaporation. The remaining depth (i.e., 90% of precipitation) was considered as surplus and converted to runoff.

## 7.2 Infiltration Rate

The volume of surplus water that infiltrates through pervious surfaces on-site was determined by applying an infiltration factor to the surplus depth. The surplus water that does not infiltrate into pervious surfaces will leave the Site as surface water runoff. The infiltration factor varies



from 0 to 1 and is estimated based on topography, soils, and vegetation cover as per the *Stormwater Management Planning and Design Manual* (MOE, 2003).

The rate of infiltration at a Site is expected to vary, based on a number of factors to be considered in any infiltration model. To partition the available water surpluses into infiltration and surface run-off, the MECP infiltration factor was used. The MECP *Stormwater Management Planning and Design Manual* (MOE, 2003) methodology for calculating total infiltration based on topography, soil type and land cover was used, and a corresponding run-off component was calculated for the soil moisture storage conditions. The infiltration factor calculated for the Site is included in Table 4.

**Table 4 Infiltration Factor**

Category	Infiltration Factor
Topography	Rolling land = 0.20
Soils	Sandy loam = 0.35
Cover	Woodland and cultivated land = 0.15
<b>Total</b>	<b>0.70</b>

The calculation of infiltration and runoff in the stages of pre-development and post-development is provided in Appendix F, and are presented in Table 5 through Table 8, below. All the numbers were rounded off to the nearest integer.

### 7.3 Pre-Development Water Balance

The water balance for the existing conditions of the Site is summarized in Table 5. The pre-development infiltration rate was calculated to be 3,793 m<sup>3</sup>/yr and the runoff rate was 2,312 m<sup>3</sup>/yr.

**Table 5 Pre-Development Water Balance**

Land Use		Area (m <sup>2</sup> )	Precipitation (m <sup>3</sup> )	Evapo-transpiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
Impervious Areas	Paved Area	394	368	37	-	331
	Roof Area	423	395	39	-	355
Pervious Area	Landscaped Area	13,615	12,703	7,284	3,793	1,626
<b>Total</b>		<b>14,432</b>	<b>13,465</b>	<b>7,360</b>	<b>3,793</b>	<b>2,312</b>

*Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.*

#### 7.4 Post-Development Water Balance

The post-development water balance is summarized in Table 6. The post-development infiltration rate was calculated to be 1,426 m<sup>3</sup>/yr and the runoff volume was 8,431 m<sup>3</sup>/yr.

**Table 6 Post-Development Water Balance**

Land Use		Area (m <sup>2</sup> )	Precipitation (m <sup>3</sup> )	Evapo-transpiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
Impervious Areas	Paved Area	4,219	3,936	394	-	3,543
	Roof Area	5,093	4,752	475	-	4,277
Pervious Area	Landscaped Area	5,120	4,777	2,739	1,426	611
<b>Total</b>		<b>14,432</b>	<b>13,465</b>	<b>3,608</b>	<b>1,426</b>	<b>8,431</b>

*Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.*

#### 7.5 Water Balance Comparison

The water balances of the pre-development and post-development scenarios are summarized below in Table 7.



**Table 7 Comparison of Pre- and Post Development Water Balance**

	Precipitation (m <sup>3</sup> )	Evapotranspiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
<b>Pre-Development</b>	13,465	7,360	3,793	2,312
<b>Post-Development</b>	13,465	3,608	1,426	8,431
<b>Change in Volume</b>	-	- 3,752	- 2,367	6,119
<b>Change in %</b>	-	- 51	- 62	265

According to the above results, there is an infiltration deficit of 2,367 m<sup>3</sup>/year compared to the pre-development infiltration. The runoff rate upon development of the Site was increased by 6,119 m<sup>3</sup>/year.

As shown below in Table 8, approximately 55% of roof runoff is required to compensate the pre-development infiltration.

**Table 8 Requirement of Infiltration from Roof Run-off**

<b>Volume of Pre-Development Infiltration (m<sup>3</sup>/year)</b>	3,793
<b>Volume of Post-Development Infiltration (m<sup>3</sup>/year)</b>	1,426
<b>Deficit from Pre to Post-Development Infiltration (m<sup>3</sup>/year)</b>	2,367
<b>% of Roof Runoff required to match the Pre-Development Infiltration</b>	55

Following the above calculations, a summary of the water balance could be provided as follows:

1. There is a net increase in run-off at the Site of 6,119 m<sup>3</sup>/year, from 2,312 m<sup>3</sup>/year to 8,341 m<sup>3</sup>/year. This increase is a result of the development of the Site with more impervious areas such as roof and paved areas and decrease in pervious areas.
2. Post-development landscape area was decreased by about 8,495 m<sup>2</sup>, when compared to the pre-development landscape, causing less infiltration and more run-off across the Site.
3. Without implementing any mitigation measures, there is a net deficit of 2,367 m<sup>3</sup>/year in the post-development infiltration on a yearly basis.



4. Based on the estimation, a diversion of 55% of general roof water for infiltration would allow the proposed development to maintain an enhanced infiltration after the development. Therefore, Cambium would recommend the implementation of any Low Impact Development (LID) measures at the Site in its present design, in order to compensate the infiltration deficit caused due to the Site development.

## 7.6 Discussions on LID Measures

It is known that low impact development (LID) practices have received increasing attention as these strategies attempt to capture the runoff and mimic the natural hydrologic cycle.

In general, there are two primary categories of LIDs. The first promotes the infiltration of stormwater close to the source, besides reducing runoff and to improve the water quality. These infiltration type LIDs are preferred when hydrogeological and physical conditions are optimal and allow for their emplacement. The proposed development does not include a stormwater management pond; therefore, the second option described below should also be considered.

The second option captures and slowly releases the water to the groundwater system through the process of storage and filtration by infiltration LIDs. Infiltration targets may be achieved through the incorporation of a variety of stormwater management practices including reduced lot grading, roof downspout disconnection, roof leaders discharging to ponding areas or soak away pits, infiltration trenches, grassed swales etc.

The conceptual water balance indicates that there will be an infiltration deficit of about 2,367 m<sup>3</sup>/year in the post-development infiltration upon development of the Site, compared to the pre-development. Based on the estimation, a diversion of 55% of the general roof water for infiltration would allow the proposed development to maintain an enhanced infiltration after the development.

Given the proposed lot design by the proponent and based on relatively deep-water table conditions in the northern half of the property, the implementation of LID features is feasible in this area of the Site. However, LID measures are not feasible in the south and southeastern



corner where groundwater levels are shallow. Downspout disconnection is a better method for the implementation of LID measures in the northern part of the Site. It should be noted that the minimum distance between the invert elevation of a LID feature such as infiltration trench and the maximum elevation of the water table should be 1.0 m. As Cambium does not provide design of LID facilities, it would be beneficial to consult with stormwater engineers for the LID design recommendations.



## 8.0 Source Water Protection and Risk Management

As per the South Georgian Bay Lake Simcoe Source Protection Plan (SBSLS SPR, 2021), the Subject Site is located in the following vulnerable areas and shown in respective figures:

- Wellhead Protection Area C (WHPA-C) (Figure 8).
- WHPA Q1/Q2 with low stress, related to groundwater recharge management (Figure 9).
- Highly Vulnerable Aquifer (HVA) with a vulnerability score of 6 (Figure 10).
- Issues Contributing Area (ICA) with chloride as contaminant (Figure 11) and a
- Significant Groundwater Recharge Area (SGRA)

Land Use Planning (LUP) Policies to manage or eliminate a threat activity were implemented through land use planning decisions (such as Official Plans, Zoning By-laws, and Site Plan Controls). Also, as per Section 58 of the Clean Water Act, a risk management plan should be developed to ensure that the threat to drinking water ceases to be significant.

The following Source Protection policies are applicable to the Site, including the Restricted Land Use (RLU-1) policy to ensure that activities in the designated area are assessed for their potential risk, and:

**DNAPL-1:** Property is located in Wellhead Protection Area C (5-year travel time) and RLU prohibits the future handling and storage of dense non-aqueous phase liquids (DNAPL-1) in any quantity (excluding incidental volumes for personal/domestic use) on the property.

Although the Site is located within WHPA-C, there is no DNAPL source present on-site and therefore, a 'Contaminant Management Plan' is not required. Also, as far as the Site use is concerned, there will not be any storage or handling of DNAPL's at the Site, other than for personal/domestic use and therefore, the RLU-1 Policy is not applicable to the Site or proposed activities on the site.

**SALT (ICA)-3:** Handling and storage of road salt is a significant drinking water threat and therefore a Risk Management Plan (RMP) and a notice to proceed will be required.



As per the South Georgian Bay and Lake Simcoe Source Protection Plan, the SALT-(ICA)-3 is categorised as *Threat 12 and 13: Application, Handling and Storage of Road Salt - issues contributing area*.

The ICA's specific to sodium and chloride were delineated under the Clean Water Act (2006). The issues contributing areas are the land areas where road salt has already affected drinking water supplies and that the elevated level of sodium and chloride in the source water are attributed to the road salt-related activities. Therefore, a reduction in winter salt application is required to mitigate anticipated chloride increases in local watercourses as well as water supply aquifers.

The proposed development is for independent residential lots and therefore, storage and handling of large quantities of road salt does not arise. Therefore, the development of Salt Management Plans is not required for the proposed development.

Best management practices for general use should be encouraged which could include the use of better salt application technologies such as electronic spreader controllers, anti-icing, and pre-wetting which would prevent ice formation and lead to reduced use of salt and therefore its environmental impacts.

## **8.1 Groundwater Recharge Management (WHPA-Q).**

WHPA-Q or Recharge Management Area (WHPA-Q/RMA) are identified to ensure a sustainable supply of groundwater. RMA was identified through a water quantity risk assessment study considering the drought, future water demand, land-use change and population growth. Accordingly, the Site falls within a WHPA-Q/RMA.

A climate-based water balance assessment was completed for the pre- and post-development scenarios for the Site as described in Section 7.0. A climate-based water balance assessment is the first step to identify the post-development infiltration deficit and then utilize the Best Management Practices (BMPs) to minimize the reduction in groundwater recharge.

Accordingly, as per the water balance assessment completed above, it was estimated that there is a net infiltration deficit between the pre- and development scenarios in the order of



about 2,367 m<sup>3</sup> /annum. This infiltration deficit should be compensated for through the use of LID/infiltration features.

The proposed development does not involve any long- term or short-term dewatering and therefore, reduction to ground water supplies was not expected.

The preferred solution for recharge management is always for the post-development infiltration deficit to be mitigated during the development process. Surficial soils are relatively permeable for the infiltration to occur. Therefore, we recommend exploring all the lot level BMPs such as downspout disconnection or any other suitable measures at feasible locations to encourage the roof run-off infiltration in order to mitigate the estimated infiltration deficit.

## 8.2 Highly Vulnerable Aquifer

Highly Vulnerable Aquifer (HVA) are aquifers that are more sensitive to contamination. In general, a HVA will consist of granular materials (e.g., sand and/or gravel) or fractured rock that has a high permeability and is near the surface of the ground.

Site exists in an area of groundwater vulnerability with a score of 6 and threat activities are moderate, but not significant. Therefore, the land use practices at the proposed development Site are not expected to cause any contamination to the potable ground supplies. The Site will be under municipal sewage service and as such is not expected to cause any groundwater contamination by nitrates.

## 8.3 Significant Groundwater Recharge Area (SGRA)

Significant Groundwater Recharge Area (SGRA) exist in areas where water from surface more easily infiltrates the ground to recharge an aquifer that is used for municipal or other drinking water supplies. Under the Clean Water Act, 2006, an SGRA helps maintain the water level in an aquifer that supplies drinking water (including to private wells) and has higher than average recharge comparatively across the Source Protection Area.

As the development will create more impervious areas across the Site causing a reduction in infiltration, we recommend implementing lot level LID measures where practicable.



## 9.0 Assessment of Potential Impacts

As discussed, there is no construction dewatering required based on the nature of the development proposed at the Site. The potential impacts due to the Site development were assessed as follows.

### 9.1 Natural Features

As discussed, an unevaluated wetland is mapped in the northwestern portion of the Site, which also runs adjacent to the western boundary of the Site and to the South of the Site. Therefore, as per Ont. Reg. 172/06. (Regulation of Development, Interference with Wetlands and Alterations to Shorelines and Watercourses), set-back distances or buffer zones as prescribed by NVCA should be followed to protect the natural features.

By implementing additional lot level infiltration recommended in Section 7.6, there will be no net loss in groundwater recharge; therefore any potential off-site impacts to natural features will be negligible.

### 9.2 Water Supply Wells near the Site

Given that no dewatering activities should be required and groundwater recharge should be retained (assuming recommendations in Section 7.5 are followed), potential impacts to surrounding water wells (private or public), if any, are not anticipated. Moreover, it is assumed that all the properties surrounding the Site are under municipal water and wastewater service and as such no impacts are anticipated on the groundwater regime.

### 9.3 Considerations on Drinking Water Vulnerability

As the Site is identified to include numerous groundwater vulnerable areas, the best management practices as discussed in Section 8.0 should be implemented across the Site to reduce water quality and quantity impacts on the municipal water supply aquifers.

Given that the Site is in an urbanized area, it's assumed that the neighbouring properties would be relying on the City's water supply. The proposed development may reduce the pervious area at the Site, which would result in decreased infiltration into subsurface and



decrease in groundwater recharge. There is a net deficit in post-development water balance due to the Site development, ultimately reducing the groundwater recharge to the local groundwater regime. Therefore, Cambium recommends implementing the lot level LID measures where possible across the Site in order to reduce the infiltration deficit, although water quantity threat is considered low due to the proposed development.

Considering that the municipal water wells were usually constructed in deep aquifers and no dewatering is required at the proposed development, the impact on the groundwater recharge is low to negligible.



## 10.0 Conclusions and Recommendations

2609216 Ontario Inc (Client) has retained Cambium Inc. to complete a hydrogeological assessment at 152 and 156 Miller Drive, City of Barrie, Ontario.

The majority of the Site is within a regulated area within the NVCA jurisdiction (as per O.Reg. 172/06) and therefore, development restrictions do apply to the proposed development. There is an unevaluated wetland mapped in the northwestern corner of the Site, which also runs adjacent to the western boundary of the Site and to the south of the Site.

Groundwater levels were measured to range from as shallow as 0.10 mbgs to as deep as 4.49 mbgs, with elevations ranging from 234.87 masl to 236.65 masl. Each dwelling in the development will be finished with half-basements or walkout basements and therefore, no dewatering was expected in most parts of Site due to proposed raise in grade to accommodate the basements. Short-term dewatering is not anticipated to be required for the installation of linear infrastructure but should be reassessed when a detailed servicing plan is prepared.

Soil percolation rates ranged from 6 to 8 min/cm, with a geometric mean of 7 min/cm, indicating the presence high transmissive soils at the Site.

The conceptual water balance indicates that there will be an infiltration deficit upon development of the Site in the order of about 2,367 m<sup>3</sup>/year. It is Cambium's opinion that the infiltration deficit can be compensated for the proposed post-development plan if roof runoff is directed into a suite of LID measures (i.e. infiltration trench/gallery, roof downspout disconnection, etc.). The LID features, if any should be designed by a qualified person in order to compensate the infiltration deficit as determined.

Adjacent landowners/properties are assumed to be on the City's water supply network and therefore are not anticipated to be influenced from the proposed development. However detailed development and construction plans should be reviewed (once prepared) for each lot to determine if dewatering is required during construction/operation of the development, and the potential influence of dewatering activities, if any.



Although, the Site is situated in a number of vulnerable areas, such as WHPA-C, WHPA-Q, HVA, SGRA and ICA related to Source Water Protection, no impacts on the municipal water supply aquifer were anticipated, provided best management practices are implemented across the Site.



## 11.0 Closing

We trust that the information in this submission meets your current requirements. If you have any questions regarding the contents of this report, please contact the undersigned.

Respectfully submitted,

### Cambium Inc.

DocuSigned by:

6AAA0661AAA8459

Nicole Latimer, M.Sc., GIT  
Project Coordinator

DocuSigned by:

677E3F2E4427404

Kevin Warner, M.Sc., P. Geo (Ltd).  
Group Manager – Water and Wastewater

DocuSigned by:

3611EDDBEA134BF

Sudhakar Kurli, M.Sc., P.Geo.  
Project Manager/Hydrogeologist



## 12.0 References

Cambium. (2019). *Geotechnical Investigation Report - 156 Miller Drive, Barrie, Ontario*. Cambium Inc.

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OGS. (2007). *Paleozoic Geology of Southern Ontario*. Ontario Geological Survey, Miscellaneous Release - Data 219.

SBSLS SPR. (2021). *Approved South Georgian Bay Lake Simcoe Source Protection Plan*. South Georgian Bay Lake Simcoe Source Protection Region.

Thornthwaite, C., & Mather, J. (1957). *Instructions and tables for computing potential evapotranspiration and the water balance*. Centerton, N.J.: Laboratory of Climatology, Publications in Climatology, v. 10, no. 3, p. 185-311.



## 13.0 Standard Limitations

### Limited Warranty

In performing work on behalf of a client, Cambium relies on its client to provide instructions on the scope of its retainer and, on that basis, Cambium determines the precise nature of the work to be performed. Cambium undertakes all work in accordance with applicable accepted industry practices and standards. Unless required under local laws, other than as expressly stated herein, no other warranties or conditions, either expressed or implied, are made regarding the services, work or reports provided.

