# **FINAL**

# HYDROGEOLOGICAL INVESTIGATION REPORT PROPOSED DEVELOPMENT AT 334 & 340 ARDAGH ROAD, CITY OF BARRIE, ONTARIO

Prepared for:
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Project: SP22-01117-00 October 30, 2024



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# 1.0. INTRODUCTION AND BACKGROUND

Sirati & Partners Consultants Limited (SIRATI) was retained by Evans Planning (the Client) to conduct a Geotechnical Investigation, Phase One Environmental Site Assessment and Hydrogeological Services for a proposed residential development at 334 & 340 Ardagh Road in the City of Barrie, Ontario (the Site). See Figure 1 for the Site Location Plan drawing.

This report is consisting of findings from Hydrogeological Investigation performed for the Site. The work carried out for this Hydrogeological Investigation was completed in accordance with SIRATI's Proposal No. P22-09-108 (dated September 12, 2022) for the above noted scope of work. The findings of the Geotechnical Investigation and Phase One Environmental Site Assessment, completed concurrently with the Hydrogeological Investigation and prepared by SIRATI, are presented under separate cover. Pertinent information derived from these reports were used to complete the hydrogeological investigation.

## 1.1. Objective

The purpose of this hydrogeological investigation is to assess the existing subsurface hydrogeological conditions within the study area, using select borehole locations at the Site to make preliminary recommendations regarding hydrogeological aspects of the Site as they pertain to the development design. The hydrogeological investigation also provides an assessment of the potential dewatering and related permitting requirements for the proposed residential development, as well as the potential impacts to the surrounding environment due to proposed development and to provide recommendations on potential mitigation measures, where required.

This Hydrogeological Investigation is to perform based on the concept design plan and other work-related documents provided to SIRATI, in addition to applicable codes and standards. The recommendations provided in this report generally follow accepted practice for hydrogeological consultants in Ontario.

If there are any changes to the design features that are relevant to the analyses included in this report, SIRATI should be contacted to review the design and confirm whether the conclusions and recommendations contained within this report still apply. It may then be necessary to carry out additional field investigation and analysis before the recommendations made by SIRATI can be relied upon.

This report has been prepared for the use by the Client and its architects and designers for this Site.

Third party use of this report without Sirati & Partners Consultants Limited consent is

prohibited. The limitation conditions presented in Section 15.0 and Appendix H form an integral part of the report and they must be considered in conjunction with this report.

## 1.2 Scope of Work

SIRATI is retained to perform Geotechnical Investigation, Phase One Environmental Site Assessment and Hydrogeological Investigation at the Site (reported under a separate cover). In conjunction with the Geotechnical Investigation and Phase One Environmental Site Assessment, this hydrogeological assessment is required to be carried out consisting of the following scope of work:

- Review of available background information: a review of available geological, environmental
  and hydrogeological information for the Site and surrounding areas and review of other
  investigation reports completed for the Site was conducted, if available. This is to provide
  background information to allow for characterization of regional and local geological and
  hydrogeological conditions.
- **Site inspection**: an inspection of the Property is to be conducted to review existing site conditions including identification of any hydrogeological features such as significant areas of potential groundwater recharge or areas of groundwater discharge.
- **Private well survey:** Review of a water well inventory survey conducted through the database maintained by the Ministry of Environment, Conservation and Parks (MECP) for the area within approximately 500 m radius of the Site. The well information was used in the assessment of potential impact on the water wells.
- Completion of boreholes/monitoring wells: Drill Boreholes and install monitoring wells at strategic locations within the Site to obtain the representative information of soil and groundwater at the Site.
- Measurement of groundwater levels: Perform well development and measure groundwater levels in the monitoring wells at the Site. Interpret the monitoring data to analyse the groundwater flow regime.
- In-situ hydraulic conductivity tests: Perform in-situ hydraulic conductivity tests (rising head tests) in two (2) of the monitoring wells (BH/MW-02A and BH/MW-04) to estimate hydraulic conductivity of the underlying soils. The estimated hydraulic conductivity will be used for dewatering rate calculations.
- Construction dewatering assessment: Review the need for short-term construction dewatering and long-term drainage requirements, and the estimate the dewatering rates based on the

observed soil and groundwater conditions and the Preliminary design provided to SIRATI for the proposed construction.

- PTTW and EASR: Evaluate the need for permit-to-take-water (PTTW) or Environmental Activity and Sector Registry (EASR) registration, based on the short-term and long-term dewatering rates.
- Water Balance (Preliminary): estimation for a preliminary water balance for the proposed development in comparison to pre-development conditions using the Thornthwaite-Mather method based on the climate information obtained from the nearest Environment Canada weather station.
- Assessment of potential impacts: Assessment of potential impacts due to the proposed development and if required, identification of mitigation measures.
- **Reporting:** Prepare a hydrogeological investigation report summarising the findings and results obtained from the investigation and provide recommendations.

As mentioned, the hydrogeological investigation was carried out concurrently with the geotechnical investigation program. As such, the data obtained from the boreholes and monitoring wells are to be incorporated into this hydrogeological investigation report.

The hydrogeological study is to be carried out in accordance with all the following acts and regulations:

- Ontario Water Resources Act.
- Ontario Regulation 387/04 (Water Taking Regulation).
- Ontario Regulation 63/16 (Water Taking under the Environmental Protection Act).
- City of Barrie Sewer Use By-Law 2021-002.

#### 2.0. LAND USE

Currently the Site (334 and 340 Ardagh Road) is used for residential purposes. The area surrounding the Site is currently utilized for residential purposes. The site features are shown on survey map included in Appendix A. See Figure 1 for the Site Location Plan drawing.

#### 3.0. **DEVELOPMENT PLAN**

As presented in the Preliminary design drawings in Appendix A (Preliminary Drawing A1 334 & 340 Ardagh Road, Barrie Ontario, Vulcan Design Inc., Nov. 30/23), it is understood that the proposed development will include a 20-unit Residential townhouse development that includes the Site properties, 334 and 340 Ardagh Road, Barrie, Ontario. These include 10 townhouse units at 334 Ardagh Road (Block 3 & Block 4) and 10 townhouse units at 340 Ardagh Road. (Block 1 & Block 2).

#### 4.0. ENVIRONMENTAL FEATURES

To assess environmental features, the databases maintained by the Ministry of Natural Resources and Forestry (MNRF), the Ministry of Environment, Conservation and Parks (MECP) and the Nottawasaga Valley Conservation Authority (NVCA) were reviewed.

Based on the data reviewed, the Site is situated within the Middle Nottawasaga River Reach Subwatershed of the Nottawasaga Valley Watershed under the jurisdiction of NVCA. The Bear Creek Central East Branch is located 345 m west of the Site and Henderson Creek is located 305 m east of the site. Figure 4 presents the location of the Site, Bear Creek Central East Branch and Henderson Creek.

Based on review of the MNRF database, the Site is not located in any area identified as an area of natural heritage & scientific interests (ANSI) or wetland, (Figure 4).

A regulated area represents the greatest physical extent of the combined hazards, plus a prescribed allowance as set out in the Conservation Authorities Act to protect and safeguard watershed health in terms of environmental areas such as wetlands, shorelines and watercourses. As such, a site located within a regulation area, development restrictions shall apply to the proposed development. The Site does not lie within a NVCA regulated area (as shown on Figure 5).

# 5.0. SOURCE WATER PROTECTION AREA

Based on review of the MECP's Source Protection Information Atlas, the Site is located within the Nottawasaga Valley S.P.A. (Source Protection Area). The Site is found in an area identified as a Quantity Wellhead Protection Area WHPA-Q1 and WHPA-Q2 (low stress). The Site is located in a Groundwater Quality Vulnerable Area (see Figure 6).

### **5.1.** Quantity Wellhead Protection Area (WHPA-Q)

Based on review of the MECP's Source Protection Information Atlas, the Site is in The City of Barrie, Ontario, Wellhead Protection Areas for Quantity (WHPA-Q1 and Q2) with low stress. The Site is greater than 1 km southwest of the nearest municipal groundwater supply well head. It is not likely that temporary shallow construction dewatering at the Site will significantly impact the deeply seated municipal groundwater supply.

#### 5.2. Wellhead Protection Area -D (WHPA-D)

Wellhead Protection Area (WHPA) is an area that is related to a wellhead, within which it is desirable to regulate or monitor drinking water threats. WHPAs are delineated for threats to quality and quantity.

Wellhead Protection Areas for Quality (WHPA-A, -B, -C, -D) are the areas near a municipal well which are sensitive to contamination, and which are arranged according to either a set distance or delineated based on the time of-travel (up to 25 years) that it would take for water entering the ground to reach the well. WHPAs are also delineated for municipal wells where nearby surface water flows can seep through soil and influence the well (WHPA-E). This situation is known as groundwater under the direct influence of surface water, or a GUDI well.

The Site is found in a WHPA-D area, which indicates that water and any pollution that may be present can reach the well within 25 years. The site has a WHPA-D score of 2. The site is within a highly vulnerable aquifer (score of 6). The area lies within a chloride contaminant issue contributing area.

#### **6.0.** PHYSICAL SETTING

#### **6.1. Topography and Drainage**

Using the interactive topographic map generator (https://atlas.gc.ca), the topography in the vicinity of the Site gently slopes from the south to the north. The area slopes from the Site (at elevation +/-247 mASL) northwards toward the low-lying wetland area of Bear Creek (400 m to the northwest at elevation +/-204 mASL). Drainage is anticipated to follow the local topography, towards the north/northwest to Bear Creek (as shown in Figure 4).

Based on the SIRATI geotechnical report, the Property is relatively flat with an elevation of 246.4 mASL to 247.2 mASL.

#### **6.2. Physiography**

According to Chapman and Putnam (1984), the Site is located within the Simcoe Lowlands, sand plain physiographic region (Nottawasaga Basin, bordering the Camp Borden Sand Plain). See Figure 7.

#### **6.3.** Overburden

According to the Surficial Geology of Southern Ontario (Ontario Geological Survey, 2003) the Site (as shown in Figure 8) is covered by Glaciolacustrine coarse-grained sediment, consisting of silt and sand matrix (secondary soil materials of clay and silt). Based on Map P.3212 Bedrock Topography Barrie Area (OGS, 1993), the overburden thickness around the Site is approximately 170 m.

# 6.4. Bedrock Geology

According to the Paleozoic Geology of Southern Ontario (Ontario Geological Survey, 2007), the Site (shown in Figure 9) is underlain by the Middle Ordovician, Simcoe Group, Shadow Lake Formation and Lindsay Formation including limestone, dolostone, shale, arkose and sandstone.

### 7.0. HYDROGEOLOGY

Water well records on file with the Ministry of the Environment, Conservation and Parks (MECP) serve as a database for this hydrogeological assessment. The well locations were provided from the MECP interactive water well record database. According to the well records, there appears to be no well record for the Site and 18 well records within a 500 m radius around the property. The locations of the recorded water wells are shown on Figure 10, see Appendix G for Well Record information.

The water well records include dug wells, domestic water wells, test holes, monitoring holes, observation holes and abandoned wells/test holes.

Based on the details in the well records, a thick layer of overburden materials is present in the study area. Bedrock was not encountered at the maximum water well depth of about 45.1 mbgs. The groundwater levels recorded in the water wells ranged from flowing conditions to 15.2 m below ground surface (eight domestic water well records were listed). Based on the details in the water well records, the overburden material noted consisted primarily of sand and sandy clayey gravelly till like deposits.

Note, one domestic type, metal cased potable type groundwater well was identified at the Site (at 340 Ardagh Road). No well tag was observed to be attached to the potable type of groundwater well and no MECP water well record was located for this well. This well is recommended to be decommissioned in accordance with O.Reg. 903 and local Municipal guidelines.

#### 8.0. FIELD WORK METHODOLOGY

### 8.1. Borehole Drilling and Monitoring Well Installation

As part of the geotechnical investigation, two (2) boreholes equipped with monitoring wells (numbered as BH/MW-01 and BH/MW-02A) were drilled at 334 Ardagh Road and two (2) boreholes equipped with monitoring wells (numbered as BH/MW-03 and BH/MW-04) were drilled at 340 Ardagh Road on January 20, 2023. The approximate borehole and monitoring well locations are shown in Figure 2.

The boreholes were advanced to depths ranging from 6.7 mbgs to 9.5 mbgs, and monitoring wells were constructed using 2-inch diameter, 1.5 m or 3 m PVC screens at depths from 6.7 mbgs to 7.9 mbgs. The construction details for the monitoring wells are presented in the table below.

Table 8-1 **Monitoring Well Construction Details** 

Monitoring Well	Ground Elevation (mAMSL)	Monitoring Well Depth (mbgs)	Screen Interval (mbgs)	Screened Soil
BH/MW-01	247.16	6.7	3.7 ~ 6.7	Sand to silty sand with gravel
BH/MW-02A	246.44	7.9	4.9 ~ 7.9	Sand to layers of clay and silt
BH/MW-03	247.21	7.6	4.6 ~ 7.6	Sand to silty sand
BH/MW-04	246.61	7.6	4.6 ~ 7.6	Sand with gravel to clay with sand

Notes: mAMSL – metres above mean sea level; mbgs – metres below ground surface

## 8.2. Groundwater Monitoring and Elevation Survey

After the well installation, groundwater levels were measured in the newly installed monitoring wells. In addition, location and elevation survey was conducted using a GPS unit for both the boreholes and the monitoring wells advanced at the Site.

## 8.3. Hydraulic Conductivity Test (Single Well Response Test/Slug Test)

In-situ hydraulic conductivity tests, also called single well response test or slug test, were conducted on two (2) monitoring wells, BH/MW-02A and BH/MW-04 on February 27, 2023. During the test, a datalogger was placed in the monitoring well after the initial water level was measured. Then, a certain amount of water was removed from the test well (for a rising head test) to create a water level drawdown in the well. The water level recovery was recorded by the datalogger, and the data was then used for estimating the hydraulic conductivity of the screened soil.

### 8.4. Groundwater Sampling and Chemical Testing

Groundwater samples were collected on May 07, 2024, from all monitoring wells BH/MW-01 BH/MW-02A, BH/MW-03, and BH/MW-04. Chemical testing was completed on the groundwater samples. Chemical testing was completed to assess the general water quality for the purpose of excess water disposal (potentially generated from the Site).

The groundwater samples were submitted to AGAT Laboratories for analysis as per City of Barrie Sewer Use By-Law 2021-002 Guidelines (a By-Law to prohibit, regulate and control discharges into bodies of waters within regional boundaries or into the regional sanitary sewers, storm sewers, sanitary sewage works and all tributary sewer systems).

#### 9.0. SUMMARIZED SITE CONDITIONS

## 9.1. Soil Stratigraphy

The soil stratigraphy as recorded from the advanced boreholes completed by SIRATI, generally consisted of an upper layer of sandy fill material (1.8 m to a maximum depth of 2.7 m at BH/MW-04) over native soils. Native soils consisted of sand deposits with varied amounts of silt and varied amounts of gravel underlain by lean clay deposits interbedded with silt and sand. No bedrock was encountered at the maximum explored depth of 9.5 mbgs.

Following is the generalized stratigraphy encountered at 334 and 340 Ardagh Road (the Site), as depicted in the borehole logs.

## 334 ARDAGH ROAD:

**Topsoil:** A 305 mm thick surficial layer of topsoil was encountered at borehole BH/MW-01.

<u>Clear Stone</u>: A 50 mm thick surficial layer of clear stone was encountered at borehole location BH/MW-02A.

**<u>Fill Material</u>**: A layer of fill layer was observed underneath the surficial topsoil and clear stone in both boreholes and extending to 1.8 m below the existing ground surface. The fill material mainly consists of sand, trace to some silt, trace gravel, trace organics, trace rootlets. Fill material was generally moist and brown and dark brown in color.

The measured SPT 'N' values in the fill material ranged was 2 and 6 blows per 300 mm penetration, indicating a very loose to loose state.

### **Cohesionless Soil Deposits:**

A layer of native cohesionless soils consist of silty sand with gravel to poorly graded sand with trace gravel, trace cobbles encountered at both borehole locations. Cohesionless soils was found underlaying the fill layer and extending to depths ranging between 6.1 m below the existing ground surface to the termination depth (6.7 m) of the borehole BH/MW-01. Native cohesionless soil deposits were generally brown in color and moist to wet.

The measured SPT 'N' values in the cohesionless soil materials ranged from 6 to 43 blows per 300 mm penetration (increasing with depth), indicating a loose to dense conditions.

Grain size analysis of cohesionless soil sample (BH/MW-01/SS6) was conducted, and the result is presented with the following fractions:

Clay: 1% Silt: 38% Evans Planning / Final Hydrogeology Investigation Report, Proposed Development 334 & 340 Ardagh Rd., Barrie, Ontario

Sand: 38% Gravel: 23%

## **Interbedded layers of Lean Clay and Silty Sand:**

Below the cohesionless soil deposits, native soil consisting of interbedded lean clay and silty sand with trace gravel, brown, and moist to wet was encountered at borehole BH/MW-02A, extending to the termination depth (9.5 m) of the borehole.

The measured SPT 'N' values in the interbedded lean clay material ranged from 18 to over 50 blows per 300 mm penetration, indicating a very stiff /compact to hard/ very dense consistency/conditions.

Two (2) grain size analysis of two (2) representative samples (BH/MW-02A/SS7, BH/MW-02A/SS9) were conducted, and the results are presented with the following fractions:

Clay: 2% to 41% Silt: 36% to 50% Sand: 9% to 61% Gravel: 0% to 1%

One representative sample (BH/MW-02A/SS7) was also subjected to Atterberg limits testing. The liquid limit was found to 37% and the plastic limit was found to 18%. The soil has a 19% plasticity index indicating an inorganic medium plastic clay (cohesive). The average soil moisture content of lean clay material at the borehole is at approximately 27% by weight and is below the liquid limit.

#### 340 ARDAGH ROAD:

**Asphalt:** A 40 mm thick surficial layer of asphalt was encountered in borehole BH/MW-03.

**Topsoil:** A 230 mm thick surficial layer of topsoil was encountered at borehole BH/MW-04.

<u>Granular fill:</u> A 200 mm thick layer of granular fill material consisting of sand and gravel was observed directly below the surficial asphalt layer at borehole BH/MW-03.

**Fill Material:** A layer of fill material was observed underneath the granular fill layer in borehole BH/MW-03 and below the surficial topsoil layer in borehole BH/MW-04. The fill material mainly consists of sand to gravelly sand, trace to some silt, trace organics, and trace rootlets. Fill material was generally moist and brown and dark brown in color and extending to depths 1.8 m and 2.7 m below the existing grade.

The measured SPT 'N' values in the fill material ranged was 1 and 10 blows per 300 mm penetration, indicating very loose to loose compacted state.

<u>Cohesionless Soil Deposits:</u> A layer of native cohesionless soil soils consist of sand/ poorly graded sand with gravel to silty sand/ silty sand with gravel, trace cobbles, and trace clay was encountered in both boreholes underlaying the fill material. Native cohesionless deposits extend to depths ranging between 6.1 m below the existing grade (in BH/MW-04) and the termination depth (8.3 m) of the borehole (in BH/MW-03). Native cohesionless soil deposits were generally brown in color and moist to wet.

The measured SPT 'N' values in the cohesionless soil materials ranged from 10 to over 50 blows per 300 mm penetration, indicating compact to very dense material condition.

Grain size and Hydrometer analysis of cohesionless soil samples (BH/MW-03/SS4, BH/MW-04/SS7, and BH/MW-04/SS5) was conducted, and the result is presented with the following fractions:

Clay: 1%

Silt: 6% to 30% Sand: 57% to 68% Gravel: 1% to 37%

<u>Lean Clay:</u> Native cohesive soil comprising of lean clay with sand, and trace gravel was encountered in borehole BH/MW-04 underlaying the cohesionless soils and extending to the termination depth (8.2 m) of the borehole.

The measured SPT 'N' values in the cohesive strata ranged from 14 to 21 blows per 300 mm penetration, indicating stiff to very stiff consistency.

Grain size analysis of two (2) cohesive soil samples (BH/MW-04/ SS7, and BH/MW-04/ SS8) were conducted, and the results are presented with the following fractions:

Clay: 14% and 30% Silt: 44% and 57% Sand: 25% and 29% Gravel: 0% and 1%

One soil sample (BH/MW-04/SS7) was also subjected to Atterberg limits testing. The liquid limit was found to be 25% and the plastic limit was measured at 15%. As such, the plasticity index of the tested soil is a 10% indicating an inorganic low plastic clay (cohesive). The average soil moisture content of the cohesive deposits at the borehole is at approximately 25% by weight and is equal to the liquid limit.

The details of the soil descriptions are presented in the Borehole Logs and gradation results in Appendix B. A geological cross-section profile is presented in Figure 12.

### 9.2. Groundwater Conditions

Groundwater conditions were observed during the borehole drilling. The boreholes were open upon completion. The boreholes encountered groundwater upon completion.

#### 9.2.1 Groundwater Levels and Elevations

Groundwater levels were measured in all the newly installed monitoring wells on three (3) occasions, February 27, 2023, March 13, 2023, and May 07, 2024. The measured and recorded groundwater levels are presented in Table 9-1 below.

Table 9-1: Measured Groundwater Levels in 2023 – 334 & 340 Ardagh Rd., Barrie.

25. 1. 1	Ground	Screen	Date: 02	/27/2023	Date: 03/13/2023		
Monitoring Well	Elevation	Depth	Depth to Groundwater	Groundwater Elevation	Depth to Groundwater	Groundwater Elevation	
	(mAMSL)	(mbgs)	(mbgs)	(mAMSL)	(mbgs)	(mAMSL)	
BH/MW-01	247.16	3.7 ~ 6.7	4.91	242.25	4.98	242.18	
BH/MW-02A	246.44	4.9 ~ 7.9	5.70	240.74	5.40	241.04	
BH/MW-03	247.21	4.6 ~ 7.6	4.91	242.30	4.91	242.30	
BH/MW-04	246.61	4.6 ~ 7.6	5.04	241.57	5.04	241.57	

Notes: mAMSL – metres above mean sea level; mbgs – metres below ground surface.

Table 9-1: Measured Groundwater Levels in 2024 – 334 & 340 Ardagh Rd., Barrie.

	Ground	Screen	Date: 05	/07/2024
Monitoring Well	Elevation (mAMSL)	Depth (mbgs) Depth to Groundwater (mbgs)		Groundwater Elevation
			(mbgs)	(mAMSL)
BH/MW-01	247.16	3.7 ~ 6.7	4.45	242.71
BH/MW-02A	246.44	4.9 ~ 7.9	4.57	241.87
BH/MW-03	247.21	4.6 ~ 7.6	4.46	242.75
BH/MW-04	246.61	4.6 ~ 7.6	4.49	242.12

Notes: mAMSL – metres above mean sea level; mbgs – metres below ground surface.

As presented above, the groundwater levels measured in the monitoring wells across the Site ranged from 4.45 mbgs at BH/MW-01 on May 07, 2024, to 5.70 mbgs at BH/MW-02A on February 27, 2023, while elevations ranged from 240.74 mAMSL at BH/MW-02A on February 27, 2023 to 242.75 mAMSL at BH/MW-03 on May 07, 2024.

It should be noted that groundwater levels can vary and are subject to seasonal fluctuations and in response to major weather events.

#### 9.2.2 Inferred Groundwater Flow Direction

Based on the water level elevations dated March 13, 2023, the groundwater elevation contours were established and are shown on Figure 11. The shallow unconfined aquifer groundwater flow direction was inferred to be northward.

## 9.3. Estimated Hydraulic Conductivity

The hydraulic conductivity (K-value) of the soils was estimated based on the results obtained from the single well response tests (slug tests).

Single well response tests or slug tests were conducted as part of this hydrogeological study in two (2) monitoring wells, BH/MW-02A and BH/MW-04 on February 27, 2023. Either a falling head test or a rising head test was performed on the wells. Based on the data obtained from the single well response test/slug test, the hydraulic conductivity for the screened soils was estimated utilizing the Aqtesolv pumping test software with the Hvorslev method. Records of the slug tests and the data processing are provided in Appendix C.

The results of the estimated hydraulic conductivity are summarized in Table 9-2 below.

Monitoring Well	Screen Depth (mbgs)	Tested Soil Type	Hydraulic Conductivity (m/s)
BH/MW-02A	4.9 ~ 7.9	Sand to layers of clay and silt	7.40 x 10 <sup>-6</sup>
BH/MW-04	4.6 ~ 7.6	Sand with gravel to clay with sand	2.58 x 10 <sup>-5</sup>
	Average	1.66 x 10 <sup>-5</sup>	

Table 9-2: Results of Estimated Hydraulic Conductivity as per Slug Tests

As presented above, the estimated hydraulic conductivity of BH/MW-02A was  $7.40 \times 10^{-6}$  m/s and  $2.58 \times 10^{-5}$  m/s at BH/MW-04, with an average of  $1.66 \times 10^{-5}$  m/s.

It is important to note that during the in-situ hydraulic conductivity test, the water level in both tests was below the top of screen.