### Reliance on Materials and Information

The findings and results presented in reports prepared by Cambium are based on the materials and information provided by the client to Cambium and on the facts, conditions and circumstances encountered by Cambium during the performance of the work requested by the client. In formulating its findings and results into a report, Cambium assumes that the information and materials provided by the client or obtained by Cambium from the client or otherwise are factual, accurate and represent a true depiction of the circumstances that exist. Cambium relies on its client to inform Cambium if there are changes to any such information and materials. Cambium does not review, analyze or attempt to verify the accuracy or completeness of the information or materials provided, or circumstances encountered, other than in accordance with applicable accepted industry practice. Cambium will not be responsible for matters arising from incomplete, incorrect or misleading information or from facts or circumstances that are not fully disclosed to or that are concealed from Cambium during the provision of services, work or reports.

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When preparing reports, Cambium considers applicable legislation, regulations, governmental guidelines and policies to the extent they are within its knowledge, but Cambium is not qualified to advise with respect to legal matters. The presentation of information regarding applicable legislation, regulations, governmental guidelines and policies is for information only and is not intended to and should not be interpreted as constituting a legal opinion concerning the work completed or conditions outlined in a report. All legal matters should be reviewed and considered by an appropriately qualified legal practitioner.

### Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

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Potential liability to the client arising out of the report is limited to the amount of Cambium's professional liability insurance coverage. Cambium shall only be liable for direct damages to the extent caused by Cambium's negligence and/or breach of contract. Cambium shall not be liable for consequential damages.

### Personal Liability

The client expressly agrees that Cambium employees shall have no personal liability to the client with respect to a claim, whether in contract, tort and/or other cause of action in law. Furthermore, the client agrees that it will bring no proceedings nor take any action in any court of law against Cambium employees in their personal capacity.



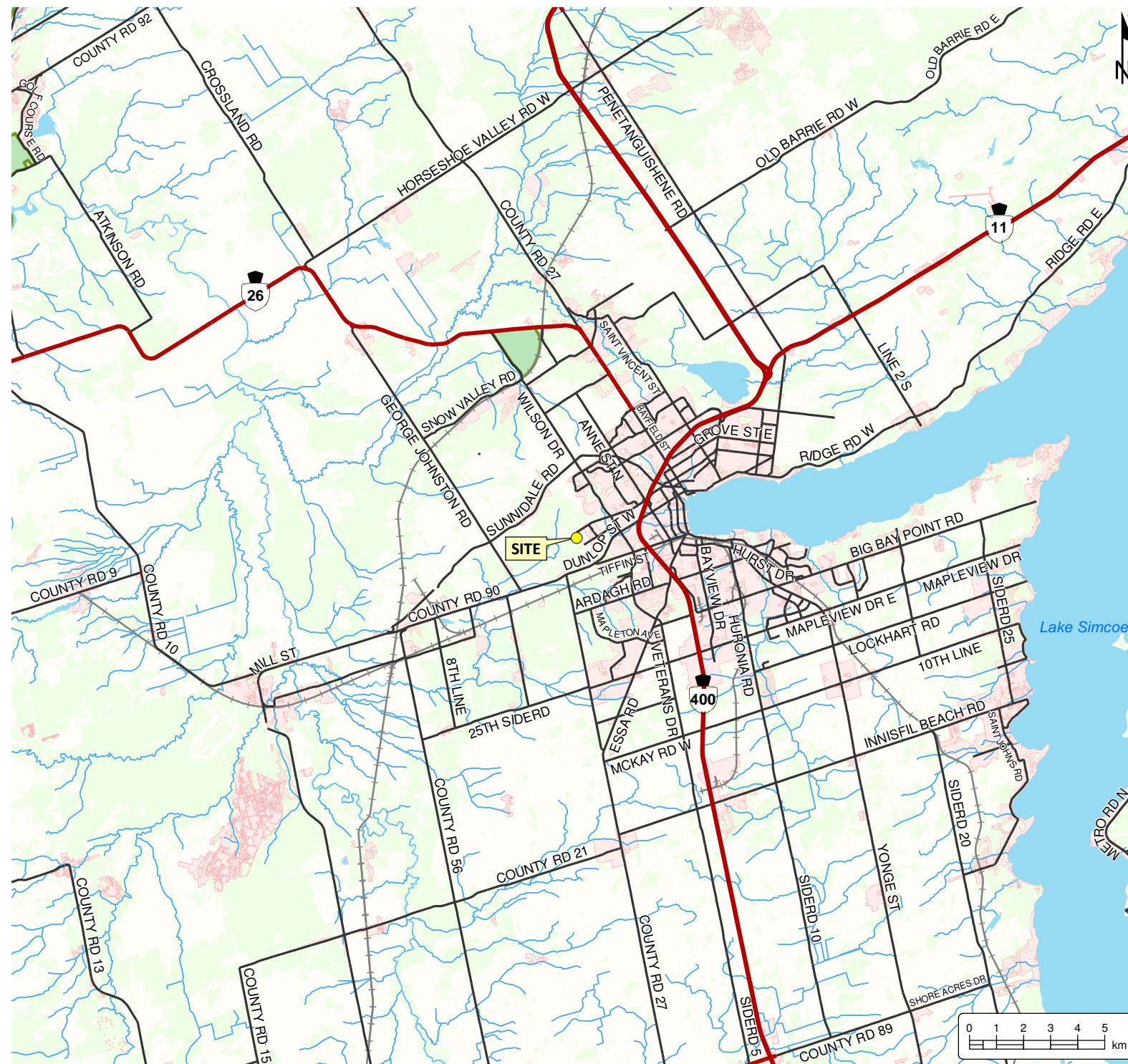
Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

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## Appended Figures

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# HYDROGEOLOGICAL INVESTIGATION

2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

## LEGEND

N

- Base mapping features are © Queen's Printer of Ontario, 2019 (this does not constitute an endorsement by the Ministry of Natural Resources or the Ontario Government).
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Tel: (705) 742.7900 Fax: (705) 742.7907  
[www.cambium-inc.com](http://www.cambium-inc.com)

## REGIONAL LOCATION PLAN

Project No.:		Date: May 2024
		Rev.:
Scale:		Projection:
1:200,000		NAD 1983 UTM Zone 17N
Created by:	Checked by:	Figure:
MAT	SK	1



**HYDROGEOLOGICAL  
INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

Site (approximate)

**Notes:**

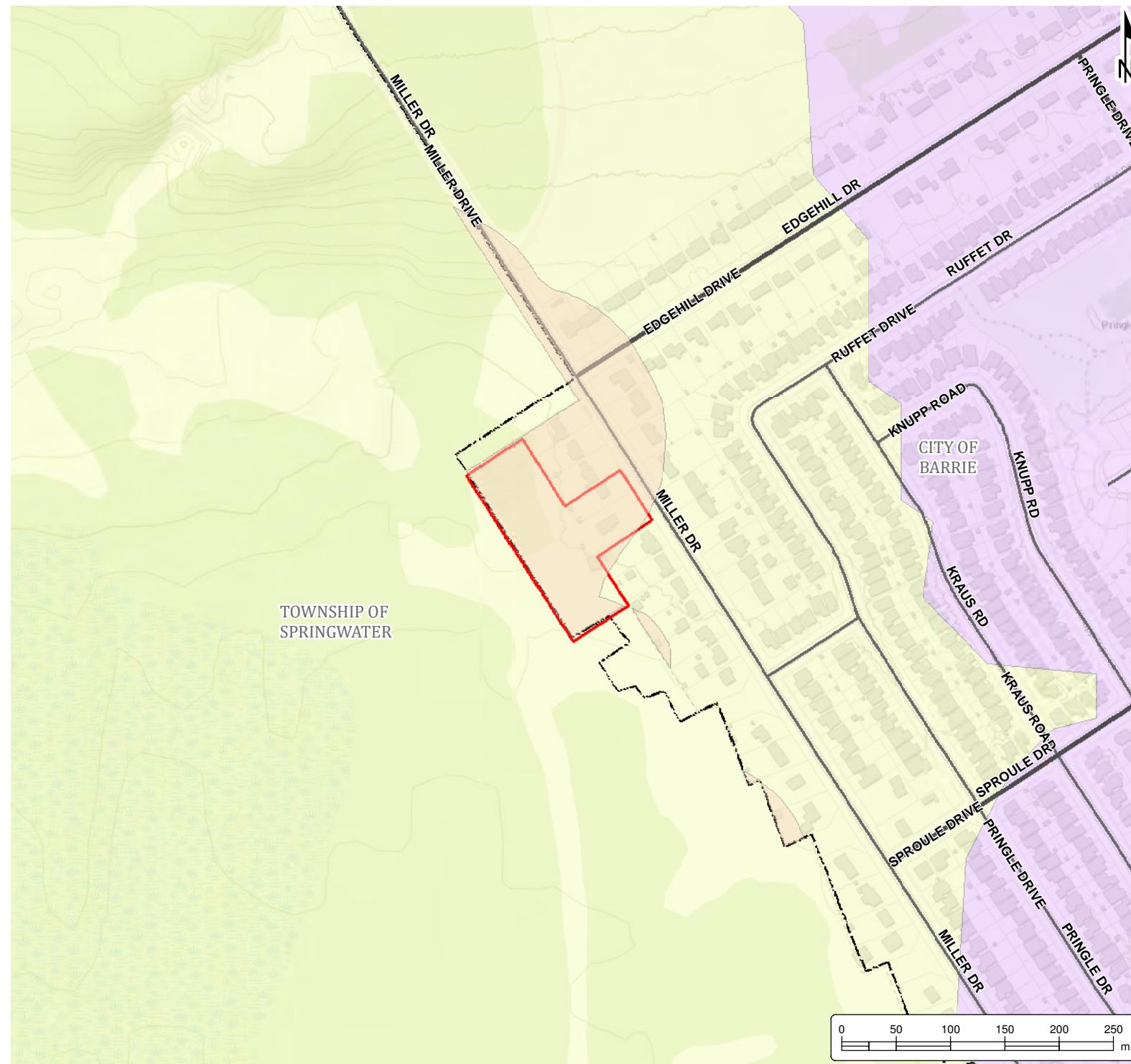
- Site plan overlay was created by Gerrits Engineering, project No. 1981-001-23, dated Feb 18, 2022.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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**SITE PLAN**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	
1:1,000	NAD 1983 UTM Zone 17N	
Created by:	Checked by:	Figure:
MAT	SK	2



## HYDROGEOLOGICAL INVESTIGATION

2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

### LEGEND

- Major Road
- Minor Road
- Conservation Authority Regulation Limits
- Lake Simcoe Region Conservation Authority
- Nottawasaga Valley Conservation Authority
- Site (approximate)

**Notes:**

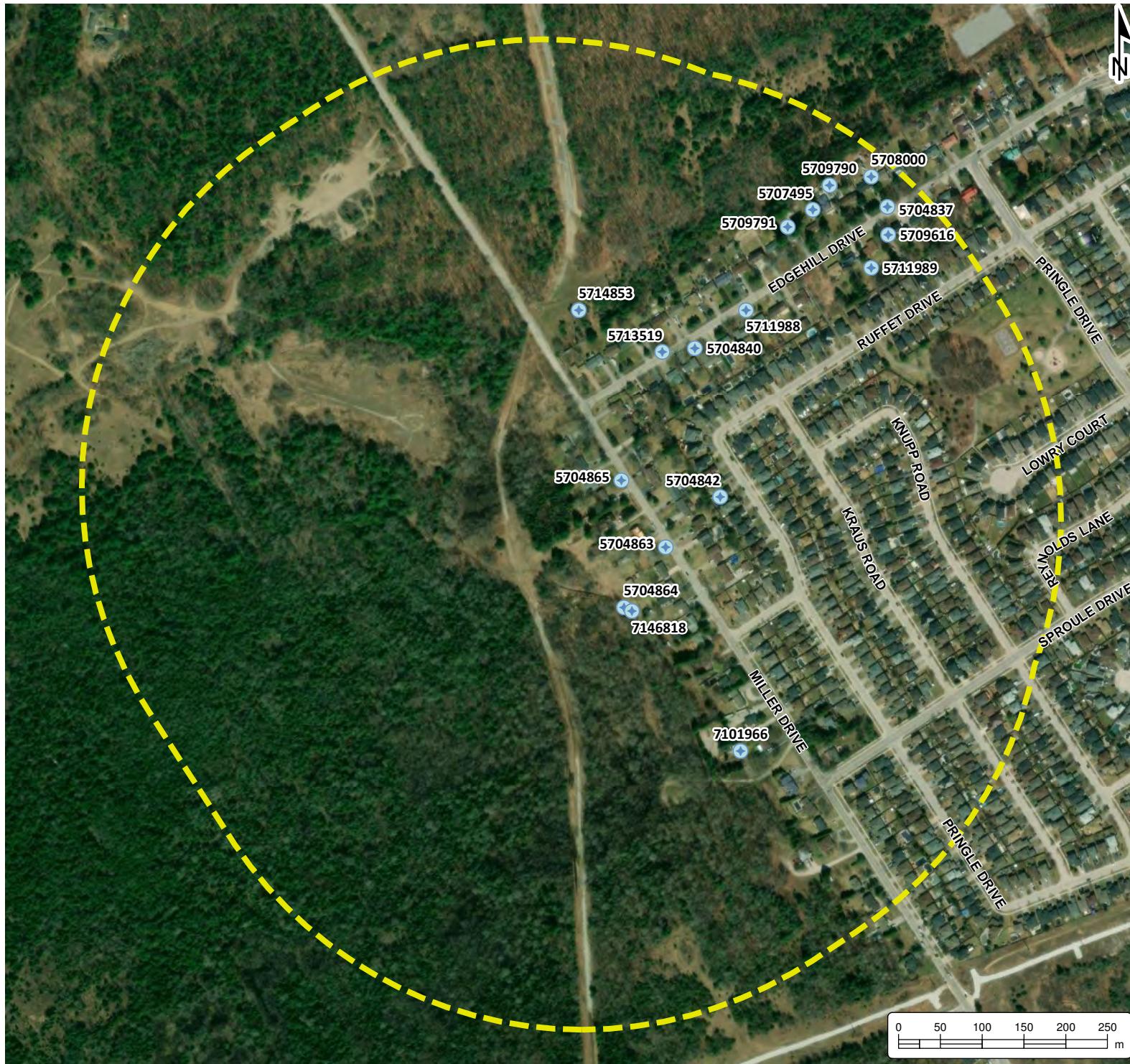
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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### CONSERVATION AUTHORITY REGULATION LIMIT

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	
1:5,000	NAD 1983 UTM Zone 17N	
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MAT	SK	3





**HYDROGEOLOGICAL  
INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Borehole
- Monitoring Well
- Site (approximate)

**Notes:**  
 - Site is approximate; boundaries were obtained from Simcoe County online GIS database.  
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**BOREHOLE LOCATION PLAN**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	
1:1,000	NAD 1983 UTM Zone 17N	
Created by:	Checked by:	Figure:
MAT	SK	5



**HYDROGEOLOGICAL  
INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Paved Area (394 m<sup>2</sup>)
- Roof Area (423 m<sup>2</sup>)
- Landscaped Area (13,615 m<sup>2</sup>)
- Site (approximate)

**NOTES:**

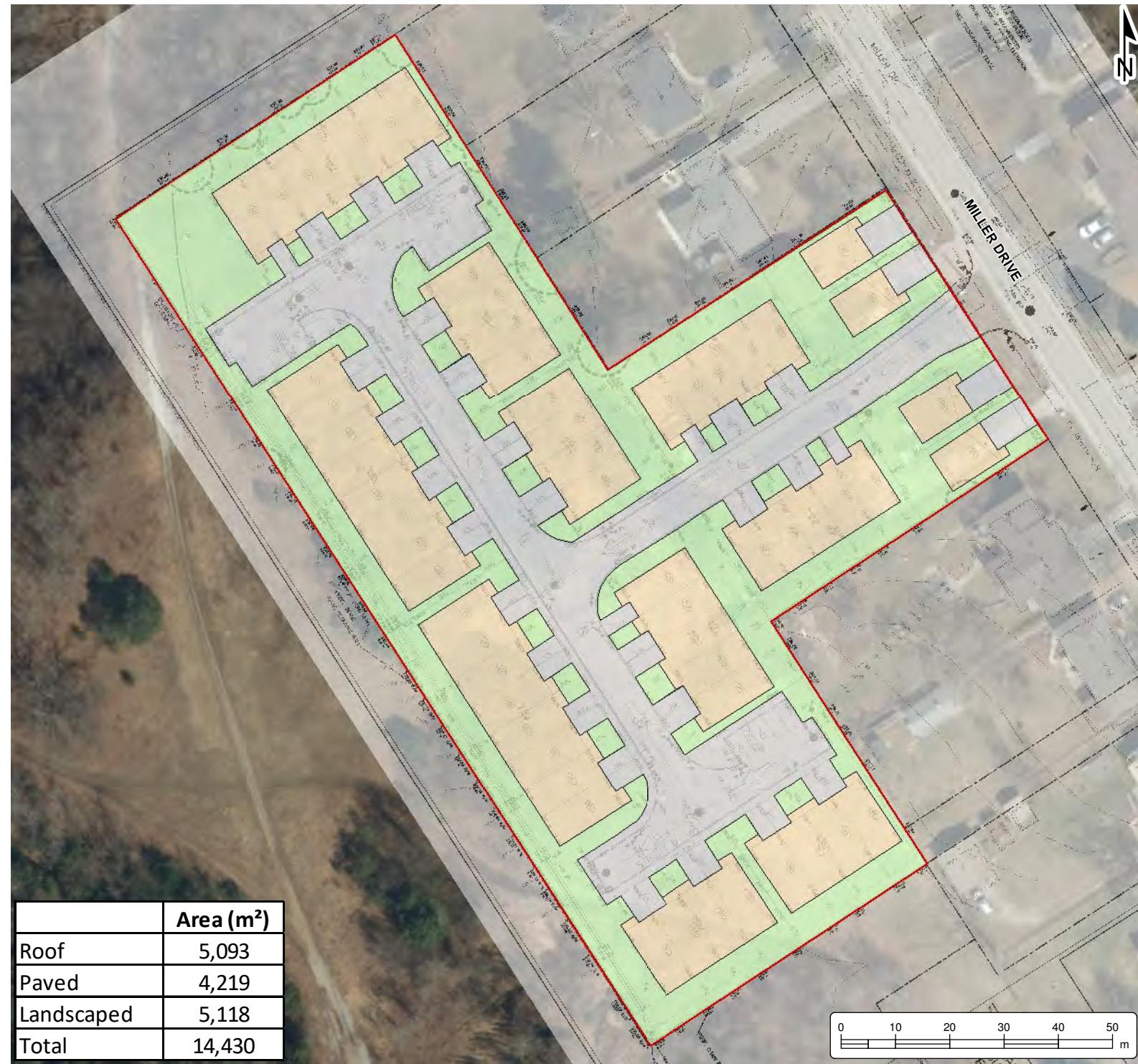
- Aerial data were obtained from the Nottawasaga Valley Conservation Authority.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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**PRE DEVELOPMENT PLAN**