#### 9.4. Water Quality

Groundwater samples were collected from all monitoring wells (BH/MW-01, BH/MW-02A, BH/MW-03, and BH/MW-04) at 334 & 340 Ardagh Rd., Barrie, Ontario (The property), on May 07, 2024, from and were submitted to AGAT Laboratories for general chemistry compared to the City of Barrie Sewer Use By-Law Guidelines. It should be noted that AGAT Laboratories is accredited by the Canadian Association of Laboratory Accreditation (CALA).

The analyzed parameters for the groundwater included CBOD5, Organics, and Inorganics in Groundwater samples from the monitoring wells, BH/MW-01, BH/MW-02A, BH/MW-03, and BH/MW-04. A copy of the laboratory Certificate of Analysis is presented in Appendix D.

The analytical results were compared with the Limits for Town of Barrie Sewer Use By-law, and exceedances were found for several parameters. The details of the exceedances are presented in Table 9-3 below, Additionally, the analytical results for Trichloroethylene, derived from groundwater analysis and in accordance with O.Reg. 153/04, Table 2, are detailed in Table 9-4.

**Table 9-3 - Exceedances of Barrie Sanitary & Storm Sewer Use By-Law Criteria (Inorganics)** 

SAMPLE ID	LOCATION	Guideline	PARAMETER	CRITERIA (mg/L)	RESULT (mg/L)					
			Total Chromium	0.08	0.12					
58	-	CTODM	Total Copper	0.01	0.12					
	H/	STORM	Total Nicke	0.05	0.12					
5846407	BH/MW-01		Total Suspended Solids	15	4030					
.07	V-0		Total Aluminum	50	94.4					
	_	SANITARY	Total Iron	50	112					
			Total Suspended Solids	350	4030					
			Total Chromium	0.08	0.14					
		GEODA 6	Total Copper	0.01	0.10					
(,)	ВН	STORM	Total Nicke	0.05	0.14					
5846873	BH/MW-02A		Total Suspended Solids	15	10700					
587:			Total Zinc	0.04	0.40					
3		SANITARY	Total Aluminum	50	98.8					
			Total Iron	50	115					
			Total Suspended Solids	350	10700					
			Total Chromium	0.08	0.13					
	BH/MW-03	-	-	-	<b>H</b>	<b>H</b>	STORM	Total Copper	0.01	0.15
58			Total Nicke	0.05	0.27					
5846874			Total Suspended Solids	15	9020					
74	V-0:		Total Aluminum	50	83.0					
	ω	SANITARY	Total Iron	50	127					
			Total Suspended Solids	350	9020					
	П		Total Copper	0.01	0.05					
58	] JH/	STORM	Total Nicke	0.05	0.07					
5846875	ΔM		Total Suspended Solids	15	4500					
375	BH/MW-04	SANITARY	Total Iron	50	56.5					
	4	SAMITAKI	Total Suspended Solids	350	4500					

Table 9-4 - Trichloroethylene of Regulation 153/04, Table 2

LOCATION	CRITERIA (µg/L)	Date 27-Feb2023	Date 13-Mar2023 (μg/L)	Date 07-May-2024 (Organic) (µg/L)
BH/MW-01	1.6	2.80	-	1.9
BH/MW-02A	1.6	1.78	-	0.4
BH/MW-03	1.6	2.46	-	0.4
BH/MW-04	1.6	-	2.18	0.9

#### DISCUSSION

- As shown in the table 9-3, there are some exceedances in groundwater samples, therefore for discharge water during the construction treatment must done.
- For discharge water in sanitary, except the water resulting from BH/MW-01 which has
  exceedance for Total Aluminium the other water analysis shows only Total Iron and TSS has
  exceedance.
- According to the criteria in Regulation 153/04, Table 2, the recent groundwater analysis indicates that only the water sample from BH/MW-01 has an exceedance for Trichloroethylene.

## 10.0. CONSTRUCTION DEWATERING

Construction dewatering is intended to lower the groundwater levels in the excavation area in order to ensure a dry working condition.

The requirements for construction dewatering generally depend on the Site's soil and groundwater conditions including soil type, soil permeability or hydraulic conductivity, local groundwater levels, and the design of the proposed development such as the foundation and/or basement elevation, as well as the size of proposed structure, etc.

## 10.1. Proposed Development, Anticipated Excavation and Dewatering

The proposed development will consist of the construction of twenty (20) residential townhouse type structures. These include 10 townhouse units at 334 Ardagh Road. (Block 3 & Block 4) and 10 townhouse units at 340 Ardagh Road. (Block 1 & Block 2). Therefore, excavations will be expected for foundation and site services installation.

The geodetic elevations for the proposed finish grade were not specifically provided in the Preliminary drawings (Preliminary Drawing A4 334 & 340 Ardagh Road, Barrie Ontario, Vulcan Design Inc., November 30, 2023) as supplied by the Client, however, based on the Preliminary drawings, the top of U/S of footing is 4' MIN. (-1.22 m.) below the finish grade. According to the provided survey Drawing 2485-Ardagh Road 334 & 340 -SRPR.DWG (Pearson + Pearson Surveying Ltd., July 7, 2022), ground level at the site is currently at 245.8 mASL in the northern area of the Site to 248.0 mAMSL at the southeast area of the Site. It would be anticipated that excavations for foundations (including site services) may extend to the preliminary/estimated elevation of 243.78 mASL (Block 1), 245.38 mASL (Block 2), 245.18 mASL (Block 3) and 245.98 mAMSL (Block 4). Based on the groundwater table across the entire site, the highest water level is at 242.75 mASL in monitoring well BH/MW-03 on May 07, 2024. Assuming a 0.5 m dry condition below the underside of the footing, groundwater is not encountered, and dewatering will not be necessary.

Perimeter excavations to accommodate foundation construction are estimated to be approximately 1.0m wider for construction workability.

It should be noted that groundwater levels can vary and are subject to seasonal fluctuations and in response to major weather events.

Please contact SIRATI for re-evaluation of the construction dewatering analyses once the preliminary design is finalized (final site grades, finalized foundation elevations, storm and septic sewer design).

#### 10.2. **Construction Dewatering Rate Estimation (Short-term)**

As discussed, groundwater control in the form of construction dewatering would not be considered for the proposed foundation excavations and associated site services but for control rain fall during the construction it should be considered.

For rain fall dewatering rate assessment, the following measurements and assumptions based on available information would be made (See Appendix A for Preliminary Design Plan Drawings).

#### 334 ARDAGH ROAD.;

Excavation area:  $336.0 \text{ m}^2 + 490.0 \text{ m}^2 = 826 \text{ m}^2$  (additional 1.0m per wall to accommodate workability)

### 340 ARDAGH ROAD.;

• Excavation area:  $490.0 \text{ m}^2 + 336.0 \text{ m}^2 = 826 \text{ m}^2$  (additional 1.0m per wall to accommodate workability)

To account for the stormwater runoff on a rainy day during the construction at the Site, a 20 mm daily rainfall was considered for the purpose of dewatering design. The total runoff volume is given by the following formula:

Total Runoff Volume (V) per day = Excavation Area x Rainfall Intensity   
= Area 
$$m^2$$
 x 0.02 m/day   
=  $m^3$  /day or L/day.

Based on the above assumptions and measurements stormwater volume estimated to accumulate at 334 Ardagh Road would be approximately 16,520 L/day.

Based on the above assumptions and measurements stormwater volume estimated to accumulate at 340 Ardagh Road would be approximately 16,520 L/day.

To this end, appropriate measures should be taken during construction to deal with surface water into excavation to enable construction. Around the perimeter of the excavation, an inceptor perimeter trench should be constructed to prevent surface water ingress entering the excavation areas.

Please contact SIRATI for re-evaluation of the construction dewatering analyses once the preliminary design is finalized (final site grades, finalized U/S of Footing elevations, finalized foundation elevations, storm and septic sewer design).

The details of storm water runoff calculations are provided in Appendix E.

## **10.3.** Sub-drain Dewatering (Long-term)

In general, a subdrainage system or weeping tile system is recommended to be constructed for the proposed building to avoid hydrostatic pressure from groundwater on the footing walls as well as to achieve a dry condition for the basement level.

Long term sub-drain dewatering may not be anticipated for 334 and 340 Ardagh Road, as per the design assumptions (available information) provided by the Client.

## 10.4. Regulatory Permits or Registration

Any construction dewatering or water taking in Ontario are governed by Ontario Regulation 387/04 – Water Taking and Transfer, an Ontario regulation made under the Ontario Water Resource Act (OWRA), and/or Ontario Regulation 63/16 – Registration under Part II.2 of the Act – Water Taking, made under Environmental Protection Act and/or Section 34 of the Ontario Water Resources Act (OWRA).

According to Section 34 of the OWRA, any water taking over 50,000 litres per day may not take place without a valid permit, which shall be applied and obtained in accordance with the MECP's permit-to-take-water (PTTW) Manual, dated April 2005.

According to O. Reg. 63/16, a PTTW will not be required for temporary construction dewatering (for six months or less) in an amount greater than 50,000 L/day but less than 400,000 L/day. However, a registration or posting shall be processed through Environmental Activity and Sector Registry (EASR).

Based on the available information, above assumptions and measurements, the temporary construction dewatering rate for 334 and 340 Ardagh Road will not be anticipated but the stormwater volume in the excavation area estimate would be approximately 16,520 L/day for each excavation area and for a total of 33,040 L/day. Therefore, no PTTW or EASR registration will be required either for the temporary construction dewatering or for the long-term drainage discharge for the proposed residential development.

#### 10.5. Point of Discharge

For land developments in an urban area, the local sewer systems are usually used to receive the water generated from a development site. It is observed that catch basins and manholes are located along Ardagh Road, which can be considered as the water discharge points. However, if the local sewer system is selected, a permit or application to use the City of Barrie sewer system will be required.

### 11.0. WATER BALANCE

A preliminary water balance for the 334 Ardagh Road. Site and the 340 Ardagh Road. Site was completed. The water balance was calculated for both pre-development and post-development conditions to assess the change in overall rate of infiltration.

#### 11.1. Site Condition

The 334 Ardagh Road Site and the 340 Ardagh Road Site have previously been developed with residential dwellings, associated garages/sheds and paved areas. There are no open water bodies, creeks or wetlands located on the Site. The 334 Ardagh Road Site and the 340 Ardagh Road Site are proposed to be redeveloped with 20 residential townhouse units (10 townhouse units at 334 Ardagh Road and 10 townhouse units at 340 Ardagh Road).

For the purpose of water balance assessment, the development area can be categorized into three (3) types of areas: paved area, building/roof area and landscape/vegetated area, which are shown in Appendix A. A summary of the site area is listed in Table 11-1A (334 Ardagh Road) and Table 11-1B (340 Ardagh Road).

Table 11-1A: Pre-and Post-Development Site Conditions 334 Ardagh Road.

Type of Land Coverage	Pre-Development Area (m²)	Post- Development Area (m²)
Paved Area	28.75	201.97
Building/Roof Area	284.88	703.23
Landscape/Vegetated Area	1,185.4	593.83
Total (m <sup>2</sup> )	1,499.03	1,499.03

Table 11-1B: Pre-and Post-Development Site Conditions 340 Ardagh Road.

Type of Land Coverage	Pre-Development Area (m²)	Post- Development Area (m²)
Paved Area	157	201.33
Building/Roof Area	223	593.76
Landscape/Vegetated Area	1118.32	703.23
Total (m <sup>2</sup> )	1498.32	1,498.32

#### 11.2. **Site Level Water Balance**

Based on the Thornthwaite and Mather methodology (1957), the water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from ground or evapotranspiration by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage ( $\Delta S$ ).

The annual water budget can be expressed as:

$$P = ET + R + I + \Delta S$$

Where:

P = Precipitation (mm/year)

ET = Evapotranspiration (mm/year)

R = Run-off (mm/year)

I = Infiltration (mm/year)

 $\Delta S$  = Change in groundwater storage (taken as zero) (mm/year)

#### **Climate Data** 11.3.

The climatic data including monthly average temperature and precipitation were obtained from Environment Canada, for Shanty Bay weather station (Climate Identifier: 6117684).

The data were available between the years 1973 to 2006 i.e. 33 years. Temporal variations of mean annual temperature and precipitation are shown in Figures 11-2 and 11-3.

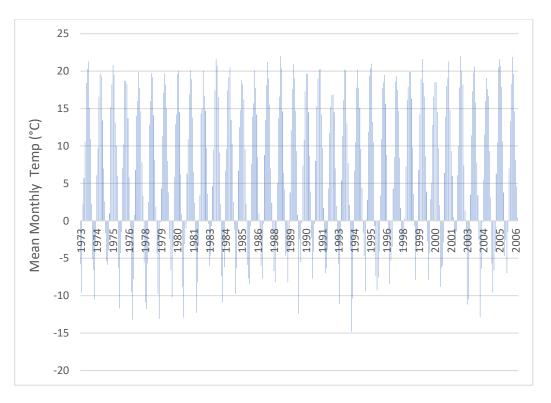


Figure 11.2: Mean Annual Temperature at the Site

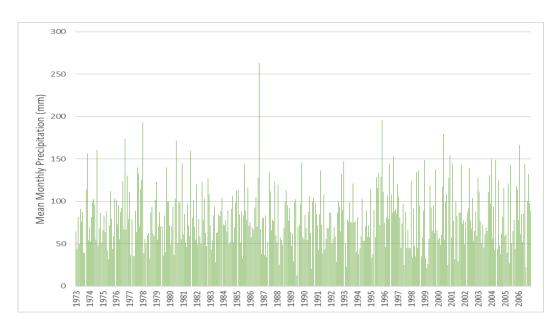


Figure 11.3: Mean Annual Precipitation at the Site

Average monthly variations of both temperature and precipitation were calculated for the period from 1973 to 2006 (33 years) and is presented below in Figure 11-3. The highest average temperature was recorded in the month of August, while the highest precipitation was in the month of November.

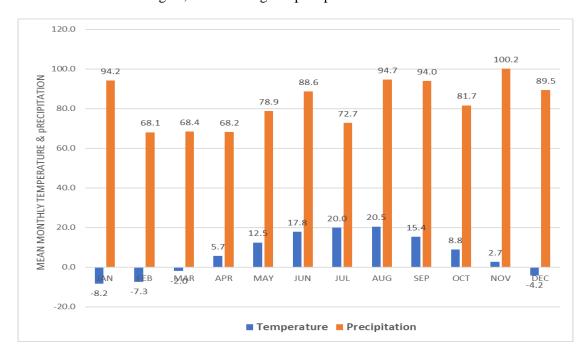


Figure 11.4: Mean Monthly Average Temperature and Precipitation at the Site

Based on the data for the precipitation and temperature, actual evapotranspiration was estimated to be about 531 mm/annum using the USGS Thornthwaite Monthly Water Balance software, and the average annual precipitation was recorded to be 999 mm/annum.

#### 11.4. Infiltration and Run-off

As mentioned above, the actual evapotranspiration was estimated to be 531 mm/annum. Given the average annual precipitation of 999 mm/annum, there is a water surplus of 468 (=999-531) mm/annum occurring at the Site, which can either infiltrate into subsurface or go as run-off.

The rate of infiltration at a site is expected to vary, based on a number of factors to be considered in any infiltration model. To partition the available water surpluses into infiltration and surface run-off, the MECP infiltration factor was used. The MECP Storm Water Management Planning and Design Manual (2003) methodology for calculating total infiltration based on topography, soil type and land cover was used, and a corresponding run-off component was calculated for the soil moisture storage conditions.

#### 11.5. Water Balance

The calculation of infiltration and runoff in the stages of pre-development and post-development is provided in Appendix F, and are presented in Tables 11-2A, 11-2B to 11-5A, 11-5B below. It should be noted that the statistical data used for the calculations was collected from drawing S1, November 30/23, prepared by Vulcan Design Inc.

Table 11-2A: Annual Pre-Development Water Balance 334 Ardagh Road

		Area	Precipitation	Evapotranspiration	Infiltration	Run-off
La	Land Use		(m³)	$(m^3)$	(m³)	$(m^3)$
	Paved Area	29	29	3	0	26
Impervious Areas	Building/ Roof Area	285	285	28	0	256
Pervious Areas	Landscape/ Vegetated Area	1185	1184	629	416	139
		1499	1498	661	416	421

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and roof areas.

Table 11-3A: Annual Post-Development Water Balance 334 Ardagh Road.

		Area	Precipitation	Evapotranspiration	Infiltration	Run-off
La	Land Use		(m³)	$(m^3)$	(m <sup>3</sup> )	(m <sup>3</sup> )
	Paved Area	202	202	20	0	182
Impervious Areas	Building/ Roof Area	703	703	70	0	632
Pervious Areas	Landscape/ Vegetated Area	594	593	315	208	69
		1,499	1,498	406	208	883

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and roof areas.

Table 11-4A: Comparison of Pre- and Post Development Water Balance Components 334

Ardagh Road

	Precipitation	Evapotranspiration	Infiltration	Run-off
	$(m^3)$	(m <sup>3</sup> )	(m³)	( <b>m</b> <sup>3</sup> )
Pre-Development	1,498	661	416	421
Post-Development	1,498	406	208	883
Change in Volume		-255	-208	463
Change in %			-50	110

Table 11-5A: Requirement for Infiltration of Roof Run-off 334 Ardagh Road

Volume of Pre-Development Infiltration (m³/annum)	416
Volume of Post-Development Infiltration (m³/annum)	208
Deficit from Pre to Post Development Infiltration (m³/annum)	208
Percentage of Roof Runoff required to match the pre-development infiltration (%)	33

Table 11-2B: Annual Pre-Development Water Balance 340 Ardagh Road

		Area	Precipitation	Evapotranspiration	Infiltration	Run-off
La	Land Use		(m³)	$(m^3)$	(m <sup>3</sup> )	$(m^3)$
	Paved Area	157	157	16	0	141
Impervious	Building/ Roof	223	223	22	0	200
Areas	Area					
Pervious	Landscape/	1,118	1,117	594	393	131
Areas	Vegetated Area					
		1,498	1,497	632	393	473

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and roof areas.

Table 11-3B: Annual Post-Development Water Balance 340 Ardagh Road

		Area	Precipitation	Evapotranspiration	Infiltration	Run-off
La	Land Use		(m³)	$(m^3)$	(m <sup>3</sup> )	(m <sup>3</sup> )
	Paved Area	201	201	20	0	181
Impervious Areas	Building/ Roof Area	703	703	70	0	632
Pervious Areas	Landscape/ Vegetated Area	594	593	315	208	69
		1,498	1,497	406	208	883

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and roof areas.

Table 11-4B: Comparison of Pre- and Post Development Water Balance Components 340 **Ardagh Road** 

	Precipitation	Evapotranspiration	Infiltration	Run-off
	$(m^3)$	(m <sup>3</sup> )	(m <sup>3</sup> )	$(m^3)$
Pre-Development	1,497	632	393	473
Post-Development	1,497	406	208	883
Change in Volume		-226	-184	410
Change in %			-47	87

Table 11-5B: Requirement for Infiltration of Roof Run-off 340 Ardagh Road

Volume of Pre-Development Infiltration (m³/annum)	393
Volume of Post-Development Infiltration (m³/annum)	208
Deficit from Pre to Post Development Infiltration (m³/annum)	184
Percentage of Roof Runoff required to match the pre-development infiltration (%)	29

#### 11.6. **Summary of Water Balance Calculation**

Based on the above calculations, a water balance summary could be made as below for 334 Ardagh Road:

- 1) There is a net increase in run-off at the 334 Ardagh Rd. Site of about 463 m<sup>3</sup>/annum (or 110% increase), from 421 m<sup>3</sup>/annum to 883 m<sup>3</sup>/annum. This increase is as a result of the development of the Site with more impervious areas such as roof and paved areas and reduction in pervious landscaped areas.
- 2) Without implementation of mitigation measures, there is a net deficit of about 208 m<sup>3</sup>/annum (50% decrease) in the post-development infiltration from 416 m<sup>3</sup> to 208 m<sup>3</sup> on a yearly basis.
- 3) There is a net volume of 632 m<sup>3</sup>/annum potentially available to be collected from roof areas, which is sufficient to fully compensate for the post-development infiltration deficit. A diversion of 33% of roof runoff will compensate the total infiltration deficit.

Based on the above calculations, a water balance summary could be made as below for 340 Ardagh Rd.:

1) There is a net increase in run-off at the 340 Ardagh Road Site of about 410 m<sup>3</sup>/annum (or 87% increase), from 473 m<sup>3</sup>/annum to 883 m<sup>3</sup>/annum. This increase is a result of the development of the Site with more impervious areas such as roof and paved areas and reduction in pervious landscaped areas.

- 2) Without implementation of mitigation measures, there is a net deficit of about 184 m<sup>3</sup> /annum (29% decrease) in the post-development infiltration from 393 m<sup>3</sup> to 208 m<sup>3</sup> on a yearly basis.
- 3) There is a net volume of 632 m<sup>3</sup>/annum potentially available to be collected from roof areas, which is sufficient to fully compensate for the post-development infiltration deficit. A diversion of 29% of roof runoff will compensate the total infiltration deficit.

#### **Discussions on Enhance Water Balance Measures** 11.7.

Based on the above water balance calculations for 334 Ardagh Road, an infiltration deficit will be anticipated in an amount of 208 m<sup>3</sup>/year due to the development of the Site. On the other hand, a total amount of 632 m<sup>3</sup>/year of roof water is anticipated to be available from the roofs of the buildings, which is sufficient to compensate for the infiltration deficit caused due to the proposed development.

Based on the above water balance calculations for 340 Ardagh Road, an infiltration deficit will be anticipated in an amount of 184 m<sup>3</sup>/year due to the development of the Site. It is our understanding that the City is restricting using LID infiltration methodology as the project site is located in an area where the groundwater is impacted with VOCs. As such, to compensates this deficit, it is our recommendation that the design engineer should use alternate recharging methodologies such as, permeable pavement in parking lots, walkways and roadways and/or create wetlands restoration/construction and improve soil permeability for the infiltration.

The soils identified at 334 and 340 Ardagh Road, were found to be mainly composed of sandy fill soils with varied amounts of silt to native sand soils, which generally have a fair to good infiltration capacity.

#### 12.0. ASSESSMENT OF POTENTIAL IMPACTS

#### 12.1. **Natural Features**

As discussed, no natural features such as creeks, open water body or wetlands are present on the Site. However, Bear Creek Central East Branch is located 345 m west of the Site and Henderson Creek is located 305 m east of the site. Given that short- and long-term dewatering are not required, no impacts are expected from the proposed development.

#### 12.2. Private Water Wells on and near the Site

As discussed in Section 7, there are no MECP water wells recorded for water supply use at the Site. However, an existing water well was observed at 340 Ardagh Road. It is recommended the existing groundwater well be decommissioned in accordance with O.Reg. 903 and Municipal guidelines prior to construction activities at the Site. In addition, the Site and its vicinity are in an urban area in the City of Barrie, where the water supply is provided through the municipal water services. Therefore, no impact would be anticipated on the private water wells or water uses.

## 12.3. Quantity Wellhead Protection Area (WHPA-Q)

The Site is located in The City of Barrie, Wellhead Protection Areas for Quantity (WHPA-Q1 and Q2) with low stress. Given that short- and long-term dewatering are not required, no impacts are expected from the proposed development.

## **12.4.** Wellhead Protection Area -D (WHPA-D)

The Site is located in a WHPA-D area, which indicates a municipal well which is sensitive to contamination, and which are arranged according to either a set distance or delineated based on the time of-travel (up to 25 years) that it would take for water entering the ground to enter the well. The Site is found in a WHPA-D area, which indicates that water and any pollution that may be present can reach the well within 25 years. The site has a WHPA-D score of 2. The site is within a highly vulnerable aquifer (score of 6). The area lies within a chloride contaminant issue contributing area.

It is known that no open water body or creek is located at the Site. Based on the water balance assessment, an infiltration deficit of about 208 m³/year at 334 Ardagh Road and 184 m³/year at 340 Ardagh Road would be estimated due to the proposed development. Therefore, the change in infiltration on Site would be an unfavorable factor that may influence a local municipal well. Significant impact to deeply seated municipal wells is not expected as diversion of roof runoff will compensate the total infiltration deficit, and any dewatering is anticipated to be within the upper aquifer only, and any municipal wells are located beyond 500 m from the Site.

#### 12.5. Local Sewage Works

The storm water runoff generated during the construction may be discharged to the local sewer systems (storm and/or sanitary) on Ardagh Road. As discussed, the storm water runoff generated from the excavation area at the Site may be discharged to the local sewer systems after appropriate treatment. However, this may increase the load to the local sewer systems.

It should be noted that a permit or agreement to use the local sewer system shall be obtained from the City of Barrie prior to treated water discharge.