Project No.:	Date:	May 2024
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1:1,000	NAD 1983 UTM Zone 17N	
Created by:	Checked by:	Figure:
MAT	SK	6



## HYDROGEOLOGICAL INVESTIGATION

2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

### LEGEND

- Roof Area
- Paved Area
- Landscape Area
- Site (approximate)

**Notes:**

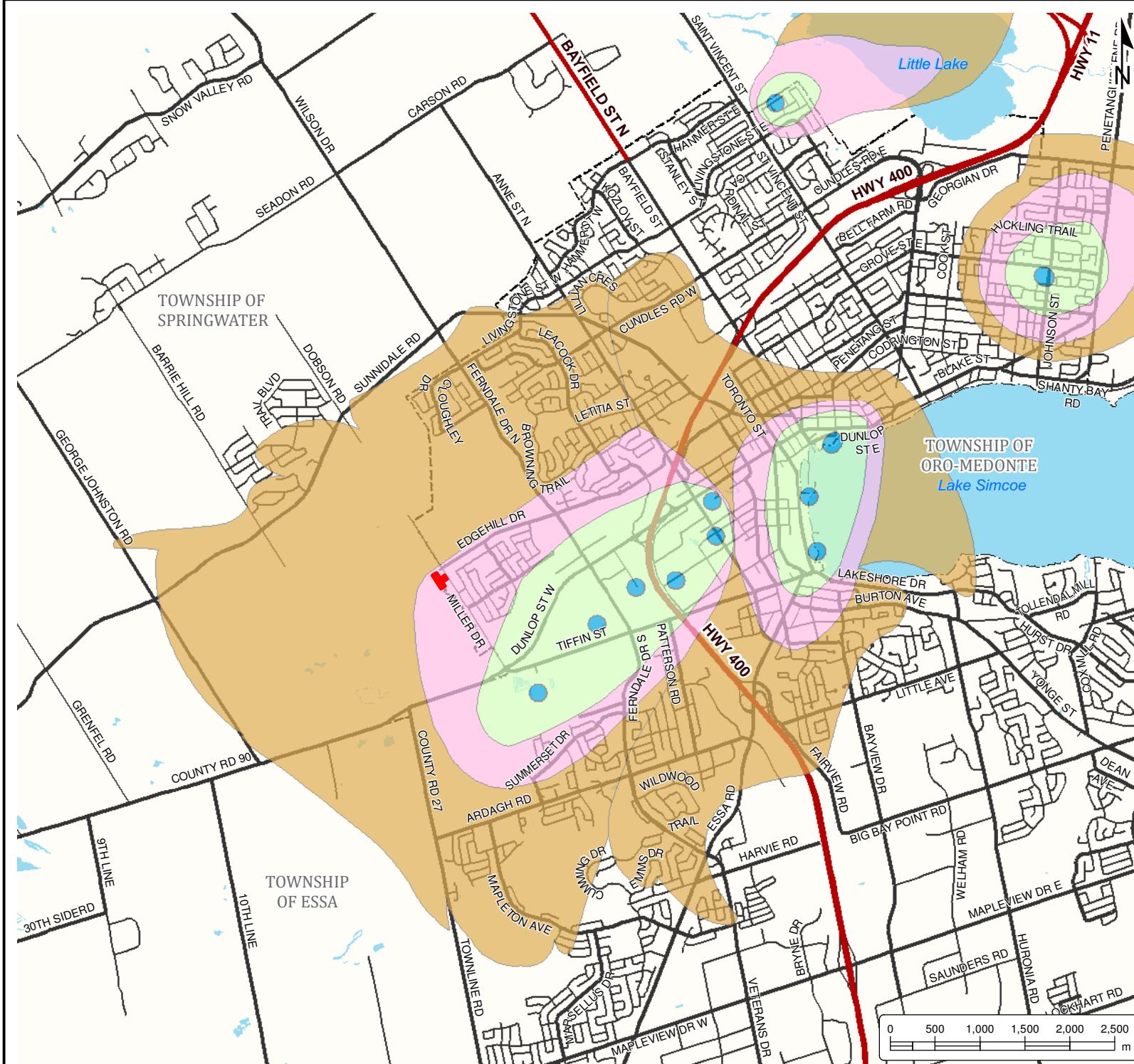
- Site plan overlay was created by Gerrits Engineering, project No. 1981-001-23, Plan No. SG-1, titled Site Grading Plan.
- The site is approximate; boundaries were obtained from Simcoe County online GIS database.
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### POST DEVELOPMENT PLAN

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	
1:1,000	NAD 1983 UTM Zone 17N	
Created by:	Checked by:	Figure:
MAT	SK	7



**HYDROGEOLOGICAL INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Highway
- Major Road
- Minor Road
- 100m Radius Offset From Operational Municipal Well
- 2 Year Capture Zone
- 10 Year Capture Zone
- 25 Year Capture Zone
- Water Area
- Site (approximate)

**NOTES:**

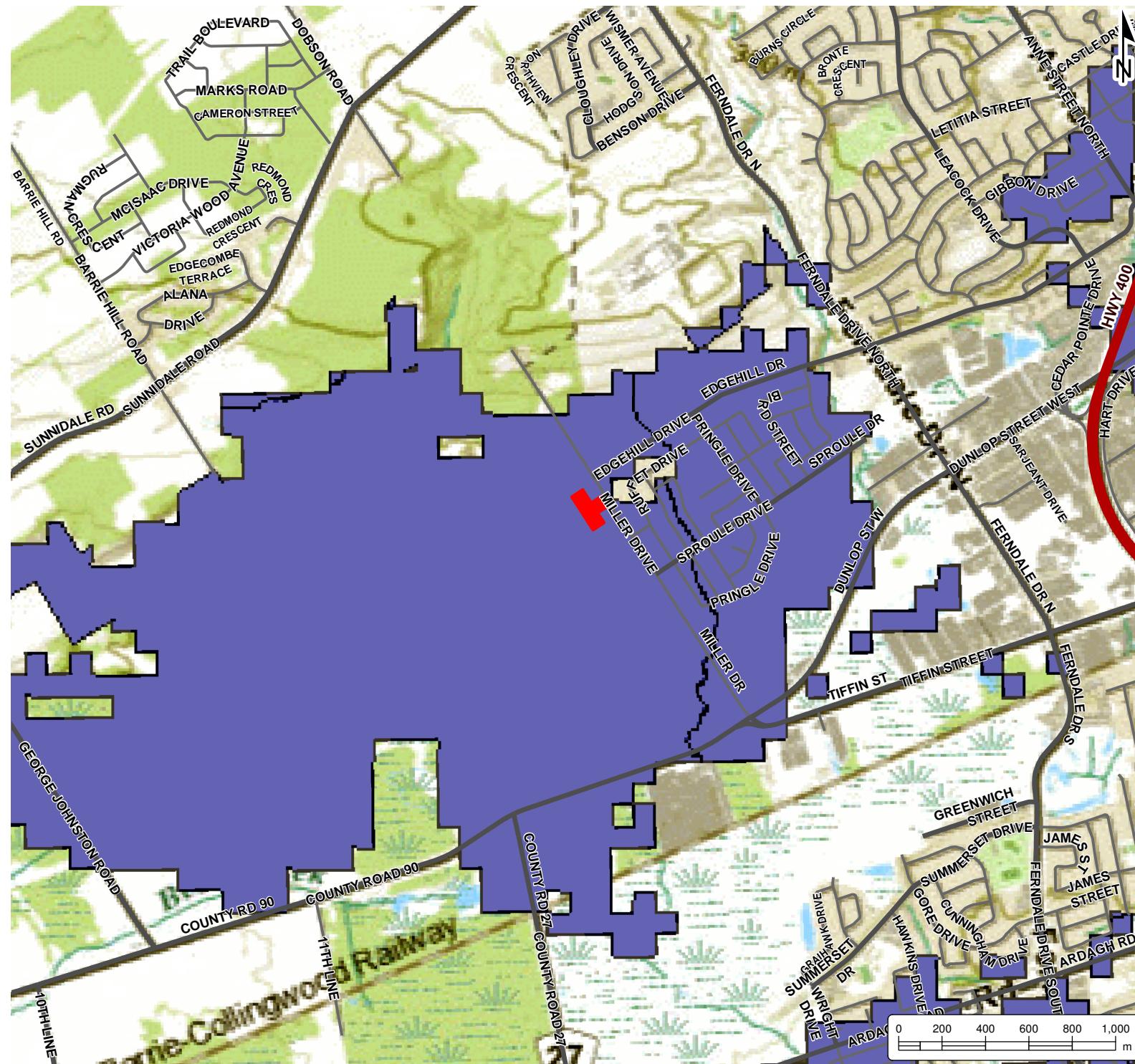
- Wellhead protection data was obtained from the City of Barrie Official Plan Schedule G, Wells and Wellhead Protection Areas.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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Peterborough, Ontario, K9H 1E5  
Tel: (705) 742-7900 Fax: (705) 742-7907  
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**WELLHEAD PROTECTION AREAS (WHPA) MAP**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	NAD 1983 UTM Zone 17N
1:60,000		
Created by: MAT	Checked by: SK	Figure: 8



**HYDROGEOLOGICAL INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Highway
- Major Road
- Minor Road
- Site (approximate)
- Highly Vulnerable Area

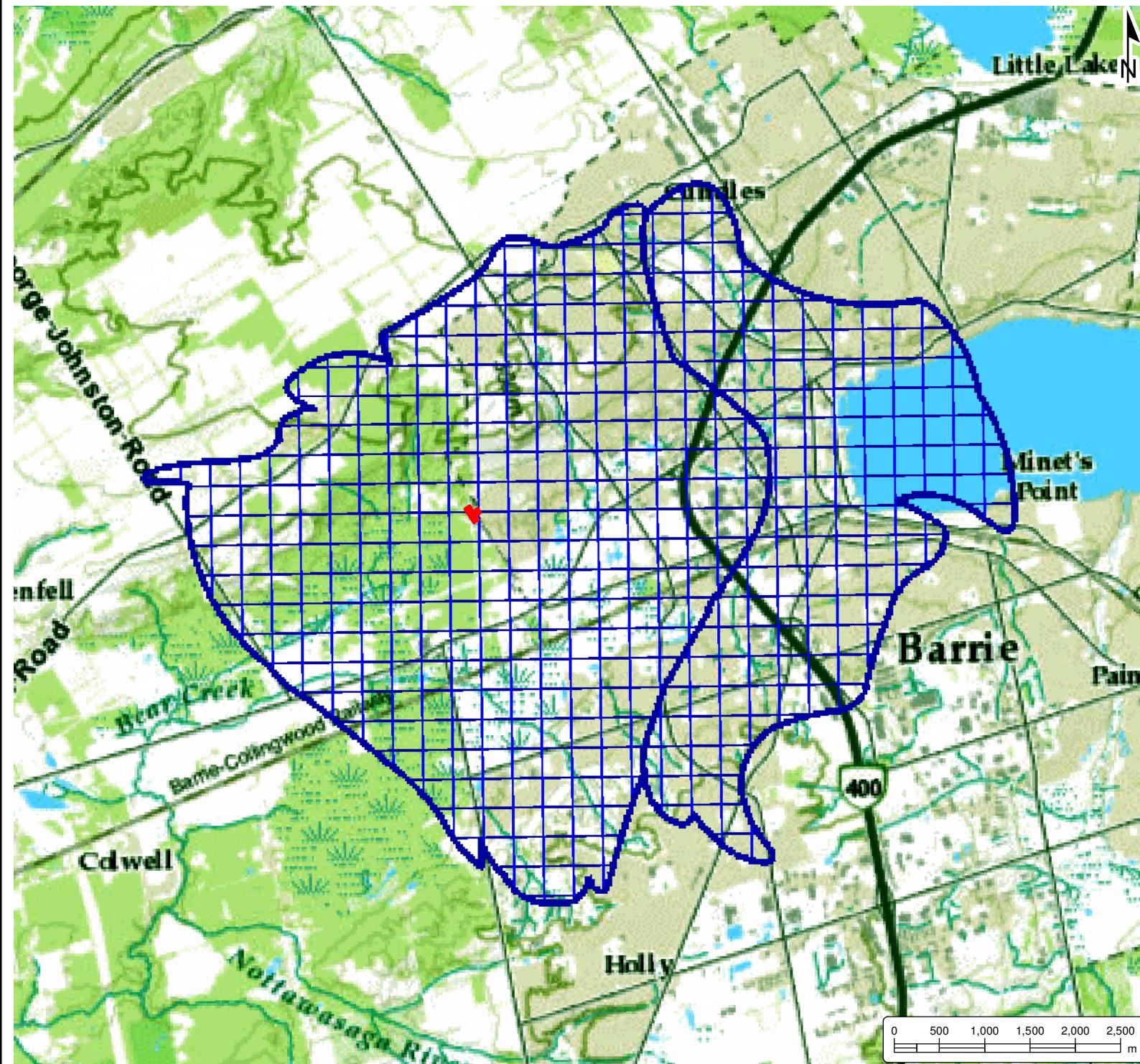
**NOTES:**

- All data were obtained from the Nottawasaga Valley Conservation Authority.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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**HIGHLY VULNERABLE AREAS MAP**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	NAD 1983 UTM Zone 17N
1:25,000		
Created by:	Checked by:	Figure:
MAT	SK	9



**HYDROGEOLOGICAL  
INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Site (approximate) (Red dot)
- Issue Contributing Area (Blue polygon)

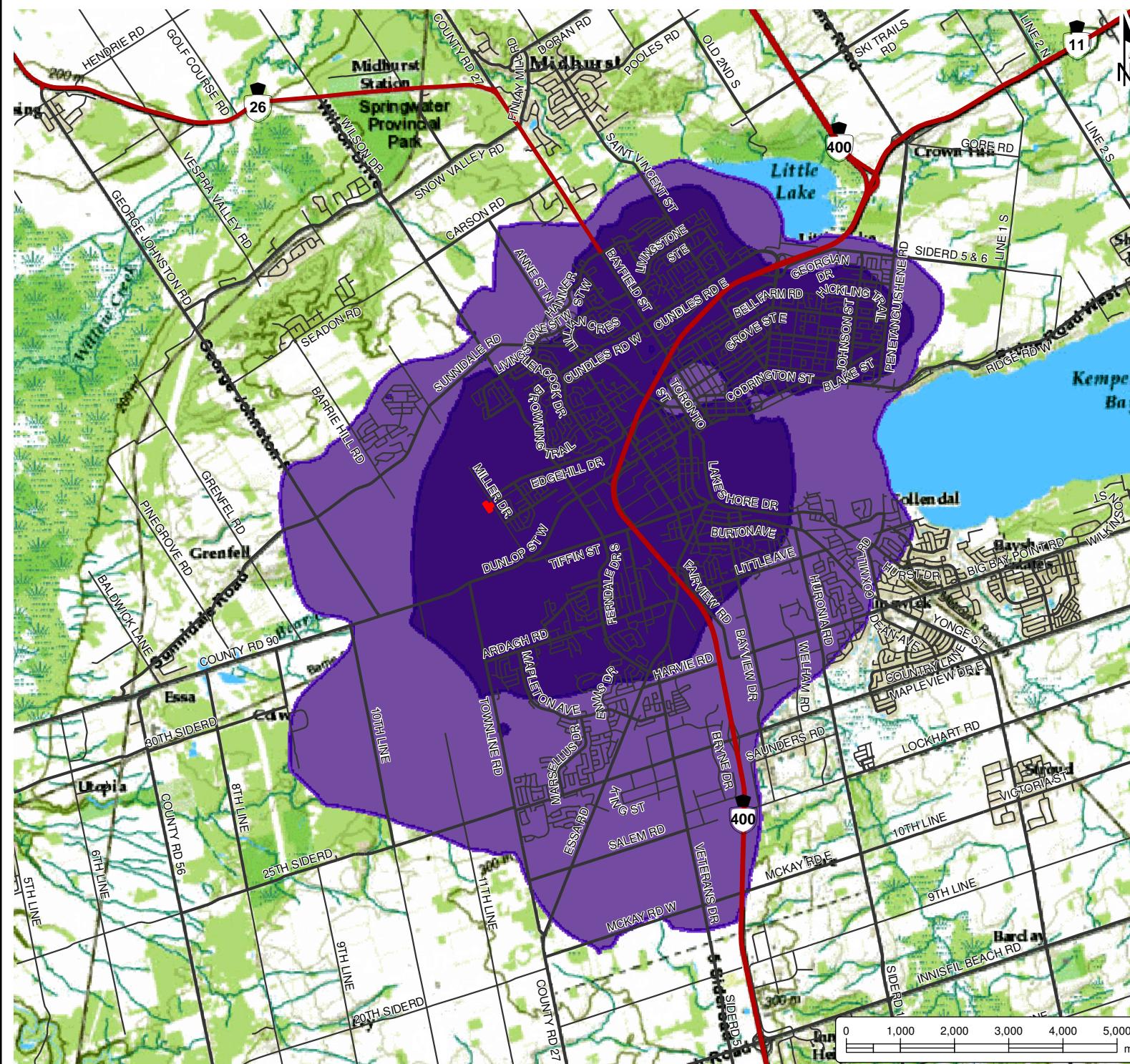
**NOTES:**

- All data were obtained from the Nottawasaga Valley Conservation Authority.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
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**ISSUE CONTRIBUTING  
AREA MAP**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	NAD 1983 UTM Zone 17N
1:60,000		
Created by:	Checked by:	Figure:
MAT	SK	10



**HYDROGEOLOGICAL INVESTIGATION**  
2609216 ONTARIO INC.  
152 & 156 Miller Drive,  
Barrie, Ontario

**LEGEND**

- Highway
- Major Road
- Minor Road
- Site (approximate)
- Wellhead Protection Area Q1
- Wellhead Protection Area Q2

**NOTES:**

- Hydro data were obtained from the Nottawasaga Valley Conservation Authority.
- Site is approximate; boundaries were obtained from Simcoe County online GIS database.
- Base mapping features are © Queen's Printer of Ontario, 2019 (this does not constitute an endorsement by the Ministry of Natural Resources or the Ontario Government).
- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.
- Cambium makes every effort to ensure this map is free from errors but cannot be held responsible for any damages due to error or omissions. This map should not be used for navigation or legal purposes. It is intended for general reference use only.



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Peterborough, Ontario, K9H 1E5  
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www.cambium-inc.com

**WELL HEAD PROTECTION AREA - Q1/Q2 MAP**

Project No.:	Date:	May 2024
9089-002	Rev.:	
Scale:	Projection:	NAD 1983 UTM Zone 17N
1:100,000		
Created by: MAT	Checked by: SK	Figure: 11



Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

---

## **Appendix A**

### **Proposed Development Plans**

---

## LEGEND

220.75 PROPOSED ELEVATION	T=TOP WALL
220.75 EXISTED ELEVATION	B= BOTTOM WALL
2.0% PROPOSED GRADE	
3:1 SLOPE	
SWALE	
HIGH POINT	

243.09 GENERALLY RECOMMENDED FIRST FLOOR ELEVATION	USF= 240.29 GENERALLY RECOMMENDED UNDERSIDE OF FOOTING ELEVATION
SPL SPLIT DRAINED	TYPICAL SOAKAWAY PIT
R2F REAR TO FRONT	
WO WALKUP LOT	
WL WALKUP LOT	
LO LOOKOUT LOT	
LU LOOKUP LOT	

TYPICAL SOAKAWAY PIT

SPL SPLIT DRAINED

R2F REAR TO FRONT

WO WALKUP LOT

WL WALKUP LOT

LO LOOKOUT LOT

LU LOOKUP LOT

TYPICAL SOAKAWAY PIT

SPL SPLIT DRAINED

R2F REAR TO FRONT

WO WALKUP LOT

WL WALKUP LOT

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TYPICAL SOAKAWAY PIT

SPL SPLIT DRAINED

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TYPICAL SOAKAWAY PIT

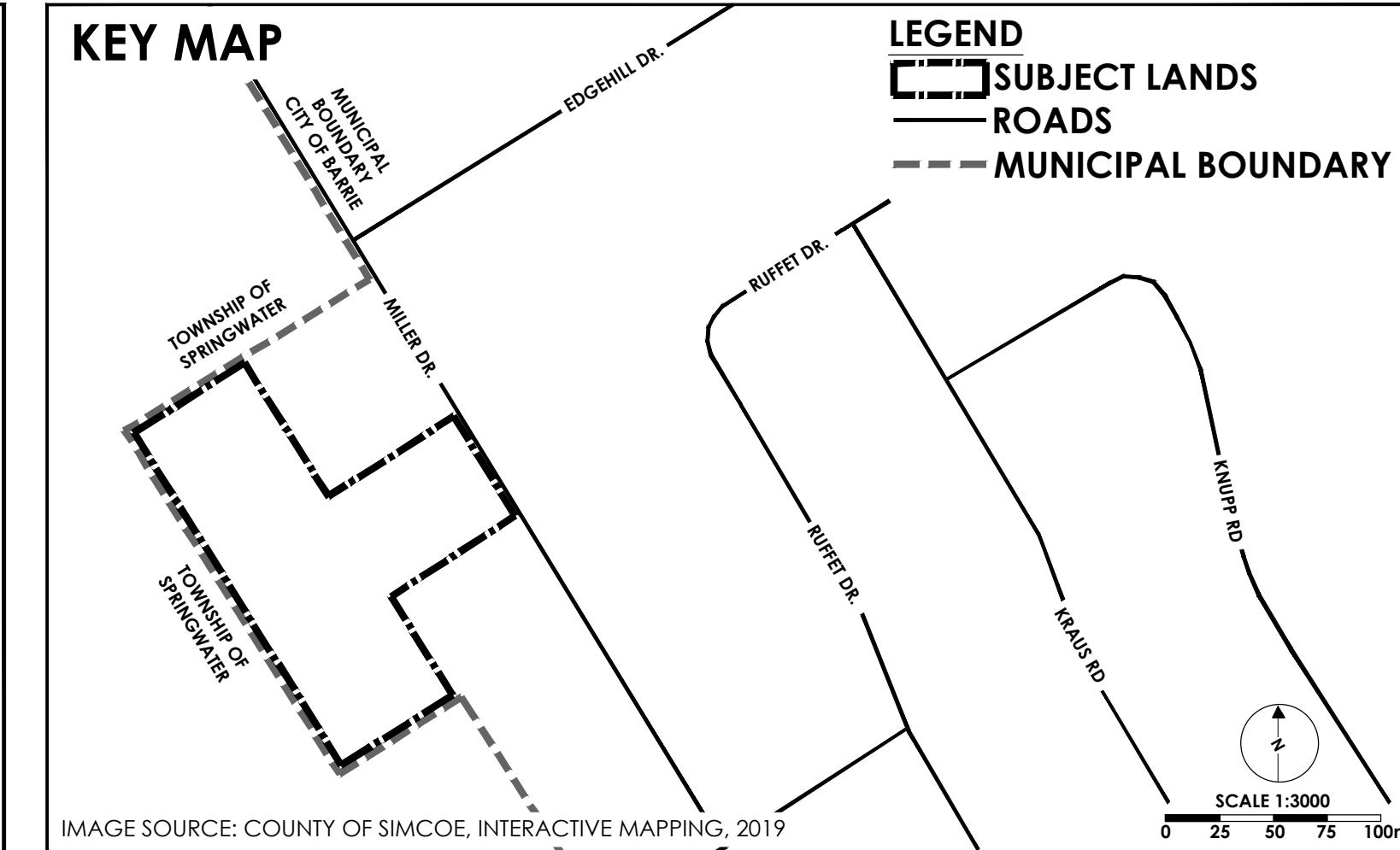
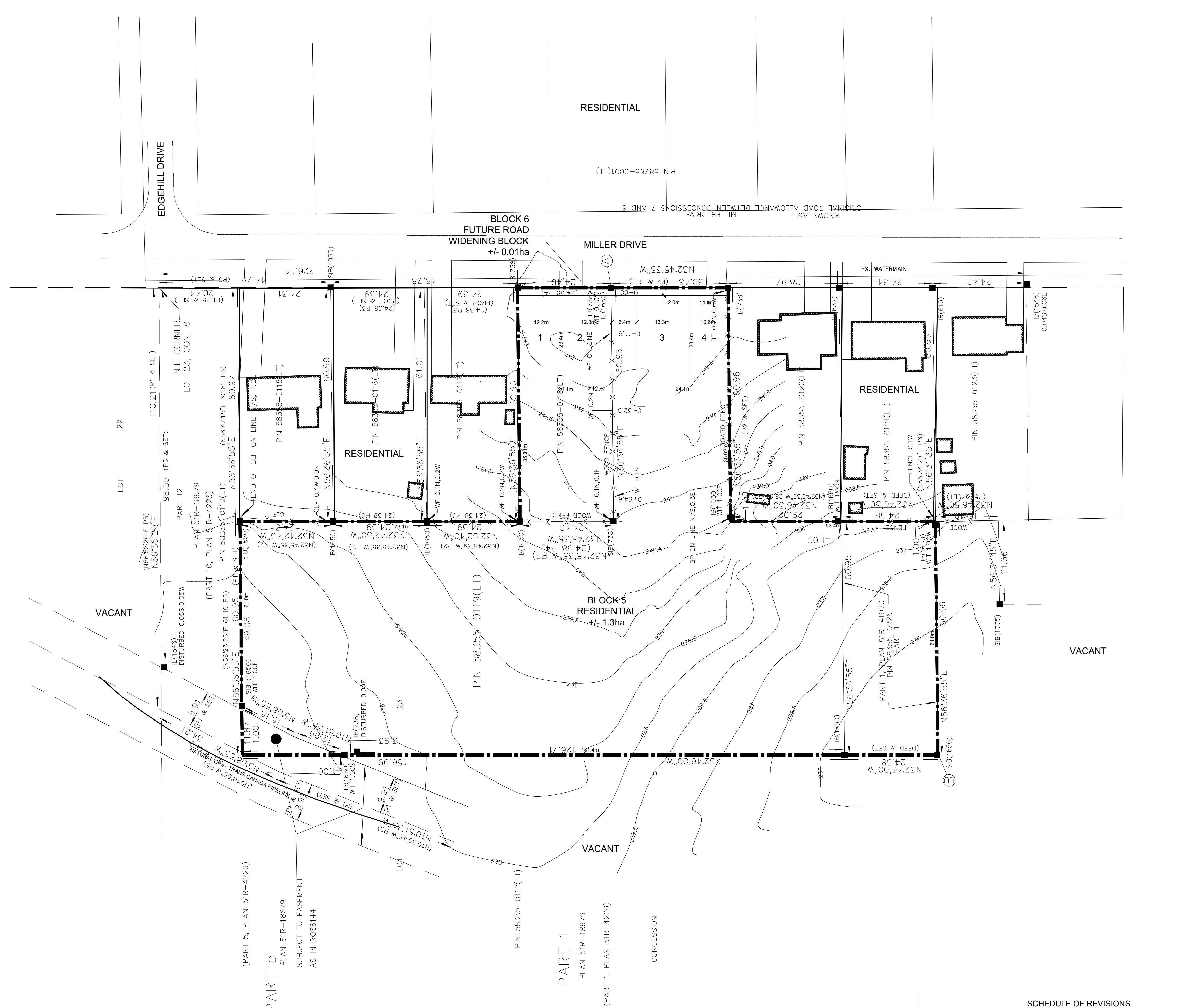
SPL SPLIT DRAINED

R2F REAR TO FRONT

WO WALKUP LOT

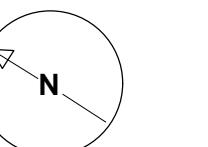
WL WALKUP LOT

LO LOOKOUT LOT



# DRAFT PLAN OF SUBDIVISION

LOT 23, CONCESSION 8,  
(GEOGRAPHIC TOWNSHIP OF VESPRA),  
CITY OF BARRIE,  
COUNTY OF SIMCOE



SCALE 1: 500



50 75 100 125

**LEGEND**

## SUBJECT LANDS

OWNER'S CERTIFICATE  
I HEREBY AUTHORIZE INNOVATIVE PLANNING SOLUTIONS TO PREPARE THIS DRAFT PLAN OF  
SUBDIVISION AND SUBMIT THIS DRAFT PLAN OF SUBDIVISION FOR APPROVAL.

---

DATE

October 1, 20  
DATE

SURVEYOR'S CERTIFICATE  
I CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO  
ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

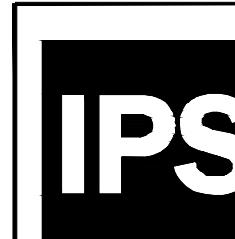
October

DINO ASTRI SURVEYING LTD.

- a) SHOWN ON PLAN
- b) SHOWN ON PLAN
- c) SEE KEY PLAN
- d) RESIDENTIAL
- e) SHOWN ON PLAN
- f) SHOWN ON PLAN
- g) SHOWN ON PLAN
- h) MUNICIPAL WATER
- i) SAND, SILT GLACIAL TILL
- j) SHOWN ON PLAN
- k) MUNICIPAL WATER & SEWAGE
- l) NONE

LAND USE STATISTICS

LAND USE	LOT No.	BLOCK No.	UNITS	AREA (ha)
RESIDENTIAL	1- 4	5	43	1.43
FUTURE ROAD WIDENING BLOCK		6		0.01
TOTAL	1	6	43	1.44



# INNOVATIVE PLANNING SOLUTIONS

PLANNERS • PROJECT MANAGERS • LAND DEVELOPERS

150 DUNLOP STREET EAST, SUITE 201, BARRIE, ONTARIO L4M 1B1

Date: April 2, 2019

Checked By: A.P

File: 17-702

Drawn By: A.S

SCHEDULE OF REVISIONS			
No.	Date	Description	By
1	Oct. 1, 2019	Decrease width of Road Widening;	AS



## Natural Heritage Mapping

Map created: 5/15/2024



0.3

0

0.16

0.3 Kilometres

Absence of a feature in the map does not mean they do not exist in this area.

This map should not be relied on as a precise indicator of routes or locations, nor as a guide to navigation. The Ontario Ministry of Natural Resources and Forestry(OMNRF) shall not be liable in any way for the use of, or reliance upon, this map or any information on this map.

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© King's Printer for Ontario, 2024

### Legend

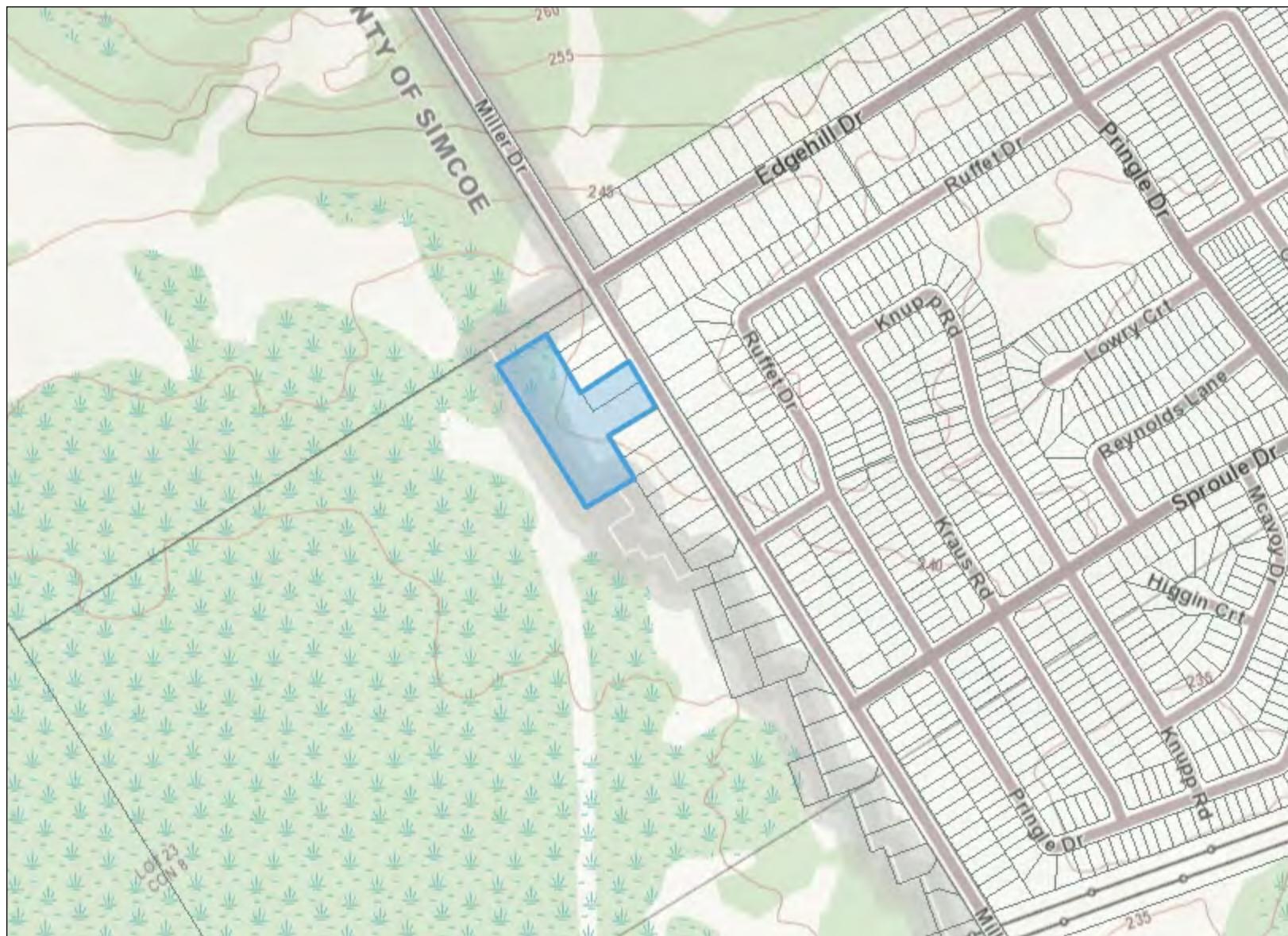
- Assessment Parcel
- ANSI
- Earth Science Provincially Significant/sciences de la terre d'importance provinciale
- Earth Science Regionally Significant/sciences de la terre d'importance régionale
- Life Science Provincially Significant/sciences de la vie d'importance provinciale
- Life Science Regionally Significant/sciences de la vie d'importance régionale
- Evaluated Wetland
- Provincially Significant/considérée d'importance provinciale
- Non-Provincially Significant/non considérée d'importance provinciale
- Unevaluated Wetland
- Conservation Reserve
- Provincial Park
- Natural Heritage System