#### 13.0. CONCLUSIONS AND RECOMMENDATIONS

This report was prepared by SIRATI in support of proposed residential development at 334 and 340 Ardagh Road, City of Barrie, Ontario (the Site). Based on the hydrogeological investigations conducted on the subject Property, the following conclusions are presented:

- The Site is situated within the Middle Nottawasaga River Reach Sub-watershed of the Nottawasaga Valley Watershed under the jurisdiction of Nottawasaga Valley Conservation Authority (NVCA). Bear Creek Central East Branch is located 345 m west of the Site and Henderson Creek is located 305 m east of the site.
- The Site is located within the Simcoe Lowlands, sand plain physiographic region. The Site is covered by Glaciolacustrine coarse-grained sediment, consisting of silt and sand matrix, the overburden thickness in the area of the Site is approximately 170 m. The Site is underlain by the Middle Ordovician, Simcoe Group, Shadow Lake Formation and Lindsay Formation including limestone, dolostone, shale, arkose and sandstone.
- The soils encountered in the boreholes at the Site generally consisted of an upper layer of sandy fill material (1.8 m to a maximum depth of 2.7 m at BH/MW-04) over native soils. Native soils consisted of sand deposits with varied amounts of silt and varied amounts of gravel underlain by lean clay deposits interbedded with silt and sand. No bedrock was encountered at the maximum explored depth of 9.5 mbgs.
- The groundwater levels measured in the monitoring wells across the Site ranged from 4.45 mbgs at BH/MW-01 to 5.70 mbgs at BH/MW-02A, while ground water elevations ranged from 240.74 mAMSL at BH/MW-02A to 242.75 mAMSL at BH/MW-03, with an inferred groundwater flow direction to the north.
- Based on the available information, the above assumptions and preliminary design, short-term dewatering may not be required for 334 and 340 Ardagh Road.
- The stormwater volume estimated to accumulate inside the excavation for 334 and 340 Ardagh Road would be approximately 16,520 L/day for each construction area and totally 33,040 L/day. EASR registration may not be required for construction dewatering.
- The average estimated hydraulic conductivity of the screened soils would be  $1.66 \times 10^{-5}$  m/s.

• Please contact SIRATI for re-evaluation of the construction dewatering analyses and long-term dewatering once the Preliminary design is Finalized (final site grades, finalized u/s of footing

elevations, finalized foundation elevations, storm and septic sewer design).

• Based on the preliminary water balance assessment for 334 Ardagh Road, an infiltration deficit of about 208 m³/year and a runoff increase of 463 m³/year will be anticipated due to the proposed development.

- Based on the preliminary water balance assessment for 340 Ardagh Road, an infiltration deficit
  of about 184 m<sup>3</sup>/year and a runoff increase of 410 m<sup>3</sup>/year will be anticipated due to the proposed
  development.
- Given that the Site is located in WHPA-D and WHPA-Q1/Q2 areas, the dewatering activities and the infiltration deficit would be a threat to the groundwater recharge and influence the municipal wells. To mitigate the infiltration deficit, it is recommended that alternate recharge measures such as 'Permeable Pavement for parking lot, Walkways and roads to allow rainwater to seep through the recharge groundwater to compensate infiltration facilities measures would be incorporated into the design. It should be noted that selection and design of the recharge methods should be carried out by the project engineer. If required, SIRATI can conduct in-situ infiltration tests (Guelph Test) to assess the soil infiltration capacity.
- As the Site is located in WHPA-D and WHPA-Q1/Q2 area, a source protection plan may be prepared to address or discuss the activities occurring on the Site.
- One (1) domestic type, metal cased potable groundwater well was identified at the Site (at 340 Ardagh Road). No well tag was observed to be attached to the well and no MECP water well record was located for this well. This well is recommended to be decommissioned in accordance with O.Reg. 903 and local Municipal guidelines.
- Please contact SIRATI if you require a door-to-door well survey for the proposed development.

#### 14.0. SELECTED BIBLIOGRAPHY

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Groundwater Lowering In Construction: A Practical Guide To Dewatering. 2<sup>nd</sup> Edition. Boca Raton: CRC Press, 2013. Print.

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Surficial Geology of Southern Ontario; Ontario Ministry of Northern Development, Mines and Forestry; http://www.mndmf.gov.on.ca/mines/ogs\_earth\_e.asp; 2010

Bedrock Geology; Ontario Ministry of Northern Development, Mines and Forestry; <a href="http://www.mndmf.gov.on.ca/mines/ogs\_earth\_e.asp">http://www.mndmf.gov.on.ca/mines/ogs\_earth\_e.asp</a>; 2010

City of Barrie Sewer Use By-Law 2021-002

Topographic map generator (<a href="https://atlas.gc.ca">https://atlas.gc.ca</a>)

MECP Source Protection Information Atlas (Web Mapping, <a href="https://www.gisapplication.Irc.gov.on.ca">https://www.gisapplication.Irc.gov.on.ca</a>)

Nottawasaga Valley Conservation Authority (Web Mapping, <a href="https://www.nvca.on.ca">https://www.nvca.on.ca</a>)

Ministry of the Environment, Conservation and Parks (MECP), Map: Well Records – Ontario.ca (<a href="https://www.ontario.ca">https://www.ontario.ca</a>)

#### 15.0. LIMITATIONS AND USE OF THE REPORT

This report was produced by SIRATI for the sole use of the Client for the Site and may not be relied upon by any other person or entity without the written authorization of SIRATI. The conclusions presented in this report are professional opinions based on the historical and current records search, visual observations and limited information provided by persons knowledgeable about past and current activities on this site. As such, SIRATI cannot be held responsible for environmental conditions at the Property that was not apparent from the available information. No investigation method can completely eliminate the possibility of obtaining partially imprecise or incomplete information; it can only reduce the possibility to an acceptable level.

Professional judgement was exercised in gathering and analyzing data and formulation of recommendations using current industry guidelines and standards. Similar to all professional persons rendering advice, SIRATI cannot act as absolute insurer of the conclusion we have reached. No additional warranty or representation, expressed or implied, is included or intended in this report other than stated herein the report.

The assessment should not be considered a comprehensive audit that eliminates all risks of encountering environmental problems. The information presented herein this report is primarily based on information collected during the hydrogeological study based on the condition of the Property at the time of site inspection/drilling followed by a review of historical data, as appended to this report.

In assessing the environmental setting of the Property, SIRATI has solely relied upon information supplied by others in good faith and has therefore assumed that the information supplied is factual and accurate. We accept no responsibility for any inaccurate information, misrepresentation or for any deficiency of the information supplied by any third party.

The scope of services performed in the execution of this investigation may not be appropriate to satisfy third parties. SIRATI accepts no responsibility for damages if any, suffered by any third party as a result of decisions made or action taken based on this report. Any use, copying or distribution of the report in whole or in part is not permitted without the express written permission of SIRATI and use of findings, conclusions and recommendations represented in this report, is at the sole risk of third parties.

In the event that during future work new information regarding the environmental condition of the Property is encountered, or in the event that the outstanding responses from the regulatory agencies indicate outstanding issues on file with respect to the Property, SIRATI should be notified in order that we may re-evaluate the findings of this assessment and provide amendments, as required.

### 16.0. SIGNATURES

Should you have any questions regarding the information presented or limitation set in this report, please do not hesitate to contact our office.

Yours truly,

Sirati and Partners Consultants Ltd.

Prepared by:

Hiva Elhami, M.Sc., G.I.T.

Junior Hydrogeologist

Reviewed by:

Praharsh Dhyani, M. Sc., P. Geo Hydrogeology & Environmental

Division Lead

pproved by: SR

Archie Sirgui, Ph.D., P.E.

Principal

# **FIGURES**





& PARTNERS

160 Konrad Crescent Markham, ON. L3R 9T9 Phone# 905 940 1582, Fax# 905 940 2440



Legend:

Approximate Property Boundary

# Project Title:

Hydrogeological Investigations

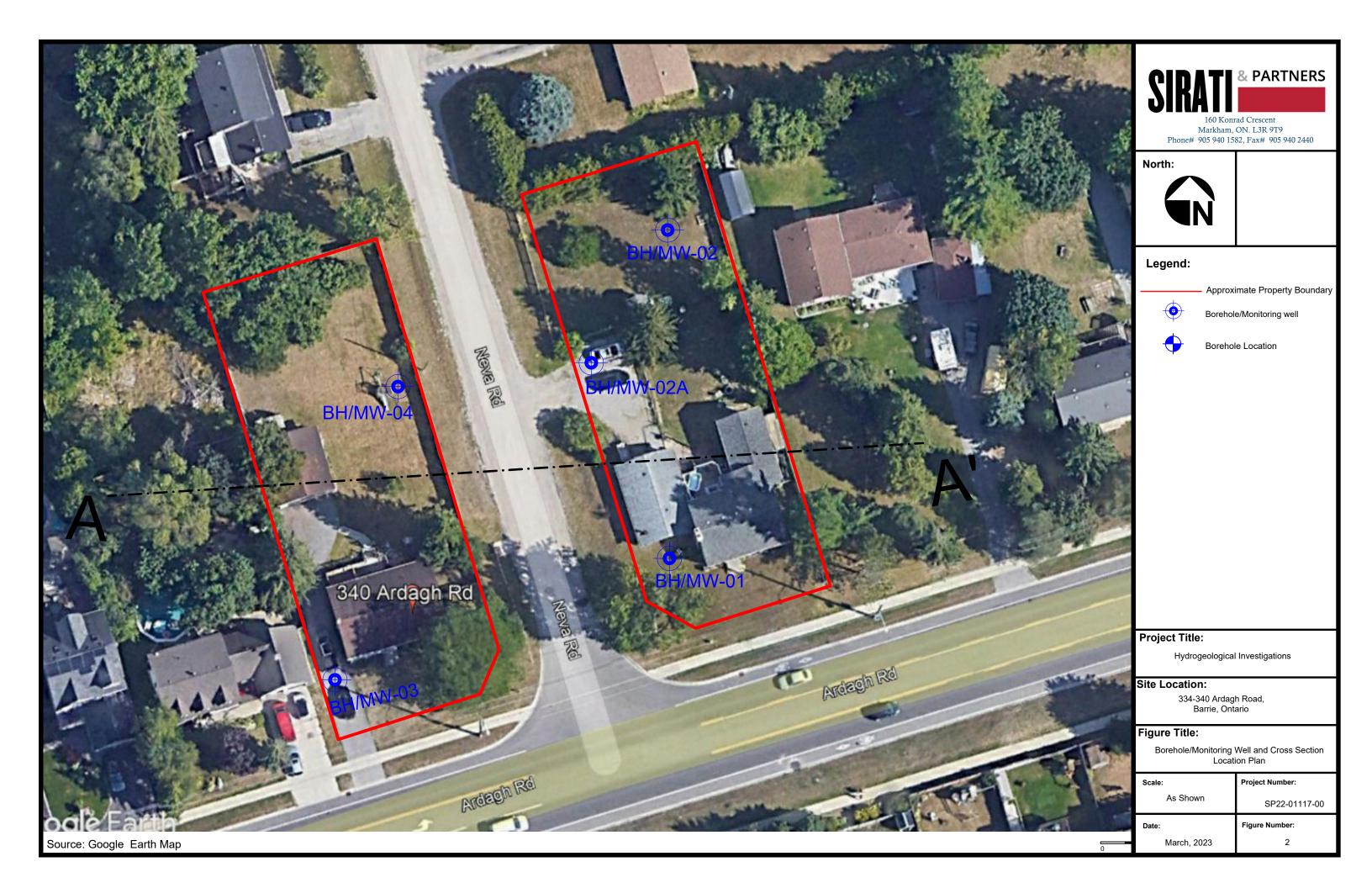
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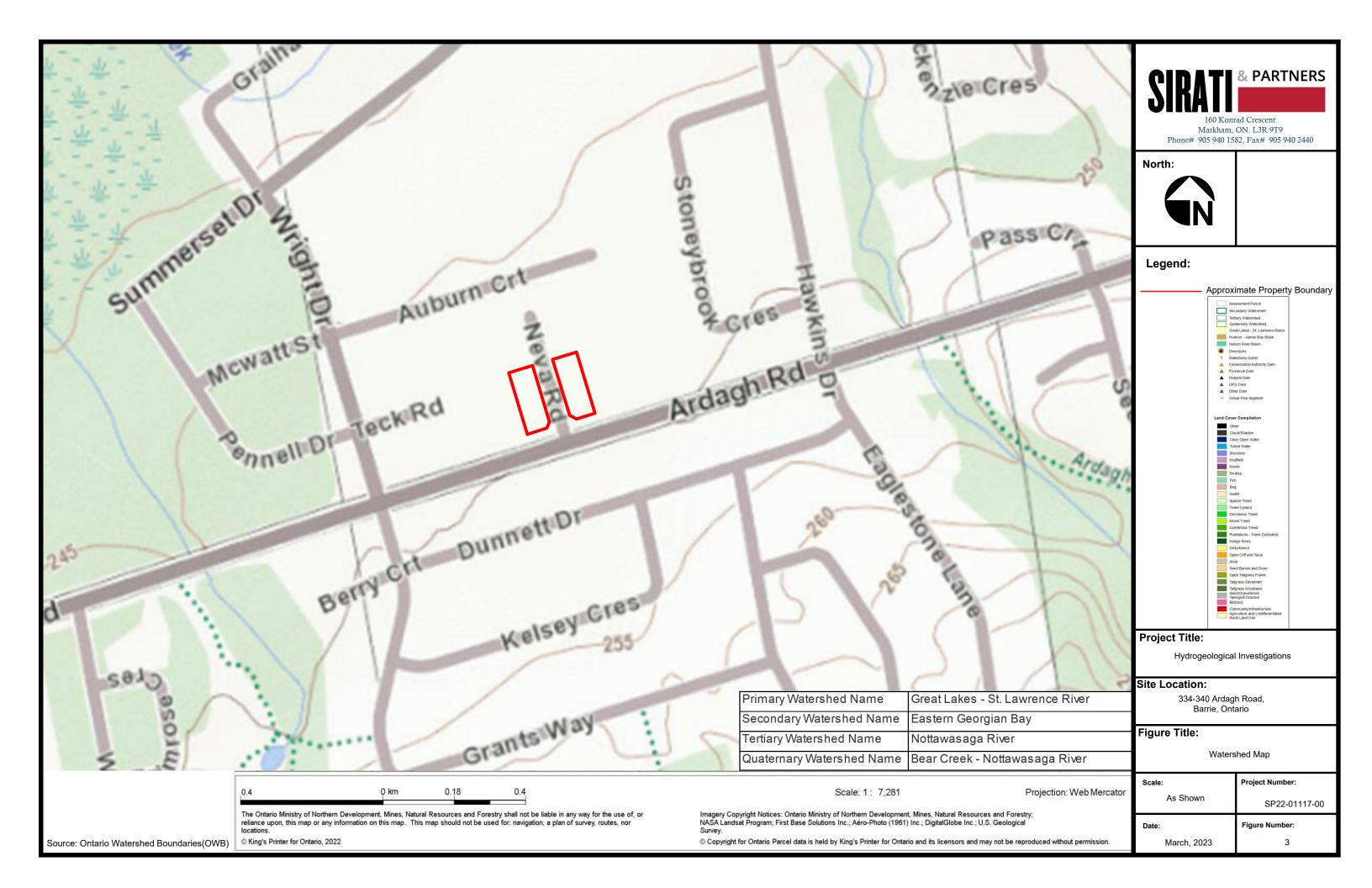
334-340 Ardagh Road, Barrie, Ontario

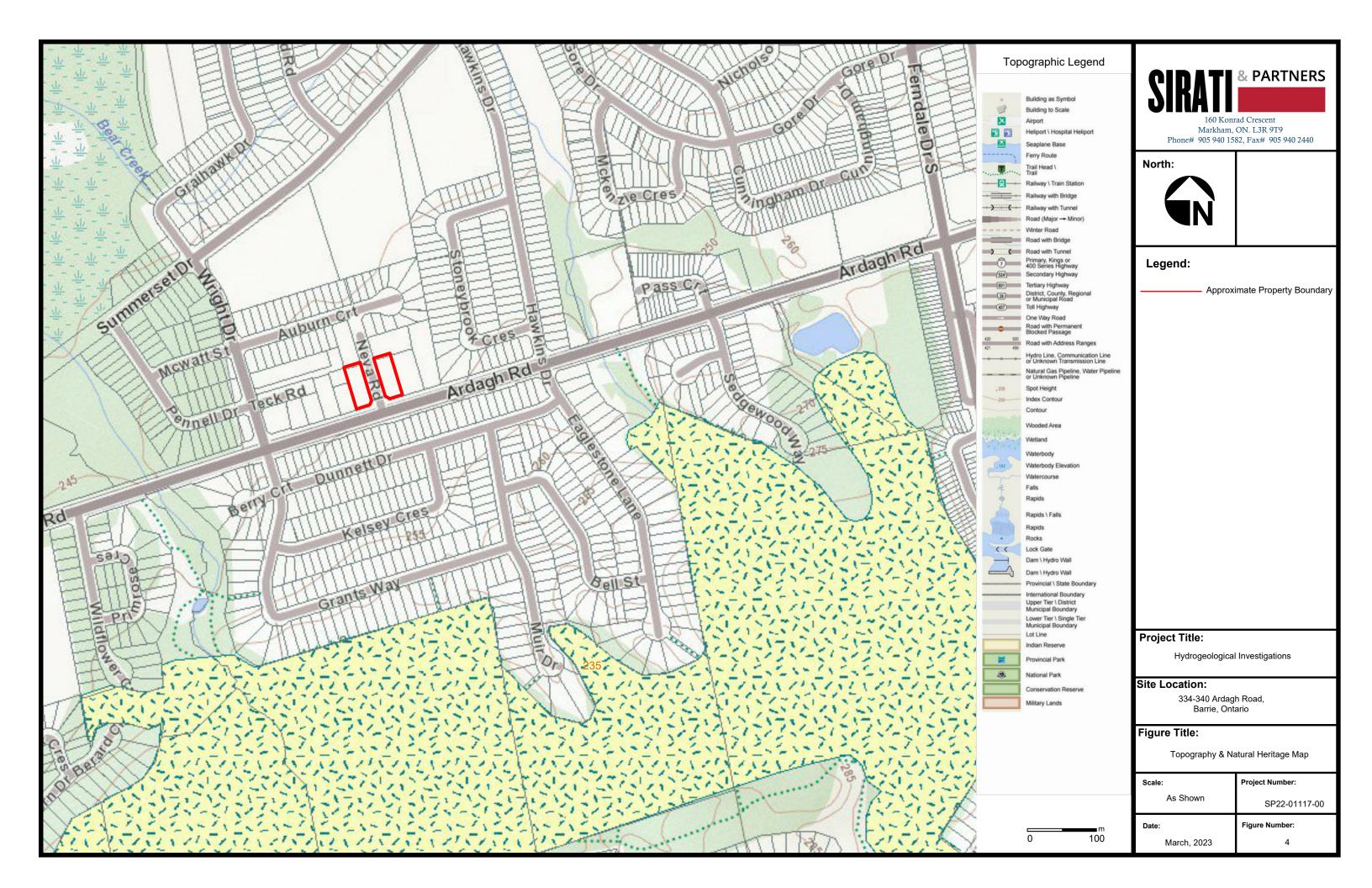
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Site Location Plan

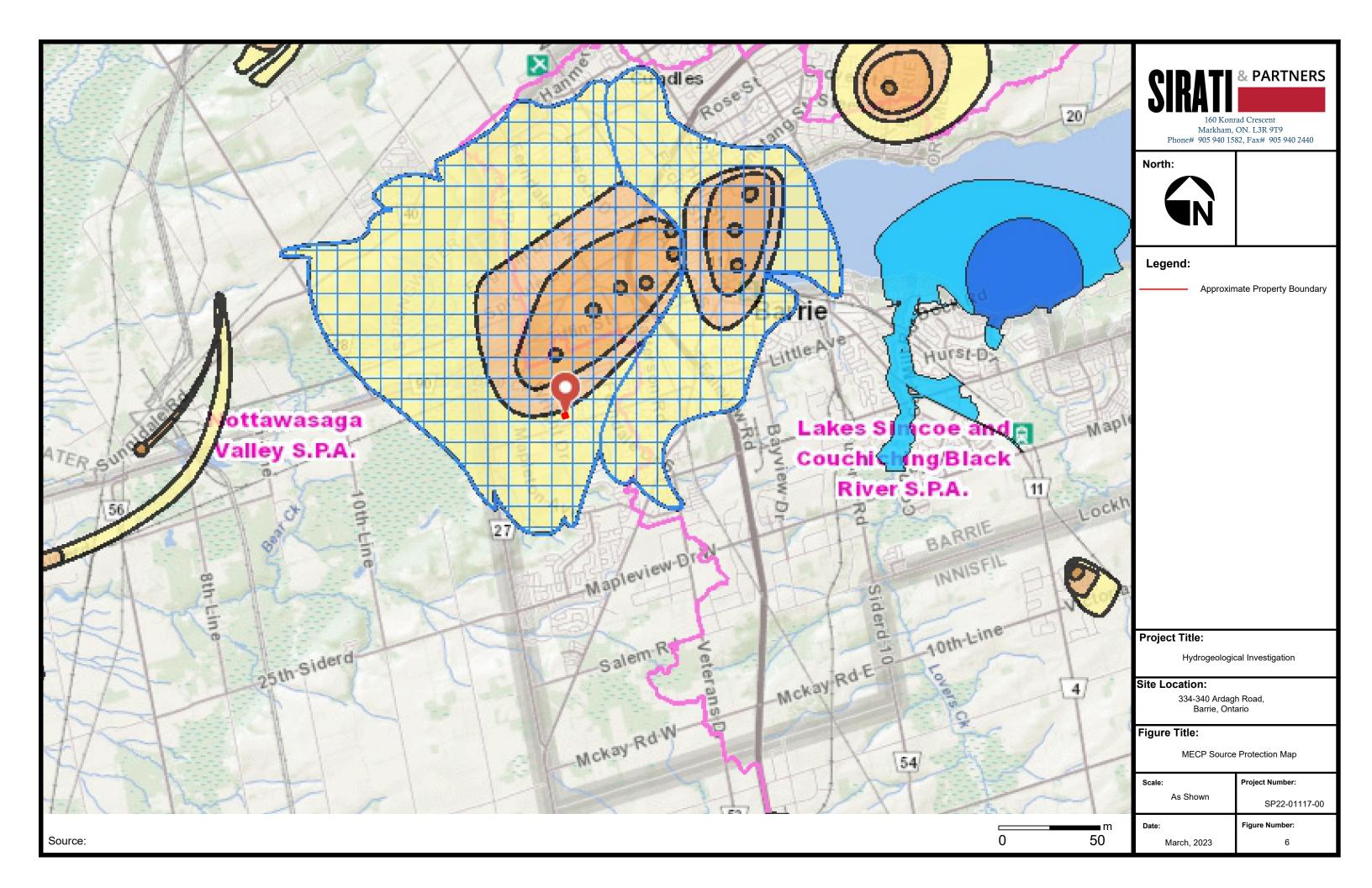
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	As Shown	SP22-01117-00
N	Date:	Figure Number:



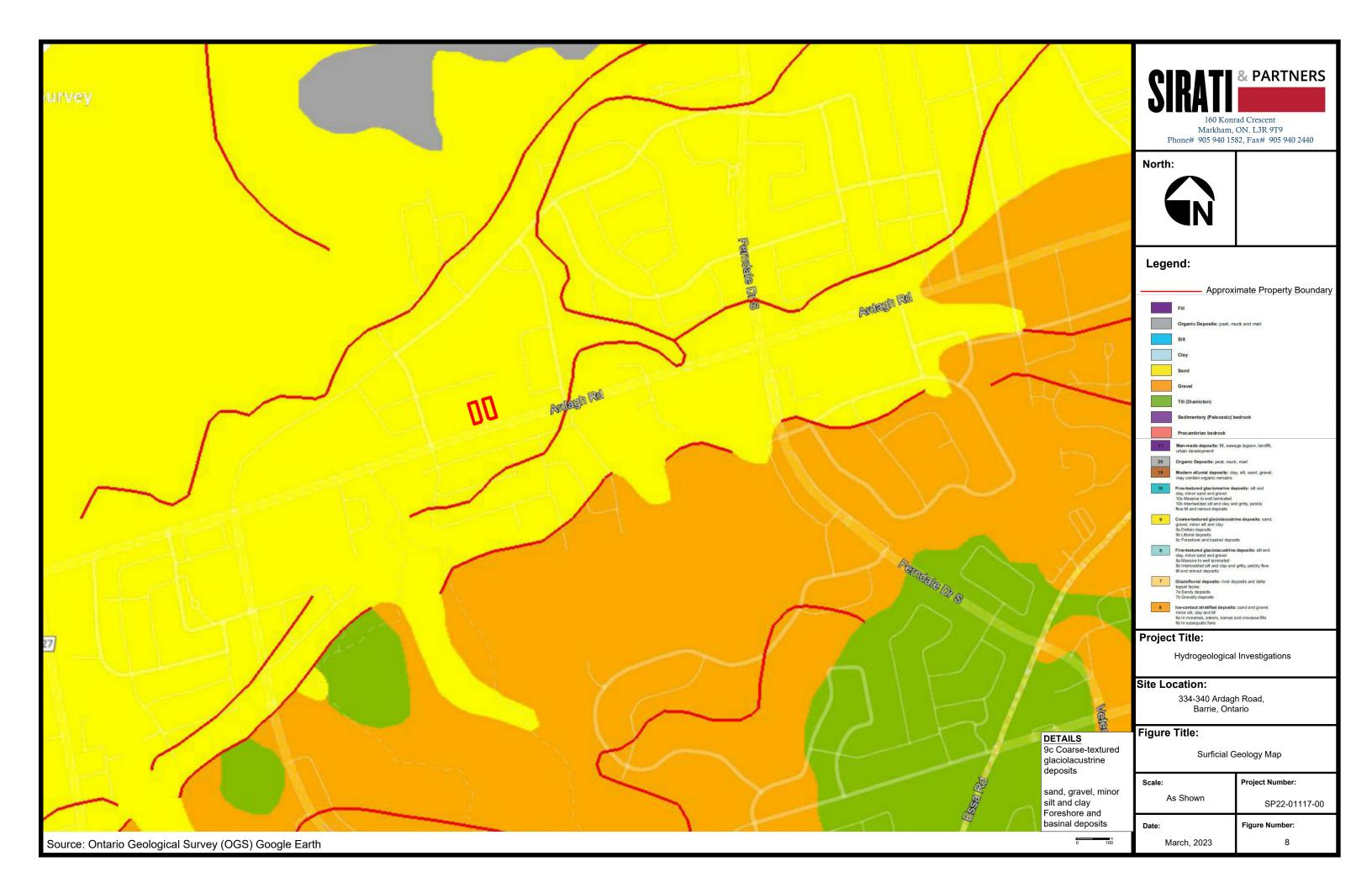


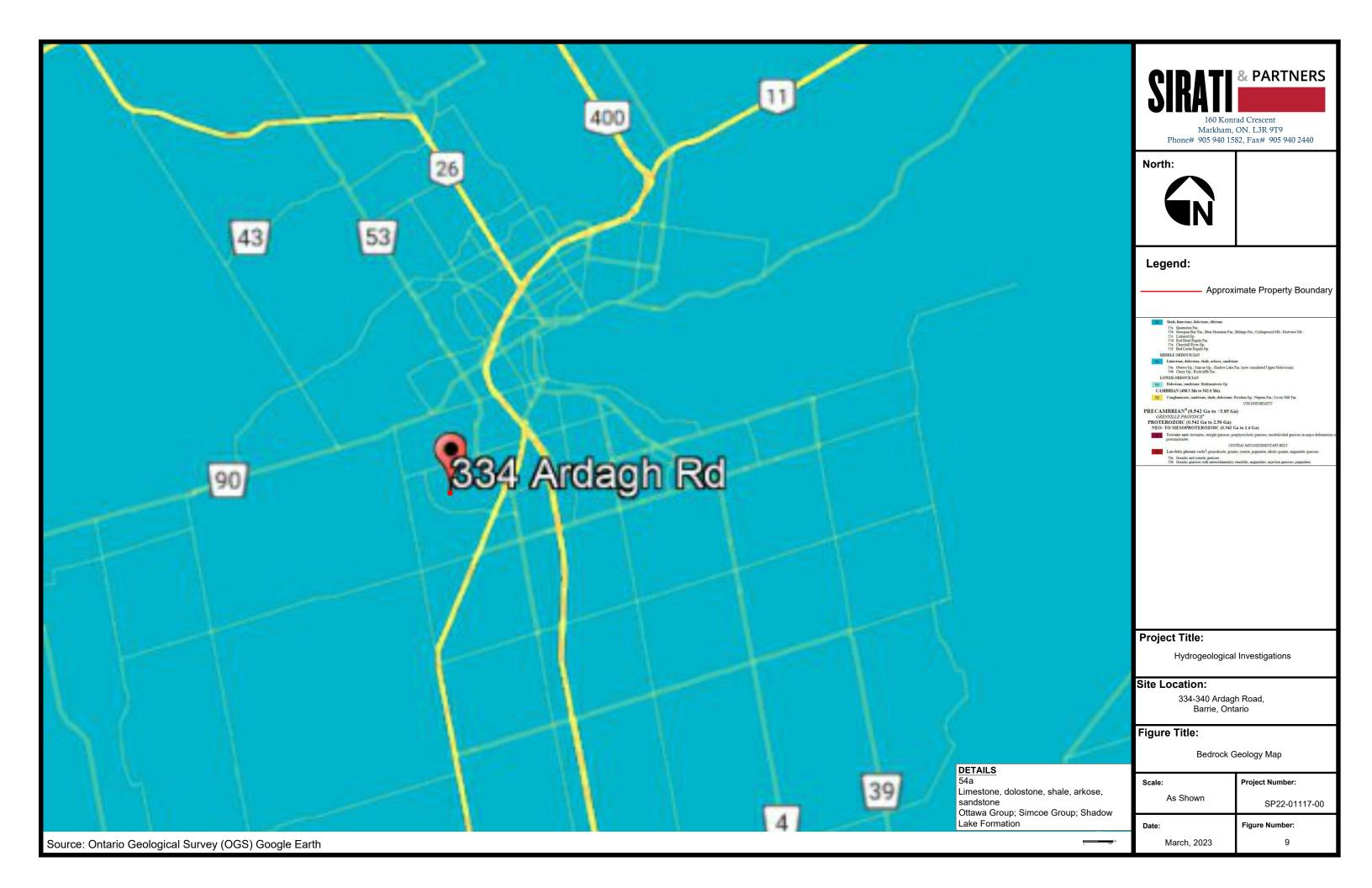


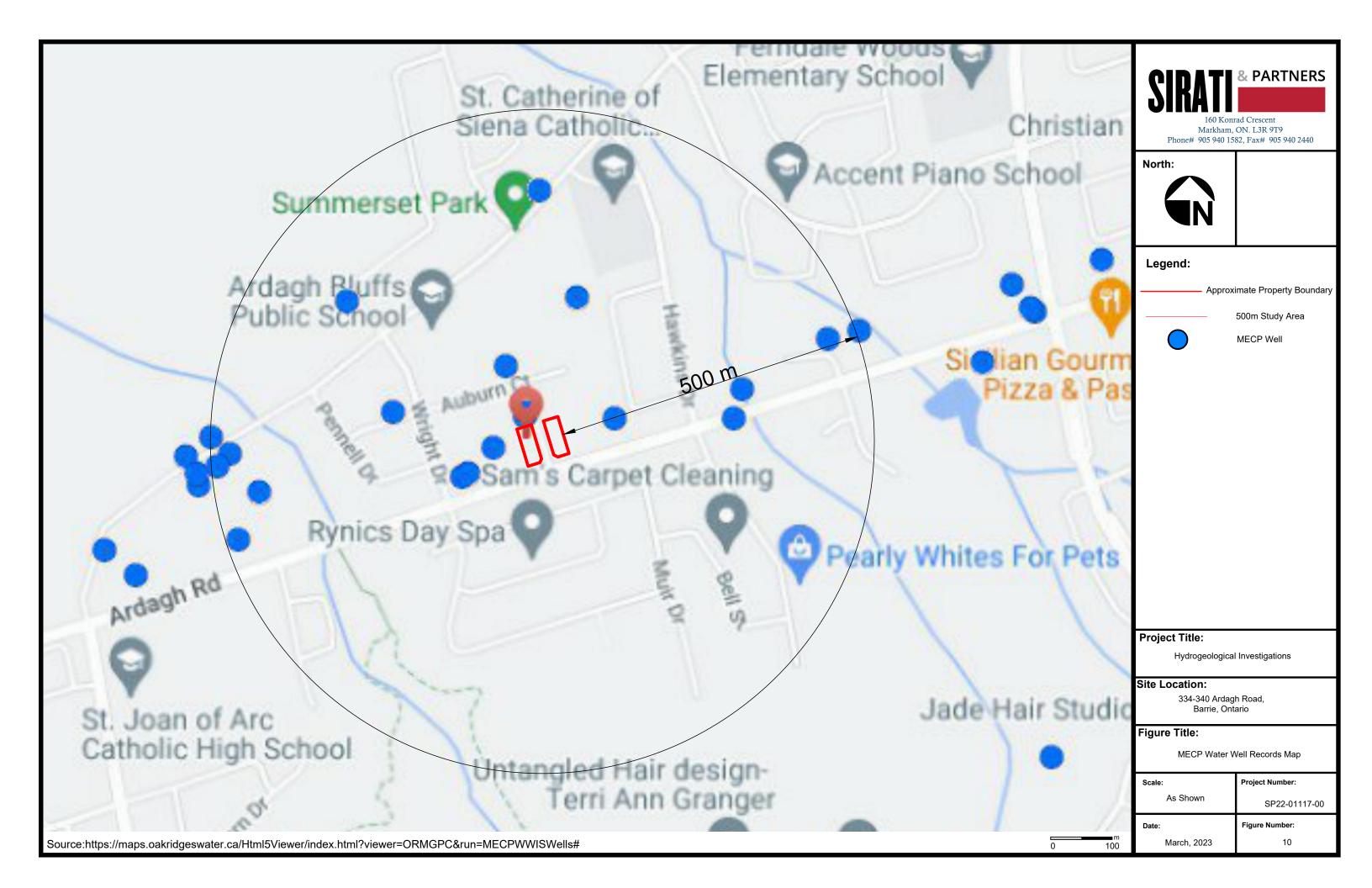


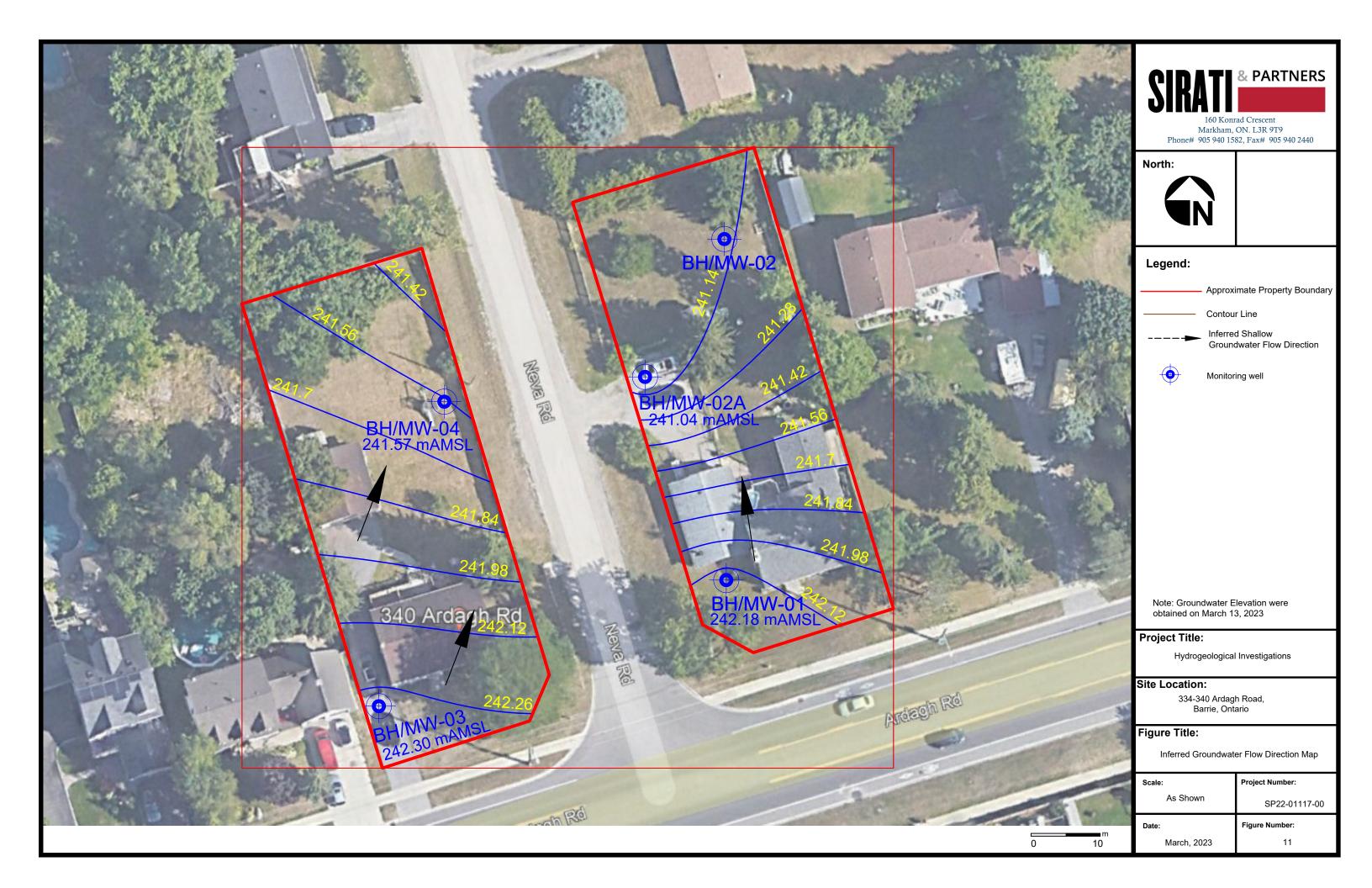


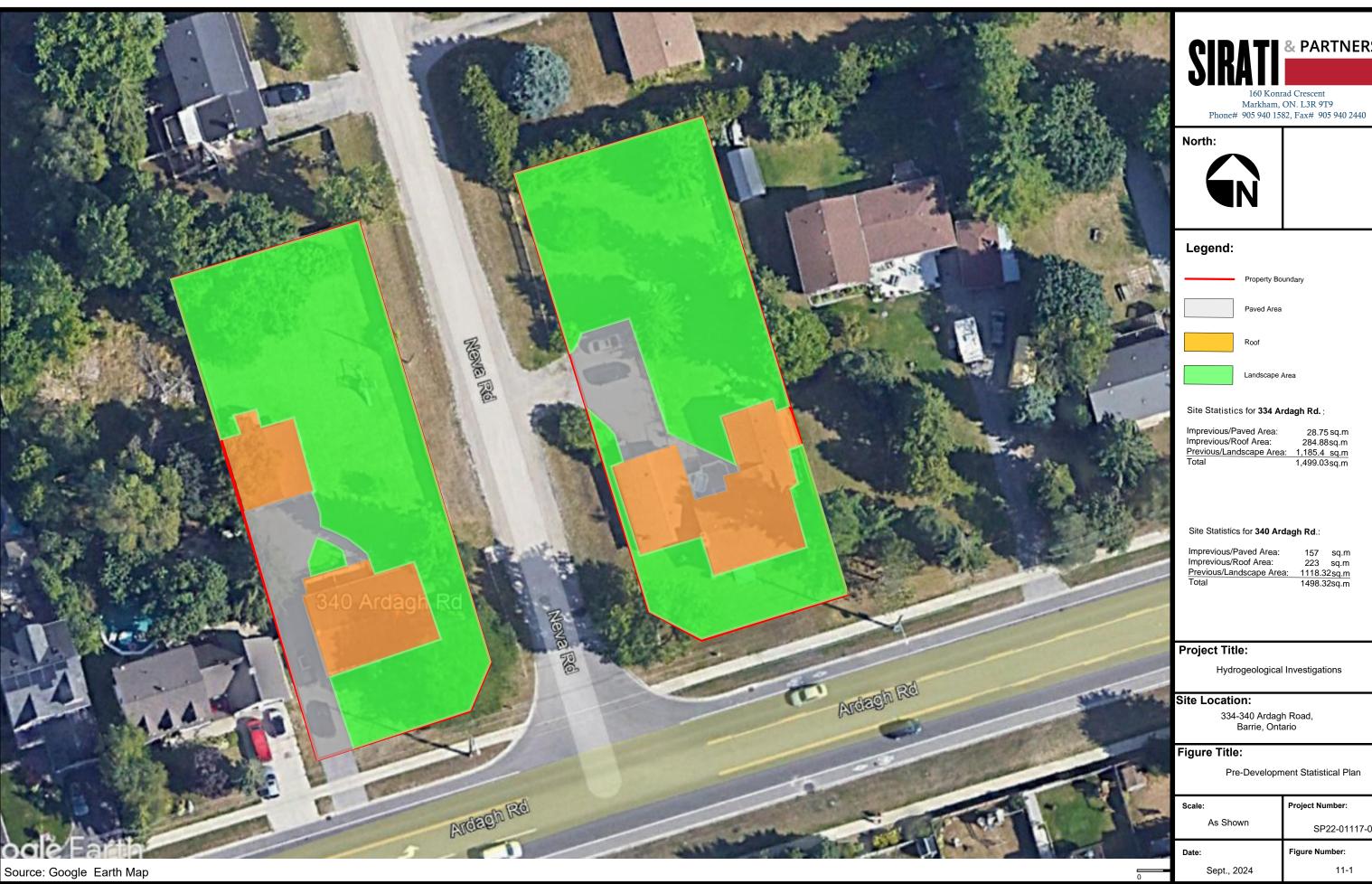












**& PARTNERS** 160 Konrad Crescent

Markham, ON. L3R 9T9



Property Boundary

Paved Area

Landscape Area

### Site Statistics for 334 Ardagh Rd.:

Imprevious/Roof Area: 284.88sq.m Previous/Landscape Area: 1,185.4 sq.m Total 1,499.03sq.m