Notes:

## Topographic Map



0 0.3 km

Projection: Web Mercator



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## Legend





Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

---

## Appendix B

### MECP Well Records

---



**Peterborough**  
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## Log of Borehole:

BH101-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914192.74, 600358.14

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 236.42 mASL

SUBSURFACE PROFILE			SAMPLE											
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
								25	50	75	10	20	30	40
0			Topsoil: Dark brown topsoil, moist, loose	1A	SS	100	4							
236			Sand: Brown fine sand, trace silt, moist, loose	1B										
1				2	SS	70	9							
235				3	SS	50	14							
2				4	SS	65	8							
234				5	SS	90	9							
3				6	SS	75	9							
233														
4														
232														
5														
231			Borehole terminated at 5.0 mbgs											
6														



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## Log of Borehole:

BH102-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914216.82, 600348.92

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 237.51 mASL

SUBSURFACE PROFILE			SAMPLE											
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
								25	50	75	10	20	30	40
0			Topsoil: Dark brown topsoil, moist, loose Sand: Brown fine sand, trace silt, moist, loose	1A										
237				1B	SS	70	3							
1				2	SS	90	7							
236				3	SS	80	7							
2			Wet	4	SS	60	12							
235			Saturated, compact	5	SS	65	12							
3				6	SS	60	13							
234														
4														
233														
5														
232			Borehole terminated at 5.0 mbgs											Groundwater and caving at 1.95 mbgs upon completion.
6														



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**Log of Borehole:**

BH103-19

Page 1 of 1

**Client:** iViva Homes**Project Name:** Geotechnical Investigation - Miller Drive**Project No.:** 9089-001**Contractor:** Walker Drilling**Method:** Hollow Stem Augers**Date Completed:** May 21, 2019**Location:** 156 Miller Drive, Barrie, ON**UTM:** 17T, 4914219.47, 600311.75**Elevation:** 237.78 mASL

SUBSURFACE PROFILE			SAMPLE												
Elevation (m)	Depth	Lithology	Description		Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
									25	50	75	10	20	30	40
0			Topsoil: Dark brown topsoil, moist, loose		1A	SS	30	3							
237			Fine Sand: Brown fine sand, trace silt, moist, loose		1B										
1					2	SS	90	3							
236					3	SS	10	7							
235					4	SS	75	9							
3					5	SS	40	12							
234					6	SS	90	21							
4															
233															
5															
232			Borehole terminated at 5.0 mbgs												
6															



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### **Log of Borehole:**

BH104-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914235.46, 600318.20

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 238.66 mASL



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## Log of Borehole:

BH105-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914259.67, 600252.72

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 238.71 mASL

SUBSURFACE PROFILE			SAMPLE												
Elevation (m)	Depth	Lithology	Description		Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
									25	50	75	10	20	30	40
0			Topsoil: Dark brown topsoil, moist, loose		1A										
238			Sand: Brown fine sand, trace silt, trace organics, moist, compact		1B	SS	60	2							
1					2	SS	50	5							
237			Sand: Brown to grey fine sand, trace silt, wet, loose		3A										
2			Compact, wet		3B	SS	70	8							
236			Fine Sand: Grey fine sand, trace silt, trace clay, moist, compact		4	SS	65	21							
3					5	SS	50	16							
235					6	SS	55	20							
4															
234															
5															
233			Borehole terminated at 5.0 mbgs												
6															



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## Log of Borehole:

BH106-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914296.41, 600235.82

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 237.53 mASL

SUBSURFACE PROFILE			SAMPLE												
Elevation (m)	Depth	Lithology	Description		Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
									25	50	75	10	20	30	40
0					1A										
237			Topsoil: Dark brown topsoil, moist, loose		1B	SS	60	11							
1			Sand: Brown sand, trace silt, moist, compact		2	SS	70	11							
236			Sand: Grey fine sand, trace silt, trace clay, saturated, compact		3	SS	100	3							
235			Loose		4	SS	100	17							
234			Compact		5	SS	60	18							
233					6	SS	90	21							
232			Borehole terminated at 5.0 mbgs												
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## Log of Borehole:

BH107-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914217.61, 600270.10

**Project No.:** 9089-001  
**Date Completed:** May 21, 2019  
**Elevation:** 237.12 mASL

SUBSURFACE PROFILE			SAMPLE								
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture	SPT (N)	Well Installation	Remarks
								25 50 75	10 20 30 40		
237	0		Topsoil: Dark brown topsoil, moist, loose Sand: Brown fine sand, trace silt, moist, compact	1A							
				1B	SS	40	4				
236	1		Sand: Brown to grey fine sand, trace silt, trace gravel, some cobbles, moist, compact	2	SS	70	11				
235	2		Wet	3	SS	60	27				
234	3		Fine Sand: Brown fine sand, trace silt, trace gravel, moist, compact	4	SS	70	21				
233	4		Saturated	5	SS	80	26				
232	5		Less gravel, grey	6	SS	70	18				
231	6		Borehole terminated at 5.0 mbgs								Groundwater first observed at 2.4 mbgs and caving at 0.3 mbgs upon completion.



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## Log of Borehole:

BH108-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914242.38, 600284.30

**Project No.:** 9089-001  
**Date Completed:** May 22, 2019  
**Elevation:** 238.47 mASL

SUBSURFACE PROFILE			SAMPLE												
Elevation (m)	Depth	Lithology	Description		Number	Type	% Recovery	SPT (N)	% Moisture		SPT (N)	Well Installation	Remarks		
									25	50	75	10	20	30	40
0			Topsoil: Dark brown topsoil, moist, loose		1A	SS	50	4							
238			Sand: Brown fine sand, trace silt, moist, loose		1B										
1			Coarser sand, compact		2	SS	65	5							
237					3	SS	10	9							
2					4	SS	50	20							
236					5	SS	65	19							
3			Saturated		6	SS	60	27							
235															
4															
234															
5															
233			Borehole terminated at 5.0 mbgs												
6															



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**Log of Borehole:**

BH109-19

Page 1 of 1

**Client:** iViva Homes**Project Name:** Geotechnical Investigation - Miller Drive**Project No.:** 9089-001**Contractor:** Walker Drilling**Method:** Hollow Stem Augers**Date Completed:** May 22, 2019**Location:** 156 Miller Drive, Barrie, ON**UTM:** 17T, 4914194.77, 600301.85**Elevation:** 236.18 mASL

SUBSURFACE PROFILE			SAMPLE								
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture	SPT (N)	Well Installation	Remarks
236	0		Topsoil: Dark brown topsoil, wet, loose	1A	SS	50	3				
				1B							
235	1		Sand: Brown fine sand, trace silt, wet, compact	2	SS	75	10				
234	2			3	SS	60	15				
233	3			4	SS	70	12				
232	4			5	SS	70	11				
231	5			6	SS	90	17				
230	6		Borehole terminated at 5.0 mbgs								Groundwater at 1.83 mbgs and caving at 0.6 mbgs upon completion



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**Log of Borehole:**

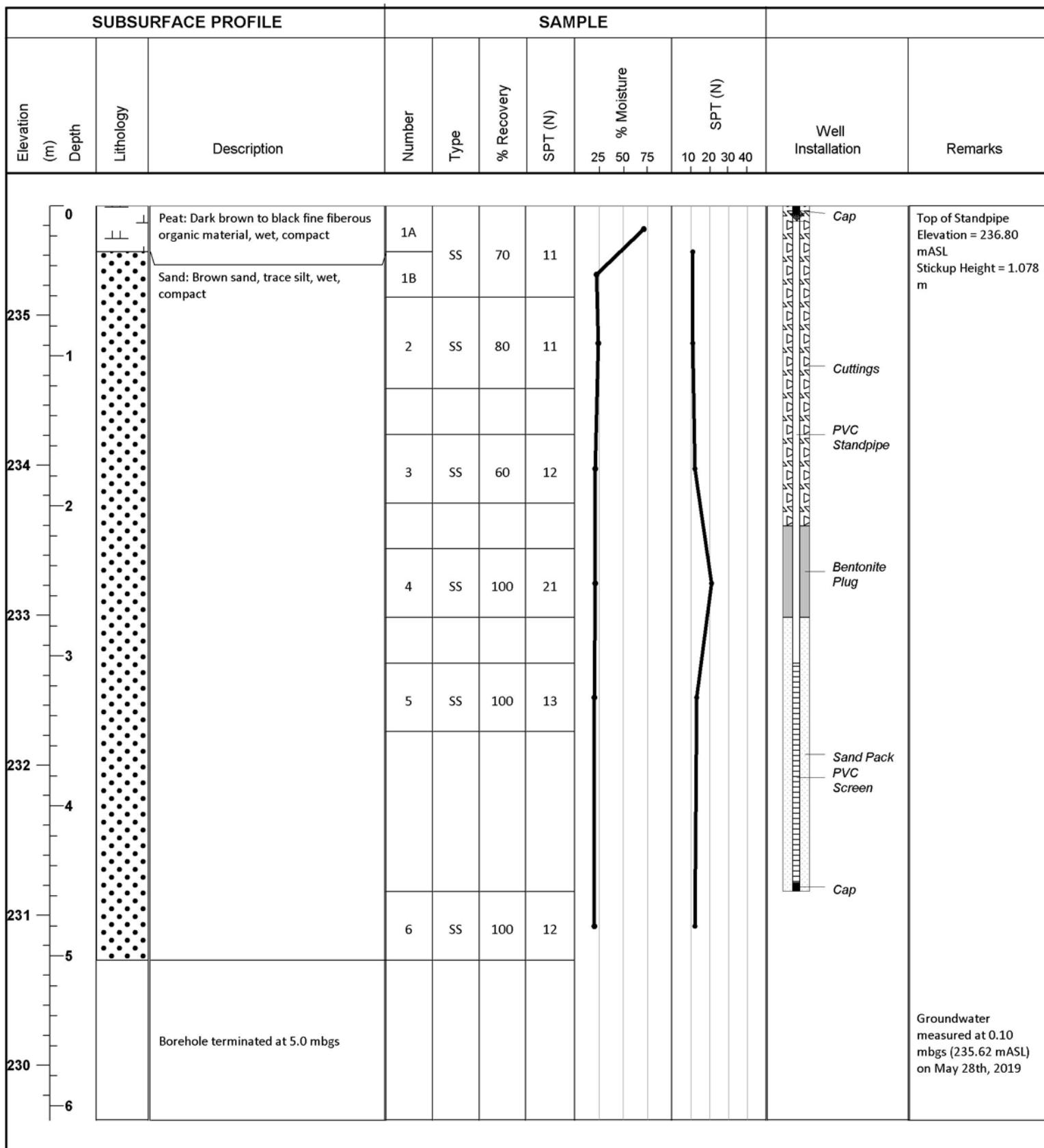
BH110-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914173.24, 600325.53

**Project No.:** 9089-001  
**Date Completed:** May 22, 2019  
**Elevation:** 235.73 mASL





**Peterborough**  
**Barrie**  
**Oshawa**  
**Kingston**  
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**www.cambium-inc.com**

## Log of Borehole:

BH111-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914245.99, 600330.94

**Project No.:** 9089-001  
**Date Completed:** May 22, 2019  
**Elevation:** 239.95 mASL

SUBSURFACE PROFILE			SAMPLE								
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture	SPT (N)	Well Installation	Remarks
							25 50 75	10 20 30 40			
0			Topsoil: Dark brown sandy topsoil, moist, loose	1A							
			Sand: Brown sand, trace silt, moist, loose, FILL	1B	SS	100	3				
239	1			2	SS	70	3				
238	2			3	SS	45	8				
237	3		Sand: Brown sand, trace silt, moist, loose	4	SS	0	6				
236	4		Compact	5	SS	60	12				
235	5			6	SS	65	19				
234	6		Borehole terminated at 5.0 mbgs								Groundwater and caving at 3.96 mbgs upon completion



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**Log of Borehole:**

BH112-19

Page 1 of 1

**Client:** iViva Homes**Project Name:** Geotechnical Investigation - Miller Drive**Project No.:** 9089-001**Contractor:** Walker Drilling**Method:** Hollow Stem Augers**Date Completed:** May 22, 2019**Location:** 156 Miller Drive, Barrie, ON**UTM:** 17T, 4914280.76, 600309.21**Elevation:** 240.12 mASL

SUBSURFACE PROFILE			SAMPLE											
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture			SPT (N)	Well Installation	Remarks	
								25	50	75	10	20	30	40
240	0		Topsoil: Dark brown topsoil, moist, loose Sand: Brown fine sand, trace silt, moist, very loose	1A										
239	1		Loose	1B	SS	50	2							
238	2		Very loose	2	SS	100	3							
237	3		Sandy Silt: Light brown sandy silt, some cobbles, saturated, compact	3	SS	75	5							
236	4		Silty Sand: Brown silty sand, saturated, compact	4	SS	40	2							
235	5		Borehole terminated at 5.0 mbgs	5	SS	50	11							
235	6			6	SS	65	15							
														Groundwater and caving at 3.66 mbgs upon completion



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**Log of Borehole:**

BH113-19

Page 1 of 1

**Client:** iViva Homes**Project Name:** Geotechnical Investigation - Miller Drive**Project No.:** 9089-001**Contractor:** Walker Drilling**Method:** Hollow Stem Augers**Date Completed:** May 22, 2019**Location:** 156 Miller Drive, Barrie, ON**UTM:** 17T, 4914285.09, 600287.07**Elevation:** 239.60 mASL

SUBSURFACE PROFILE			SAMPLE								
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N)	% Moisture	SPT (N)	Well Installation	Remarks
0			Topsoil: Dark brown topsoil, moist, loose	1A	SS	50	2				
239			Sand: Brown fine sand, trace silt, moist, very loose	1B							
1				2	SS	100	2				
238				3	SS	45	0				
2				4	SS	70	19				
237		Compact		5	SS	70	19				
3			Wet, some clay	6	SS	65	17				
236											
4											
235											
5											
234			Borehole terminated at 5.0 mbgs								Groundwater and caving at 3.66 mbgs upon completion
6											



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## Log of Borehole:

BH114-19

Page 1 of 1

**Client:** iViva Homes  
**Contractor:** Walker Drilling  
**Location:** 156 Miller Drive, Barrie, ON

**Project Name:** Geotechnical Investigation - Miller Drive  
**Method:** Hollow Stem Augers  
**UTM:** 17T, 4914285.62, 600321.57

**Project No.:** 9089-001  
**Date Completed:** May 22, 2019  
**Elevation:** 240.94 mASL

SUBSURFACE PROFILE			SAMPLE												
Elevation (m)	Depth	Lithology	Description		Number	Type	% Recovery	SPT (N)	25	50	75	SPT (N)	Well Installation	Remarks	
0			Topsoil: Dark brown topsoil, moist, loose		1A	SS	75	3						Cap	Top of Standpipe Elevation = 241.85 mASL Stickup Height = 0.907 m
240	1		Sand: Brown fine sand, trace silt, moist, loose		1B									Cuttings	GSA SS2: 0% Gravel 83% Sand 13% Silt 4% Clay
239	2	Compact			2	SS	100	4						PVC Standpipe	
238	3		Increased clay and silt content, wet		3	SS	90	11						Bentonite Plug	
237	4				4	SS	70	17						Sand Pack PVC Screen	
236	5		Borehole terminated at 5.0 mbgs		5	SS	90	11						Cap	Groundwater measured at 4.49 mbgs (236.45 mASL) on May 28th, 2019
235	6				6	SS	75	10							



Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

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## **Appendix C**

### **Borehole Logs**

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UTM 117z 600439E

Cadastral 4914254 The Ontario Water Resources Commission Act

Elev. 230800

## WATER WELL RECORD

WATER RESOURCES DIVISION	57	N.
NOV 9 1964		
ONTARIO WATER RESOURCES COMMISSION		

B  
4840Basin County or District Simcoe  
Con. # 711 Lot 23

Township, Village, Town or City

Date completed 15 SEPT 1964  
Address R.R. # 2 Barrie, Ont.

## Casing and Screen Record

Inside diameter of casing	4"
Total length of casing	87
Type of screen	JOHNSON S.S. SLET 6
Length of screen	4"
Depth to top of screen	87
Diameter of finished hole	4"

## Pumping Test

Static level	20'
Test-pumping rate	2 G.P.M.
Pumping level	66'
Duration of test pumping	1 1/2 DAYS.
Water clear or cloudy at end of test	CLEAR
Recommended pumping rate	2 G.P.M.
with pump setting of	86° feet below ground surface

## Well Log

## Overburden and Bedrock Record

Dirty Sqnd  
Grey Clay  
Silty Clay  
Grey Clay  
Fine Sqnd  
Grey Clay

From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
0'	23'		
23'	60'		
60'	64'		
64'	87'		
87'	91'		
91'	94'	87	FRESH

For what purpose(s) is the water to be used?

House

Is well on upland, in valley, or on hillside?

Drilling or Boring Firm Scott Wells

Address R.R. # 1 Oro Station  
Ont.

Licence Number 1240

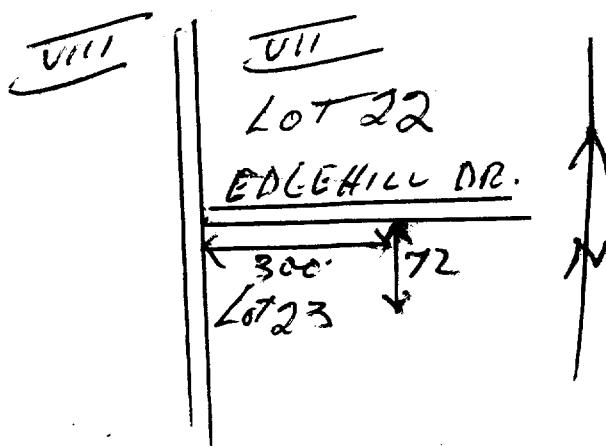
Name of Driller or Borer G.R. Scott

Address R.R. # 1 Oro Station Ont.

Date NOV 4 1964  
Signature of Licensed Drilling or Boring Contractor

## Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



UTM 17 600469 E  
9 R 4914076 N  
Elev. 9 R 10906  
Basin 221



RECEIVED

57 N.

4842

12N - 6 1954

**GEOLOGICAL BRANCH  
DEPARTMENT of LINES**

## The Well Drillers Act

## Department of Mines, Province of Ontario

## Water Well Record

### **Pipe and Casing Record**

## Pumping Test

Casing diameter(s) .....	2"	Date Sept. 20. 1953 .....	
Length(s) of casing(s) .....	54'	Static level .....	3'
Type of screen .....	Johnson Red Head	Pumping level .....	15'
Length of screen .....	48"	Pumping rate .....	300 gpm per 100 ft.
Distance from top of screen to ground level .....	54'	Duration of test .....	1 hr.
Is well a gravel-wall type? .....	—	Distance from cylinder or bowls to ground level .....	—

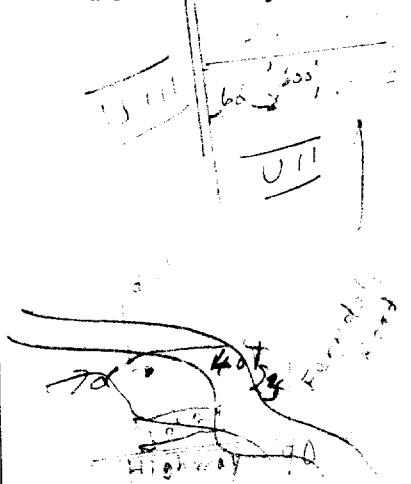
## Water Record

Kind (fresh or mineral) . . . . .	Depth(s) to Water Horizon(s)	Kind of Water	No. of Feet Water Rises
Quality (hard, soft, contains iron, sulphur, etc) . . . . .			
Appearance (clear, cloudy, coloured) . . . . .			
For what purpose(s) is the water to be used? . . . . .			
How far is well from possible source of contamination? . . . . .			
What is the source of contamination? . . . . .			
Enclose a copy of any mineral analysis that has been made of water . . . . .			

### Well Log

### Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



Situation: Is well on upland, in valley, or on hillside? On hillside  
Drilling Firm John H. Johnson  
Address 1000 N. Main St., Salt Lake City, Utah  
Name of Driller John H. Johnson Address Same as above  
Date July 28, 1953 Licence Number 5302

**FORM 5**



UTM 17 z 600404 E

57 N° 4863

S.R. 4914016 N The Ontario Water Resources Commission Act

Elev. S.R. 10800

# WATER WELL RECORD

Basin or District 58MCOE

Township, Village, Town or City YES PIA

Con. 8

Lot E.H. Lot 23

Date completed

12

4

/66

(day)

month

year)

Owner

ress

RR 2 BARRIE

(print in block letters)

## Casing and Screen Record

Inside diameter of casing 4"  
 Total length of casing 15' 9"  
 Type of screen Johnston S.S. no 10 mesh  
 Length of screen 3'  
 Depth to top of screen 15' 9"  
 Diameter of finished hole 4"

## Pumping Test

Static level 30'  
 Test-pumping rate 6 G.P.M.  
 Pumping level 90'  
 Duration of test pumping 2 hrs  
 Water clear or cloudy at end of test clear  
 Recommended pumping rate 5' G.P.M.  
 with pump setting of 120' feet below ground surface

## Well Log

### Overburden and Bedrock Record

fine sand  
 sandy clay  
 runny sand & clay.  
 hard pan.  
 fine dirty sand ridges of clay gravel  
 hard pan.  
 coarse water bearing sand

	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
	0	20		
	20	125		
	125	140	143	
	140	143	143	water
	143	158	158	fresh
	158	159	159	
	159	162	162	

For what purpose(s) is the water to be used? Household

Is well on upland, in valley, or on hillside? upland

Drilling or Boring Firm Ken Wright &amp; Son

Address Durham

Licence Number 2077

Name of Driller or Borer Larry Wright

Address Durham

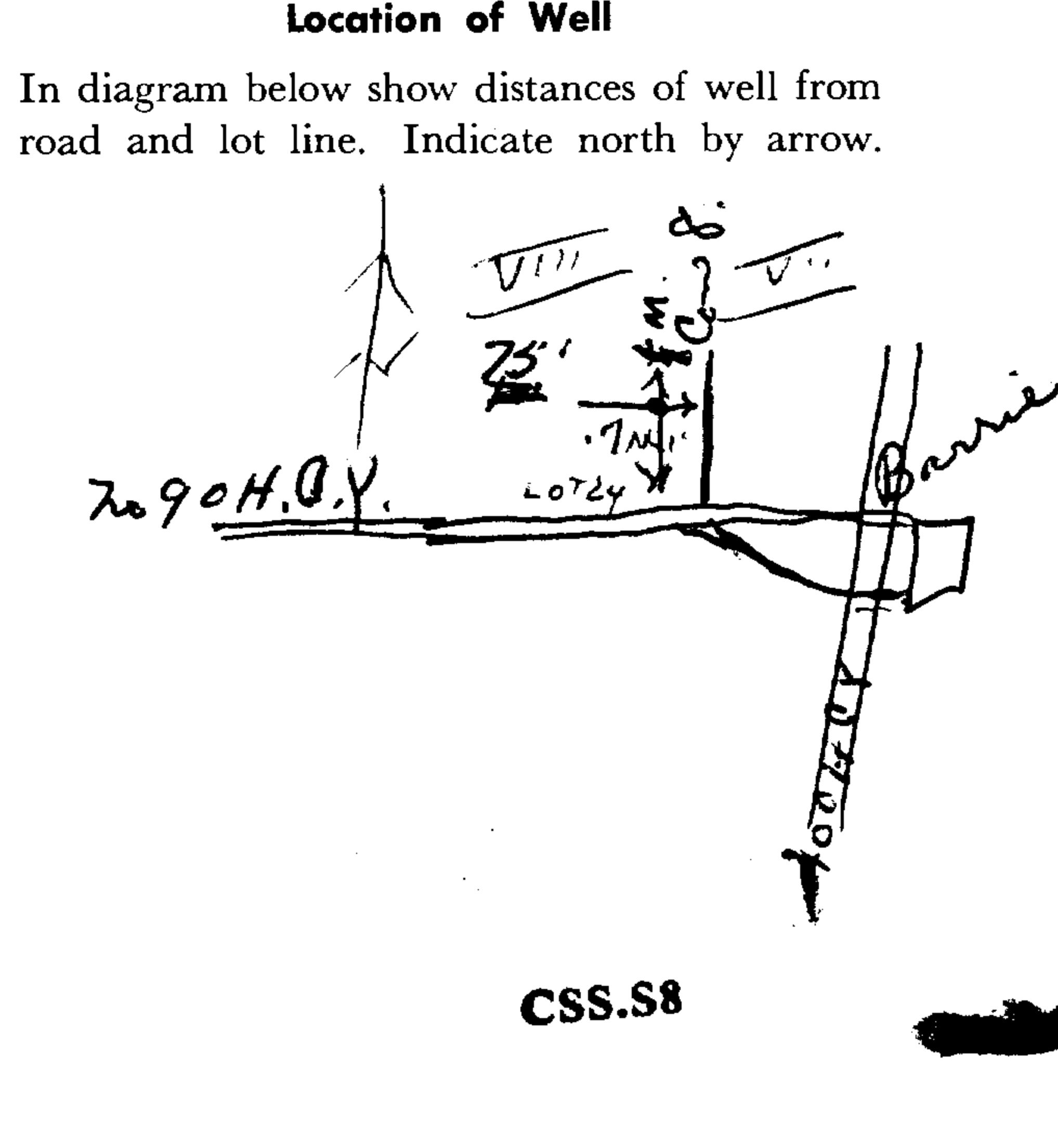
Date 12/4/66

Signature of Licensed Drilling or Boring Contractor

Form 7 15M-60-4138

CSS.S8

OWRC COPY





57 N° 4864

UTM 17 z 6003541

S.R. 4913944 The Ontario Water Resources Commission Act

Elev. S.R. 07751

# WATER WELL RECORD

Basin or District Simeo

Con. VIII Lot 23

Owner Morris Cast Construction Ltd. Address BARRIE

(print in block letters)

Township, Village, Town or City Vespa

MARCH

1967

Casing and Screen Record	
Inside diameter of casing	6"
Total length of casing	214'
Type of screen	Johnson's S.S. (32 ft 16 1/2 ft 18)
Length of screen	6'
Depth to top of screen	214'
Diameter of finished hole	6"

Pumping Test	
Static level	34 FT
Test-pumping rate	30 G.P.M.
Pumping level	90'
Duration of test pumping	3 hrs
Water clear or cloudy at end of test	clear
Recommended pumping rate	30 G.P.M.
with pump setting of	90' feet below ground surface

Well Log		Water Record		
Overburden and Bedrock Record		From ft.	To ft.	Depth(s) at which water(s) found
Top soil		0	1	
Yellow sand		1	56	56
Very fine grey sand		56	163	24 - 40
Silt with coarse sand		163	187	
Hard, clay, sand, silt		187	208	
Clay, silt, sand		208	215	
Med. sand (some silt)		215	222	214-222
Sandy clay		222		Fresh

For what purpose(s) is the water to be used? Domestic

Subdivision

Is well on upland, in valley, or on hillside? Low Land

Drilling or Boring Firm HENRY HAMMERS

Address RR #3 BARRIE ONT

Licence Number 2216

Name of Driller or Borer A. HAMMERS

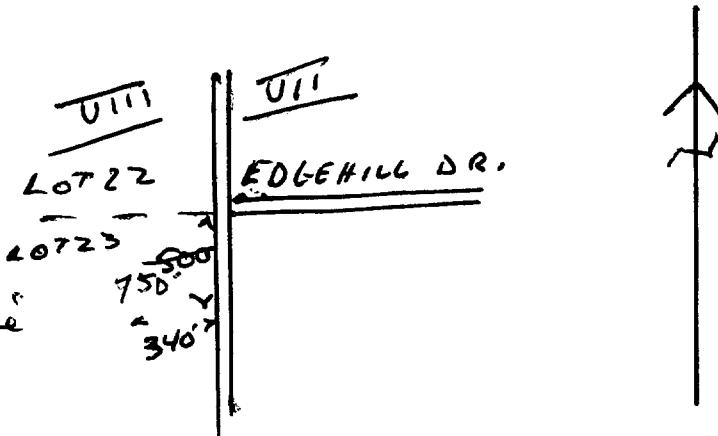
Address 90 Cumberland St. BARRIE

Date APR. 6/67

(Signature of Licensed Drilling or Boring Contractor)

## Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



RECEIVED

JUL 2 1952

57

No

4865

UTM 217z 6003511E.  
 9R 4914096N  
 Elev. 0800  
 Basin 1211

GEOLOGICAL BRANCH  
DEPARTMENT OF MINESThe Well Drillers Act  
Department of Mines, Province of Ontario

## Water Well Record

p, Village, Town or City. *Nepean*.....

Town or City).....

s. *Barrie. R.R.H.2*.....Date Completed. *May 20, 1952* Cost of Well (excluding pump).....

## Pipe and Casing Record

## Pumping Test

Casing diameter(s) .....	2"	Date .....	<i>May 20, 1952</i>
Length(s) of casing(s) .....	20'	Static level .....	<i>4'</i>
Type of screen .....	<i>Johnson</i>	Pumping level .....	<i>4'</i>
Length of screen .....	48	Pumping rate .....	<i>300 gal/min</i>
Distance from top of screen to ground level .....	20	Duration of test .....	<i>3 hrs</i>
Is well a gravel-wall type? .....	<i>No</i>	Distance from cylinder or bowls to ground level .....	<i>15 ft. pump</i>

## Water Record

Kind (fresh or mineral) .....	<i>Fresh</i>	Depth(s) to Water Horizon(s)	Kind of Water	No. of Feet Water Rises
Quality (hard, soft, contains iron, sulphur, etc.) .....	<i>Hard</i>	<i>4'</i>	<i>Hard</i>	<i>16'</i>
Appearance (clear, cloudy, coloured) .....	<i>Clear</i>	<i>20</i>		
For what purpose(s) is the water to be used? .....	<i>House and garage</i>			
How far is well from possible source of contamination? .....				
What is the source of contamination? .....	<i>None</i>			
Enclose a copy of any mineral analysis that has been made of water.....				

## Well Log

## Overburden and Bedrock Record

From

To

0 ft.

....ft.

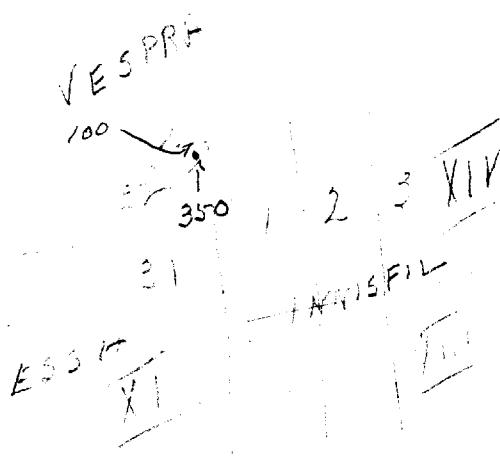
0 - 10

10 - 18

18 - 24

## Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.

Situation: Is well on upland, in valley, or on hillside? *Valley*.....Drilling Firm. *MacLean Construction*.....Address. *Barrie. R.R.H.2*.....Name of Driller. *MacLean Construction*..... Address. *Barrie. R.R.H.2*.....Date. *July 3, 1952*..... Licence Number. *532*.....

Signature of Licensee



## WELL RECORD

310/SE

Water management in On.

USES PROVIDED

CORRECT BOX WHERE APPLICABLE

11

12

MUNICIP.

CON.

COUNTY OR DISTRICT: SIMCOE  
 OWNER (SURNAME FIRST): VESPA  
 ADDRESS: NORTH BAY

5708000

57016

CON.















## WATER WELL RECORD

31 DS 60

1. PRINT ONLY IN SPACES PROVIDED  
2. CHECK  CORRECT BOX WHERE APPLICABLE

11

713519

MUNICIP.

CON.

57016

CON.

07

COUNTY OR DISTRICT

TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

CON., BLOCK, TRACT, SURVEY ETC.

LOT 25-27

Sudan

Vespera

7

023

14250

5 ELEVATION

5

0800

22

10 12 17 18 24 26

15 25

30 31

11 13 14 16 18 20 22 24 26

11 13 14 16 18 20 22 24 26

1 3 5 7 9 11 13 15 17 19 21 23 25 27

## LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Grey	Sand	Clay	Grey Sand & Clay	15	45
Grey	Clay		Grey clay	15	83
Blue	Clay		Blue clay	83	111
Brown	Sand	Clay	Brown sand & clay	111	115
Grey	Clay		Grey clay	115	123
Brown	Sand		Brown Sand	123	124
Grey	Clay		Grey clay	124	131
Brown	Sand		Brown Med Sand	131	134

31 bolt50128105

10083205

0111305

011562805

0123205

0124605

32 01311205

01311209

32

43

54

65

75

41 WATER RECORD

WATER FOUND AT - FEET	KIND OF WATER
10-13	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIA. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET
10-11	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	12	13-16
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	19	20-23
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	26	27-30

SCREEN SIZE(S) OF OPENING 31-33 DIAMETER 34-36 LENGTH 39-40

SCREEN MATERIAL AND TYPE	008 #8	05000 03	FEET
DEPTH TO TOP OF SCREEN	41-44	80	FEET
Johnson Stank 0/31			

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET	MATERIAL AND TYPE	CEMENT GROUT, LEAD PACKER, ETC.
FROM	TO	
10-13	14-17	
18-21	22-25	
26-29	30-33	80

71 PUMPING TEST

PUMPING TEST METHOD	10 PUMPING RATE	11-14 DURATION OF PUMPING
1 <input type="checkbox"/> PUMP 2 <input checked="" type="checkbox"/> BAILER	0004 GPM	15-16 HOURS 17-18 MINS
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING
023 FEET	22-24 15 MINUTES 26-28 30 MINUTES 29-31 45 MINUTES 32-34 60 MINUTES 35-37 FEET	1 <input type="checkbox"/> PUMPING 2 <input type="checkbox"/> RECOVERY
IF FLOWING, GIVE RATE	38-41 PUMP INTAKE SET AT	WATER AT END OF TEST 42 FEET
GPM	FEET	1 <input type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE 46-49 GPM
□ SHALLOW <input checked="" type="checkbox"/> DEEP	FEET	FEET
50-53	—	— GPM./FT. SPECIFIC CAPACITY

54 FINAL STATUS OF WELL

1 <input type="checkbox"/> WATER SUPPLY	5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
2 <input type="checkbox"/> OBSERVATION WELL	6 <input type="checkbox"/> ABANDONED, POOR QUALITY
3 <input type="checkbox"/> TEST HOLE	7 <input type="checkbox"/> UNFINISHED
4 <input type="checkbox"/> RECHARGE WELL	

55-56 WATER USE

1 <input type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
2 <input type="checkbox"/> STOCK	6 <input type="checkbox"/> MUNICIPAL
3 <input type="checkbox"/> IRRIGATION	7 <input type="checkbox"/> PUBLIC SUPPLY
4 <input type="checkbox"/> INDUSTRIAL	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
10 <input type="checkbox"/> OTHER	9 <input type="checkbox"/> NOT USED

57 METHOD OF DRILLING

1 <input type="checkbox"/> CABLE TOOL	6 <input type="checkbox"/> BORING
2 <input type="checkbox"/> ROTARY (CONVENTIONAL)	7 <input type="checkbox"/> DIAMOND
3 <input type="checkbox"/> ROTARY (REVERSE)	8 <input type="checkbox"/> JETTING
4 <input type="checkbox"/> ROTARY (AIR)	9 <input type="checkbox"/> DRIVING
5 <input type="checkbox"/> AIR PERCUSSION	

CONTRACTOR NAME OF WELL CONTRACTOR

MARCHILDON DRILLING 3660

ADDRESS Peter Marchildon

NAME OF DRILLER OR BORER RPT# 2 Shanty Bay 3660

SIGNATURE OF CONTRACTOR

LICENCE NUMBER

SUBMISSION DATE DAY 16 MO. 8 YR 76

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE INDICATE NORTH BY ARROW.

edgehill Drive

150'

300'

175'

N

VIII

VII

YY

DRILLERS REMARKS:

yellow house

PLANE 677

OFFICE USE ONLY

DATA SOURCE	58 CONTRACTOR	59-62 DATE RECEIVED	63-68
	3660	150976	80
DATE OF INSPECTION	INSPECTOR		
REMARKS:	yellow house		



1.0/171

# **WATER WELL RECORD**

3105E

Ontario		1. PRINT ONLY IN SPACES PROVIDED		11		5714853		MUNICIP.		CON.		07	
		2. CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE						57016		CON		22 23 24	
COUNTY OR DISTRICT		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		3		8		CON. BLOCK, TRACT, SURVEY, ETC.		LOT		25-27	
SIMCOE		Vespra		✓11						022			
OWNER (SURNAME FIRST)		ADDRESS						DATE COMPLETED		48-53			
BARRIE PUC		Rayfield Street Barrie						DAY 25		MO. 10		YR 77	
U. ZONE		EASTING		NORTHING		RC		ELEVATION		BASIN CODE		II III IV	
11		6003.00		69143.00		5		0825 5		23		31 47	

## LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Brown	Sand	clay	layers	0	25
Grey	clay	& some stones	soft	25	76
Grey	clay	soft & fine sand	layers	76	96
Grey	clay sandy	& some gravel	hard packed	96	104
Grey	clay	sandy	soft	104	116
Grey	clay sandy	& some stones	hard packed	116	126
Fine	Sand	& fine gravel	pebbled.	126	148
Grey	clay		hard	148	184
Grey	fine sand		pebbled	184	260
Grey	clay sandy	& fine sand	layers	260	273
Grey	fine sand	& clay?	pebbled	273	293
Grey	clay		hard	293	355

31 *Sheld* 355 362  
32

<b>PUMPING TEST</b>	<b>PUMPING TEST METHOD</b>		10	<b>PUMPING RATE</b>	11-14	<b>DURATION OF PUMPING</b>	
	<input type="checkbox"/> PUMP <input type="checkbox"/> BAILER			GPM		15-16 HOURS	17-18 MIN
	<b>STATIC LEVEL</b>	<b>WATER LEVEL END OF PUMPING</b>	25	<b>WATER LEVELS DURING PUMPING</b>		<input type="checkbox"/> PUMPING <input type="checkbox"/> RECOVERY	
	19-21 FEET	22-24 FEET	15 MINUTES 26-28 FEET	10 MINUTES 29-31 FEET	45 MINUTES 32-34 FEET	60 MINUTES 35-37 FEET	
	IF FLOWING, GIVE RATE		36-41	<b>PUMP INTAKE SET AT</b>		<b>WATER AT END OF TEST</b>	
			GPM		FEET	<input type="checkbox"/> CLEAR	<input type="checkbox"/> CLOUDY
<b>RECOMMENDED PUMP TYPE</b>		<b>RECOMMENDED PUMP SETTING</b>	43-45	<b>RECOMMENDED PUMPING RATE</b>	46-48		
<input type="checkbox"/> SHALLOW <input type="checkbox"/> DEEP			FEET		GPM		
50-53		<b>GPM./FT. SPECIFIC CAPACITY</b>					

<b>FINAL STATUS OF WELL</b>	54	1 <input type="checkbox"/> WATER SUPPLY 2 <input type="checkbox"/> OBSERVATION WELL 3 <input type="checkbox"/> TEST HOLE 4 <input type="checkbox"/> RECHARGE WELL	5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY 6 <input type="checkbox"/> ABANDONED, POOR QUALITY 7 <input type="checkbox"/> UNFINISHED	
	55-56	1 <input type="checkbox"/> DOMESTIC 2 <input type="checkbox"/> STOCK 3 <input type="checkbox"/> IRRIGATION 4 <input type="checkbox"/> INDUSTRIAL <input type="checkbox"/> OTHER	5 <input type="checkbox"/> COMMERCIAL 6 <input type="checkbox"/> MUNICIPAL 7 <input type="checkbox"/> PUBLIC SUPPLY 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING 9 <input type="checkbox"/> NOT USED	
	<b>WATER USE</b>	57	1 <input type="checkbox"/> CABLE TOOL 2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 3 <input type="checkbox"/> ROTARY (REVERSE) 4 <input type="checkbox"/> ROTARY (AIR) 5 <input type="checkbox"/> AIR PERCUSSION	
			6 <input type="checkbox"/> BORING 7 <input type="checkbox"/> DIAMOND 8 <input type="checkbox"/> JETTING 9 <input type="checkbox"/> DRIVING	
	<b>METHOD OF DRILLING</b>			

<b>CONTRACTOR</b>	<b>NAME OF WELL CONTRACTOR</b>	<b>LICENCE NUMBER</b>	
	<i>International Water Supply Inc.</i>	<i>2801</i>	
<b>ADDRESS</b>	<i>P.O. Box 310 Barrie Ont</i>		
<b>NAME OF DRILLER OR BORER</b>	<b>LICENCE NUMBER</b>		
<b>SIGNATURE OF CONTRACTOR</b>	<b>SUBMISSION DATE</b>		
	<b>DAY</b> <i>5</i>	<b>NO.</b> <i>12</i>	<b>YR</b> <i>7</i>

**LOCATION OF WELL**

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

The diagram shows a vertical line representing a road or lot line. To the left of this line is a vertical line labeled 'VIII' at the top and 'Angus' with an arrow pointing left at the bottom. To the right of the vertical line is a horizontal line labeled 'VII' at the top and 'Edgchill Drive' at the bottom. Between these lines is a horizontal line labeled 'Lot 22' above and 'Lot 23' below. A diagonal line labeled 'Hwy 90' slopes upwards from left to right. A diagonal line labeled 'Barrie' slopes upwards from right to left. The distance from the vertical line to the 'Hwy 90' line is labeled '661' with a small arrow. The distance from the vertical line to the 'Barrie' line is labeled '1381' with a small arrow.

<b>OFFICE USE ONLY</b>	DATA SOURCE	58	CONTRACTOR	59-62	DATE RECEIVED	63-68	80
	DATE OF INSPECTION		INSPECTOR			040178	
REMARKS:	<i>Conley 177779</i>					P	<i>321</i>
						WI	



## Ministry of the Environment

Measurements recorded in:  Metric  Imperial

**Well Tag No. (Place Sticker and/or Print Below)**

A 063865

## Well Record

## **Regulation 903 Ontario Water Resources Act**

Page \_\_\_\_\_ of \_\_\_\_\_

Annular Space			
Depth Set at (m/t)		Type of Sealant Used (Material and Type)	Volume Placed (m <sup>3</sup> /ft <sup>3</sup> )
From	To		
0	109'	QUIK - GROUT	21 CU FT
	106'	K - PACKER	

After test of well yield, water was: <input checked="" type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, specify _____	Draw Down		Recovery	
	Time (min)	Water Level (m/ft)	Time (min)	Water Level (m/ft)
If pumping discontinued, give reason:	Static Level	32'		
Pump intake set at (m/ft)	1		1	
	2		2	
Pumping rate (l/min / GPM)	3		3	
10 GPM	4		4	<del>NR</del>
Duration of pumping	5		5	
8 hrs + _____ min	10		10	
Final water level end of pumping (m/ft)	15		15	
90'	20		20	
If flowing give rate (l/min / GPM)	25		25	
Recommended pump depth (m/ft)	30		30	
100'	40		40	
Recommended pump rate (l/min / GPM)	50		50	
10 GPM	60		60	
Well production (l/min / GPM)				
Disinfected?				
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No				

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft)		Insufficient Supply <input type="checkbox"/> Abandoned, Poor Water Quality <input type="checkbox"/> Abandoned, other, specify <hr/> <input type="checkbox"/> Other, specify <hr/>
			From	To	
5 1/2"	STEEL	#14	109	114	

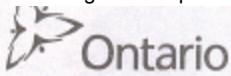
Please provide a map below following instructions on the back.

Water Details		Hole Diameter	
Water found at Depth <b>109</b> (m/ft)	<input checked="" type="checkbox"/> Gas	Kind of Water: <input checked="" type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____	Depth (m/ft) From _____ To _____ Diameter (cm/in) _____
Water found at Depth (m/ft)	<input type="checkbox"/> Gas	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____	_____
Water found at Depth (m/ft)	<input type="checkbox"/> Gas	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____	_____

**Comments:**

Well Contractor and Well Technician Information	
Business Name of Well Contractor	Well Contractor's Licence No.
<b>DRURY WELL DRILLING LTD</b>	1851
Business Address (Street Number/Name)	Municipality
RR#1	<b>BARRIE</b>

Well owner's information package delivered	Date Package Delivered	20070824
	Date Work Completed	20070824
<input checked="" type="checkbox"/> Yes	APPROVED	
<input type="checkbox"/> No	RECEIVED	



## Well Owner's Information

First Name	Last Name / Organization	E-mail Address			<input type="checkbox"/> Well Constructed by Well Owner
CITY OF BARRIE		cityinfo@barrie.ca			
Mailing Address (Street Number/Name) 70 COLLIER ST., BOX 400		Municipality	Province	Postal Code	Telephone No. (inc. area code)
		BARRIE	ON	L4M4T5	7057264242

## Well Location

Address of Well Location (Street Number/Name) MILLER DRIVE		Township	Lot	Concession
County/District/Municipality SIMCOE		WESPEA	23	8
UTM Coordinates	Zone	Easting	Northing	Municipal Plan and Sublot Number
NAD 83 176003784914162				Other

## Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	To
	DECOMMISSION	MILLER DRIVE TEST WELL			
		MOE 57-4864			
		150mm	SILICA GRAVEL	67.6	64.0
			BENTONITE GROUT	64.0	2.5m
			EXCAVATE AND CUT OFF 2m BGL		
			BENTONITE CHIPS	2.5	2.0
			NATIVE MATERIAL	2.0	0

## Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)

## Method of Construction

## Well Use

<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Diamond	<input type="checkbox"/> Public	<input type="checkbox"/> Commercial	<input type="checkbox"/> Not used
<input type="checkbox"/> Rotary (Conventional)	<input type="checkbox"/> Jetting	<input type="checkbox"/> Domestic	<input type="checkbox"/> Municipal	<input type="checkbox"/> Dewatering
<input type="checkbox"/> Rotary (Reverse)	<input type="checkbox"/> Driving	<input type="checkbox"/> Livestock	<input type="checkbox"/> Test Hole	<input type="checkbox"/> Monitoring
<input type="checkbox"/> Boring	<input type="checkbox"/> Digging	<input type="checkbox"/> Irrigation	<input type="checkbox"/> Cooling & Air Conditioning	
<input type="checkbox"/> Air percussion		<input type="checkbox"/> Industrial		
<input type="checkbox"/> Other, specify		<input type="checkbox"/> Other, specify		

## Construction Record - Casing

## Status of Well

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft)	From	To	<input type="checkbox"/> Water Supply	<input type="checkbox"/> Replacement Well
						<input type="checkbox"/> Test Hole	<input type="checkbox"/> Recharge Well
						<input type="checkbox"/> Dewatering Well	<input type="checkbox"/> Observation and/or Monitoring Hole
						<input type="checkbox"/> Alteration (Construction)	<input type="checkbox"/> Abandoned, Insufficient Supply
						<input type="checkbox"/> Abandoned, Poor Water Quality	<input type="checkbox"/> Abandoned, other, specify
						<input type="checkbox"/> Other, specify	

## Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft)	From	To

## Water Details

## Hole Diameter

Water found at Depth (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested	<input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Depth (m/ft)	From	To	Depth (m/ft)	From	Diameter (cm/in)

## Well Contractor and Well Technician Information

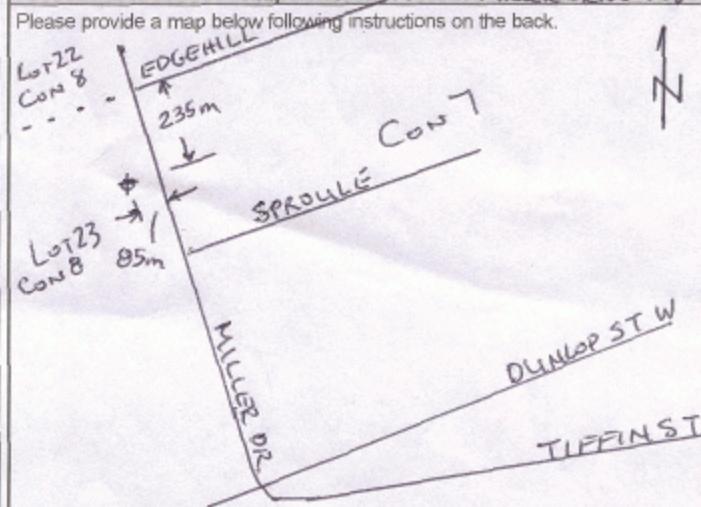
Business Name of Well Contractor	Well Contractor's Licence No.
INTERNATIONAL WATER SUPPLY LTD	2 8 0 1
Business Address (Street Number/Name) 342 BAYVIEW DRIVE	Municipality BARRIE

Province	Postal Code	Business E-mail Address
ON	LAN4Y8	IWS@IWS.CA
Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)	
7057330111	MARCHILDON, PETER	
Well Technician's Licence No.	Signature of Technician and/or Contractor	
0364	Terry Brown	
20100614		

## Results of Well Yield Testing

After test of well yield, water was:		Draw Down	Recovery
Time (min)	Water Level (m/ft)	Time (min)	Water Level (m/ft)
Static Level			
1		1	
2		2	
3		3	
4		4	
5		5	
10		10	
15		15	
20		20	
25		25	
30		30	
40		40	
50		50	
60		60	

## Map of Well Location MILLER DRIVE TW



Comments:

Well owner's information package delivered	Date Package Delivered	Ministry Use Only
<input type="checkbox"/> Yes	Y Y Y Y M M D D	Audit No.
<input type="checkbox"/> No	Date Work Completed	z115265
	20100531	Received
		JUN 17 2010



Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

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**Appendix D**  
**Grain Size Analysis Data**

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## Grain Size Distribution Chart

Project Number: 9089-001

Client: iViva Homes

Project Name: Miller Drive Site

Sample Date: May 21, 2019

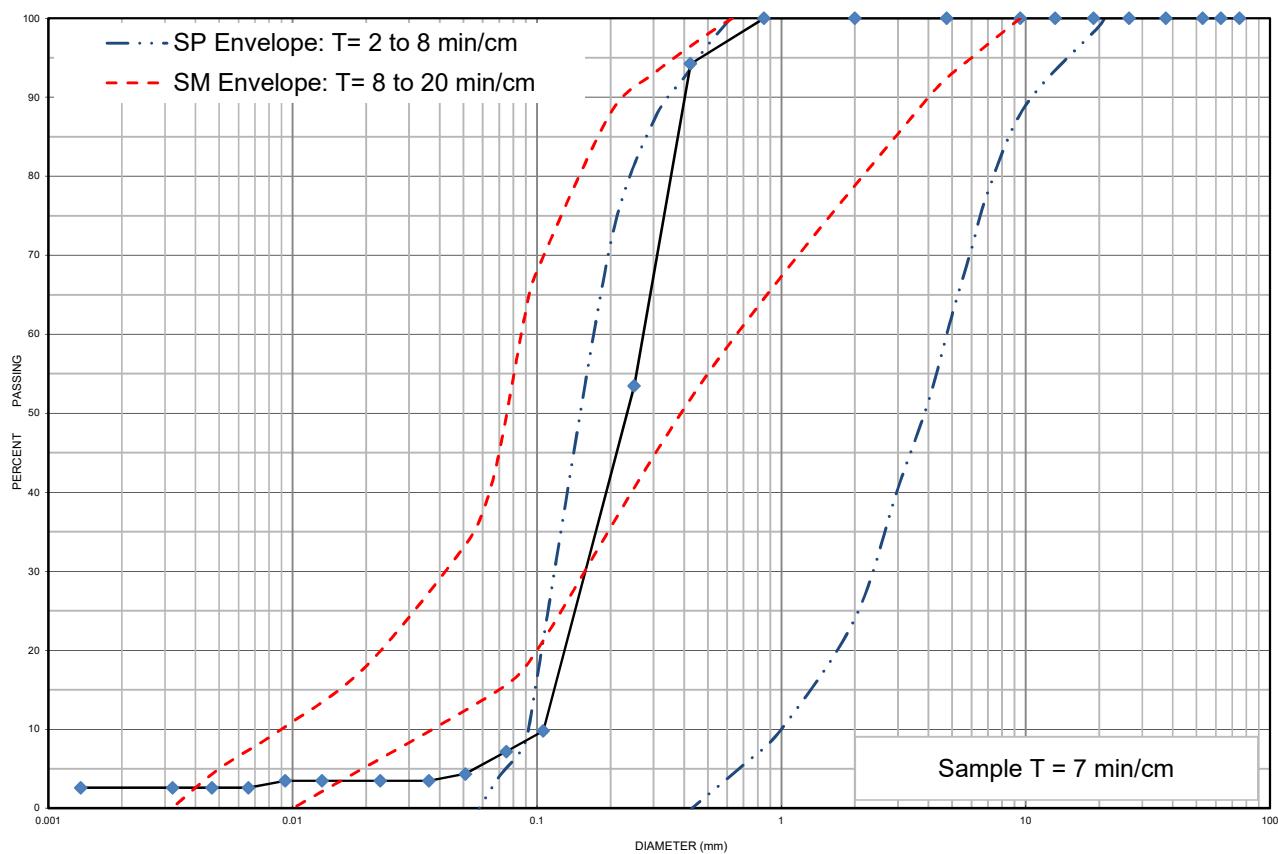
Sampled By: Jacob Bell - Cambium Inc.

Location: BH 101-19 SS 3

Depth: 1.5 m to 2 m

Lab Sample No: S-19-0334

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM		FINE			MEDIUM			COARSE			BOULDERS	
CLAY	SILT	SAND			GRAVEL							

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 101-19	SS 3	1.5 m to 2 m	0	93	7		22.4
Description	Classification	D <sub>60</sub>	D <sub>30</sub>	D <sub>10</sub>	C <sub>u</sub>	C <sub>c</sub>	
Sand trace Silt trace Clay	SP	0.270	0.1600	0.1	2.45	0.86	

Issued By: Jacob Bell  
(Senior Project Manager)

Date Issued: June 29, 2021



## Grain Size Distribution Chart

Project Number: 9089-001

Client: iViva Homes

Project Name: Miller Drive Site

Sample Date: May 21, 2019

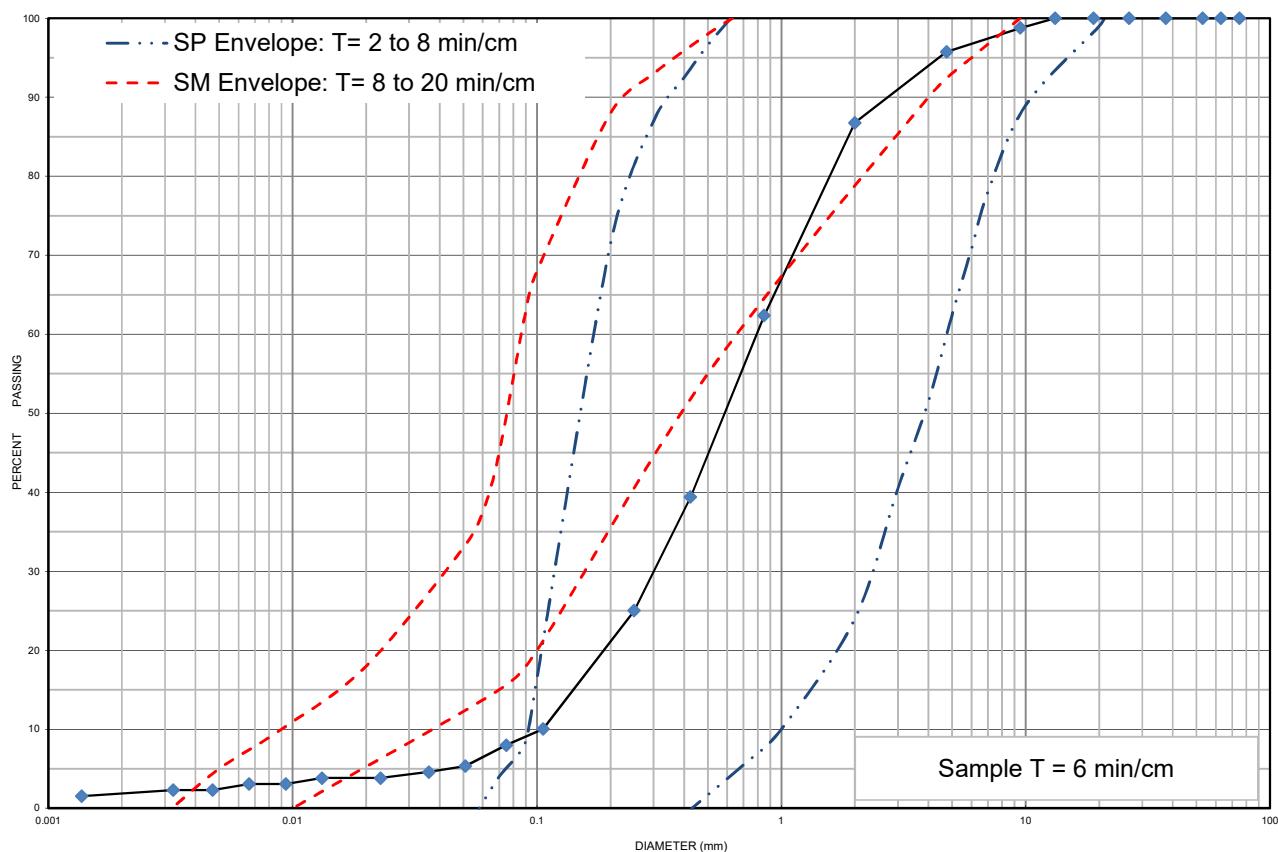
Sampled By: Jacob Bell - Cambium Inc.

Location: BH 108-19 SS 4

Depth: 2.3 m to 2.7 m

Lab Sample No: S-19-0335

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM		FINE			MEDIUM			COARSE			BOULDERS	
CLAY	SILT	SAND			GRAVEL							

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture		
BH 108-19	SS 4	2.3 m to 2.7 m	4	88	8		10.7		
			<b>Description</b>	<b>Classification</b>	<b>D<sub>60</sub></b>	<b>D<sub>30</sub></b>	<b>D<sub>10</sub></b>		
			Sand trace Silt trace Gravel trace Clay	SP	0.790	0.3000	0.1	C <sub>u</sub>	C <sub>c</sub>
								7.18	1.04

Issued By: \_\_\_\_\_  
(Senior Project Manager)

Date Issued: \_\_\_\_\_

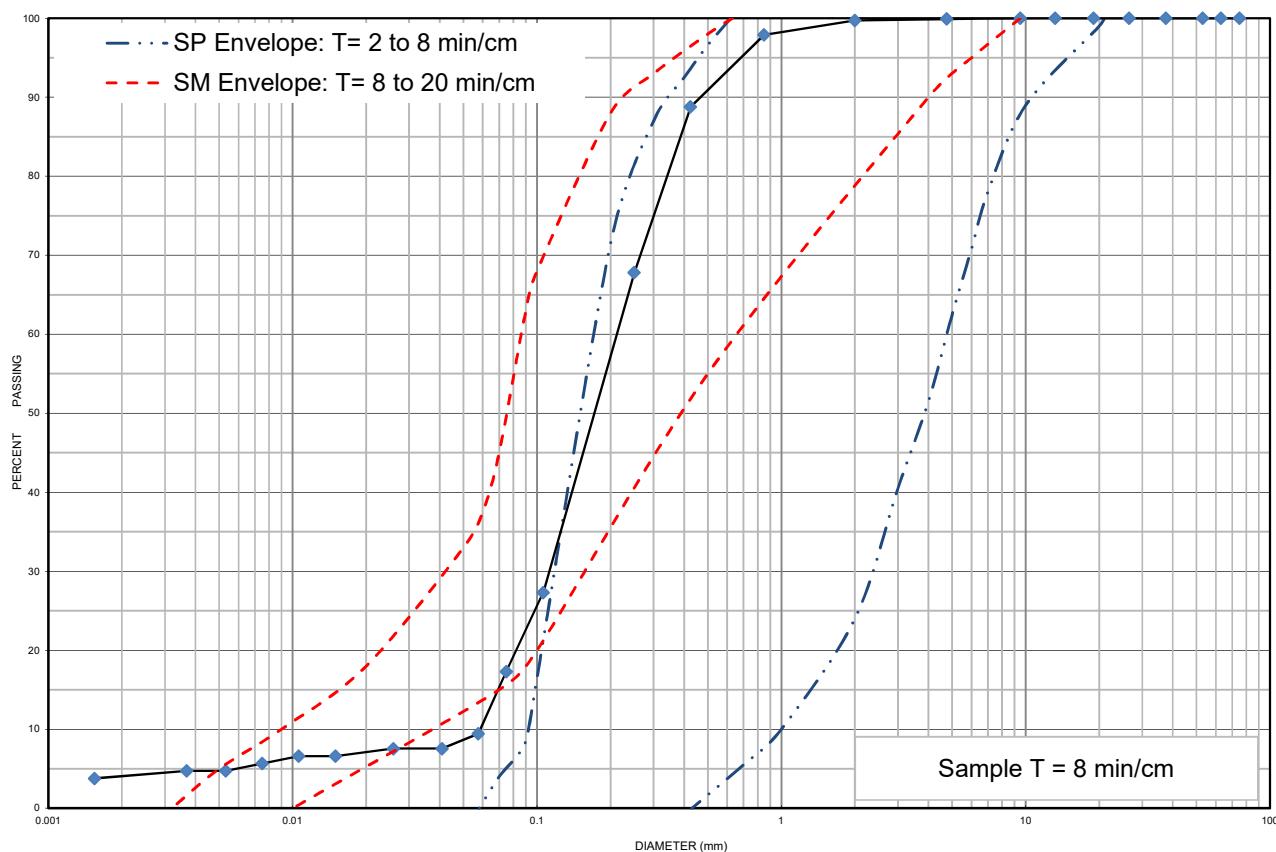
June 29, 2021



## Grain Size Distribution Chart

**Project Number:** 9089-001      **Client:** iViva Homes  
**Project Name:** Miller Drive Site  
**Sample Date:** May 21, 2019      **Sampled By:** Jacob Bell - Cambium Inc.  
**Location:** BH 114-19 SS 2      **Depth:** 0.6 m to 1.2 m      **Lab Sample No:** S-19-0336

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
		SAND	GRAVEL					

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 114-19	SS 2	0.6 m to 1.2 m	0	83	17		7.2
Description	Classification	D <sub>60</sub>	D <sub>30</sub>	D <sub>10</sub>	C <sub>u</sub>	C <sub>c</sub>	
Sand some Silt trace Clay	SP	0.220	0.1200	0.1	4.15	1.23	

Issued By: Jacob Bell  
(Senior Project Manager)

Date Issued: June 29, 2021



Hydrogeological Investigation – 152 & 156 Miller Drive, City of Barrie, ON – Revision 1  
2609216 Ontario Inc.  
Cambium Reference: 9089-002  
May 15, 2024

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## **Appendix E**

## **SWHT Results**

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Cambium Inc.  
135 Bayfield St #102, Barrie, ON L4M 3B3

**Slug Test Analysis Report**

Project: Hydrogeological Investigation

Number: 9089-002

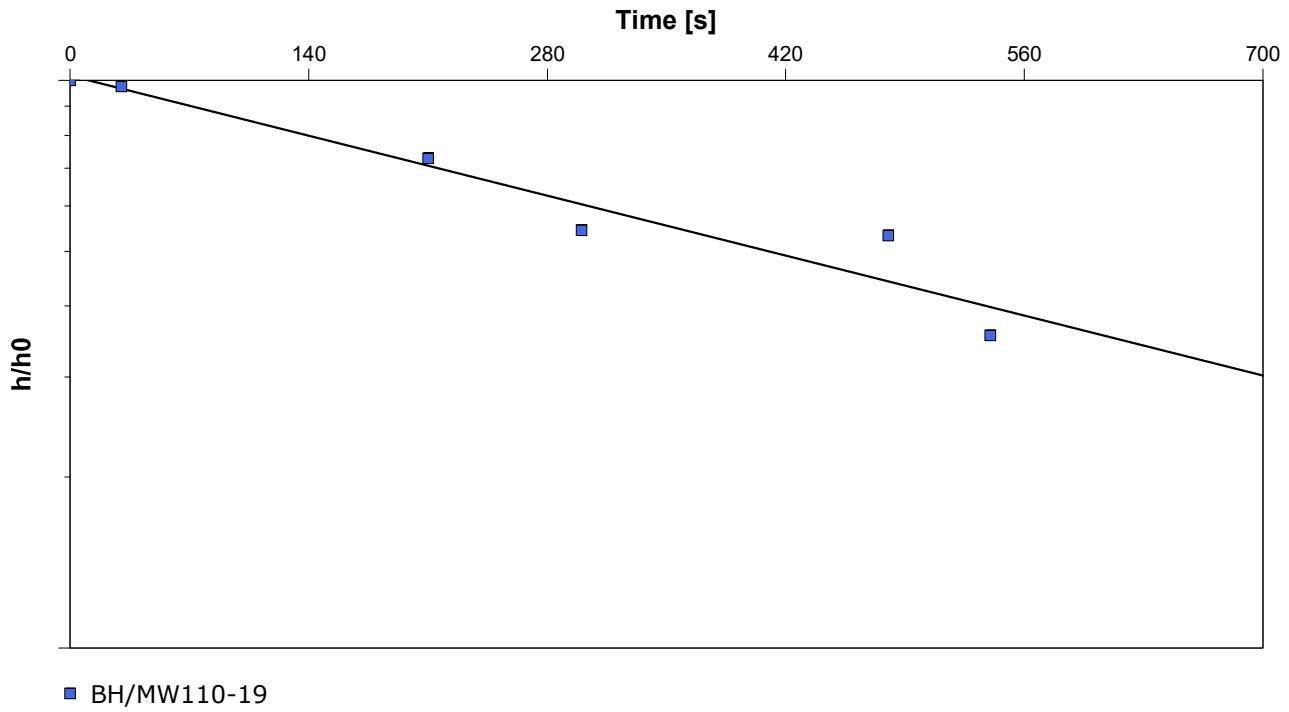
Client: Park City Inc.

Location: 152 & 156 Miller Drive, Barrie      Slug Test: BH/MW110-19      Test Well: BH/MW110-19

Test Conducted by: Chris/Nicole      Test Date: 6/23/2021

Analysis Performed by: Sudhakar Kurli      Hvorslev      Analysis Date: 6/25/2021

Aquifer Thickness: 3.64 m



Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]
BH/MW110-19	$1.24 \times 10^{-6}$



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## **Appendix F**

### **Water Balance Calculations**

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# Water Balance Calculations

152 156 Miller Drive, Barrie, Ontario



# Pre- and Post-Development Water Balance Calculations

152 156 Miller Drive, Barrie, Ontario

## 1 Climate Information

Precipitation	933 mm/yr
Actual Evapotranspiration	535 mm/yr
Water Surplus	398 mm/yr

## 2 Infiltration Rates

### Table 2 Approach - Infiltration factors

Topography: Rolling Land	0.2
Soil Type: Sandy Loam	0.35
Cover: Cultivated land/Woodland	0.15
<b>Total Infiltration Factor</b>	<b>0.7</b>

Infiltration (Water Surplus \* Infiltration Factor) **279 mm/yr**

Run-off (Water Surplus - Infiltration) **119 mm/yr**

### Table 3 Approach - Typical Recharge Rates

Coarse Sand and Gravel	>250	mm/yr
Fine to medium sand	200-250	mm/yr
Silty sand to sandy silt	150-200	mm/yr
Silt	125-150	mm/yr
Clayey Silt	100- 125	mm/yr
Clay	<100	mm/yr

Site development area is underlain predominantly by sandy silt to silty clay

Based on the above, the recharge rate is typically **150-200 mm/yr**

## 3 Pre-Development Property Statistics

	ha	m <sup>2</sup>
Total Paved Area	0.04	394
Total Roof Area	0.04	423
Total Landscape Area	1.36	13,615
<b>Total</b>	<b>1.44</b>	<b>14,432</b>

## 4 Post-Development Property Statistics

	ha	m <sup>2</sup>
Total Paved Area	0.41	4,219
Total Roof Area	0.28	5,093
Total Landscape Area	0.31	5,120
<b>Total</b>	<b>1.01</b>	<b>14,432</b>



# Pre- and Post-Development Water Balance Calculations

152 156 Miller Drive, Barrie, Ontario

## 5 Pre-Development Water Balance

Land Use		Area (m <sup>2</sup> )	Precipitation (m <sup>3</sup> )	Evapotranspiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
Impervious Areas	Paved Area	394	368	37	-	331
	Roof Area	423	395	39	-	355
Pervious Areas	Landscape Area	13,615	12,703	7,284	3,793	1,626
	Totals	14,432	13,465	7,360	3,793	2,312

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

## 6 Post-Development Water Balance

Land Use		Area (m <sup>2</sup> )	Precipitation (m <sup>3</sup> )	Evapotranspiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
Impervious Areas	Paved Area	4,219	3,936	394	-	3,543
	Roof Area	5,093	4,752	475	-	4,277
Pervious Areas	Landscape Area	5,120	4,777	2,739	1,426	611
	Totals	14,432	13,465	3,608	1,426	8,431

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

## 7 Comparison of Pre- and Post-Development

	Precipitation (m <sup>3</sup> )	Evapotranspiration (m <sup>3</sup> )	Infiltration (m <sup>3</sup> )	Run-off (m <sup>3</sup> )
Pre-Development	13,465	7,360	3,793	2,312
Post-Development	13,465	3,608	1,426	8,431
Change in Volume	-	3,752	2,367	6,119
Change in %	-	51	62	265

## 8 Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration (m <sup>3</sup> /yr)	3,793
Volume of Post-Development Infiltration (m <sup>3</sup> /yr)	1,426
Deficit from Pre to Post Development Infiltration (m <sup>3</sup> /yr)	2,367
Percentage of Roof Runoff required to match the pre-development infiltration (%)	55