### Site Statistics for 340 Ardagh Rd.:

Imprevious/Paved Area: Imprevious/Roof Area: Imprevious/Paved Area:157sq.mImprevious/Roof Area:223sq.mPrevious/Landscape Area:1118.32sq.mTotal1498.32sq.m

Hydrogeological Investigations

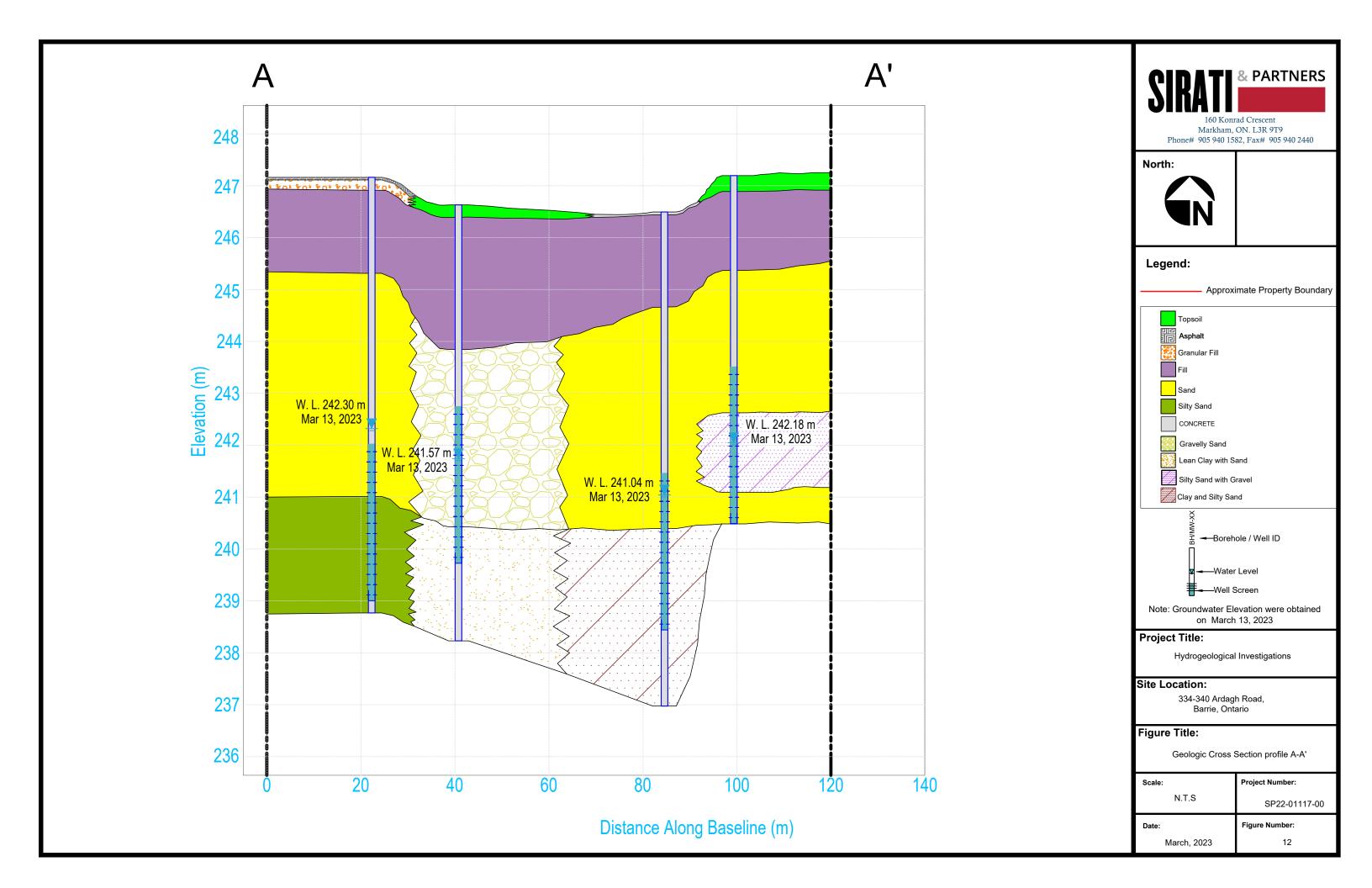
334-340 Ardagh Road, Barrie, Ontario

Pre-Development Statistical Plan

Scale:	Project Number:
As Shown	SP22-01117-00

Figure Number:

11-1

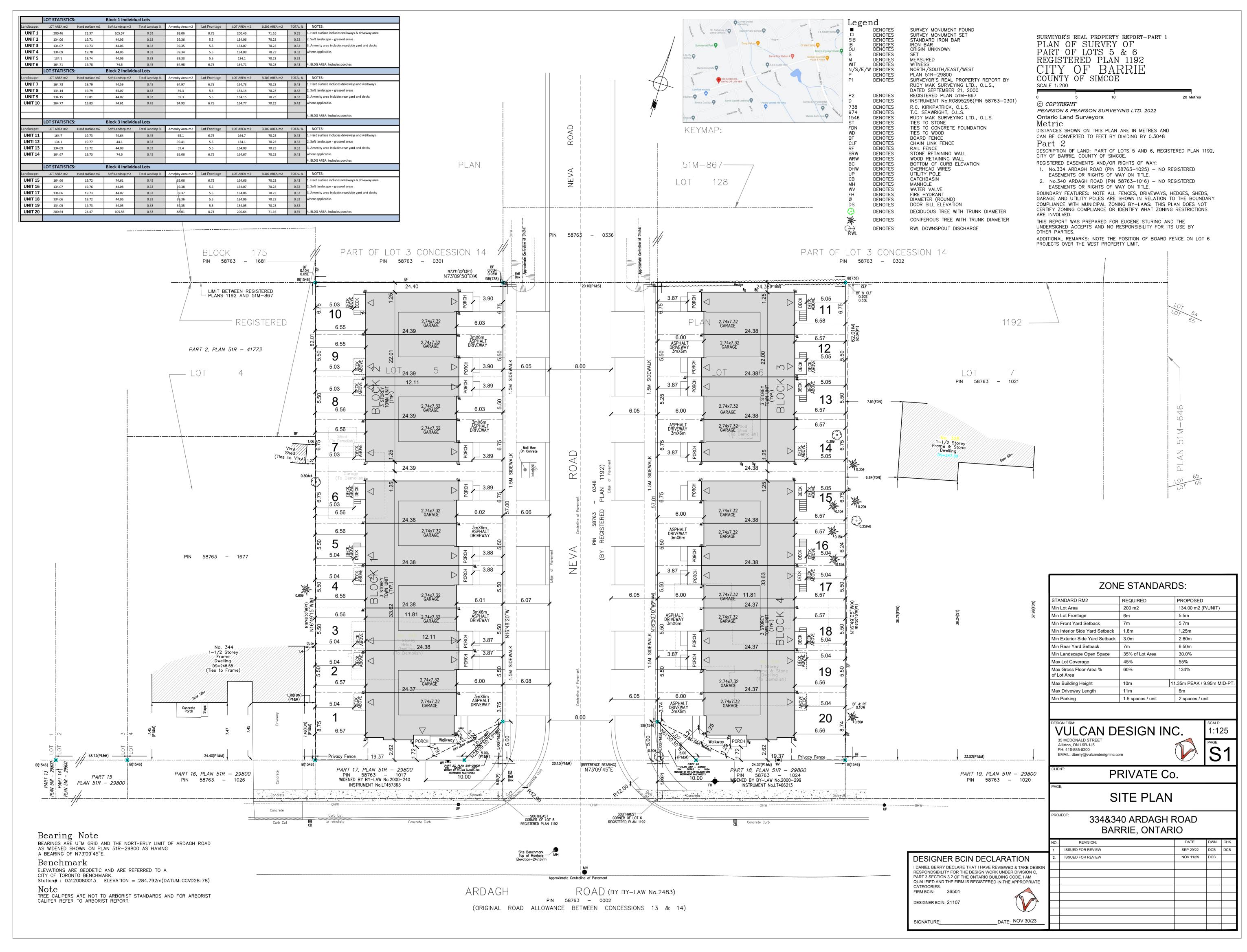


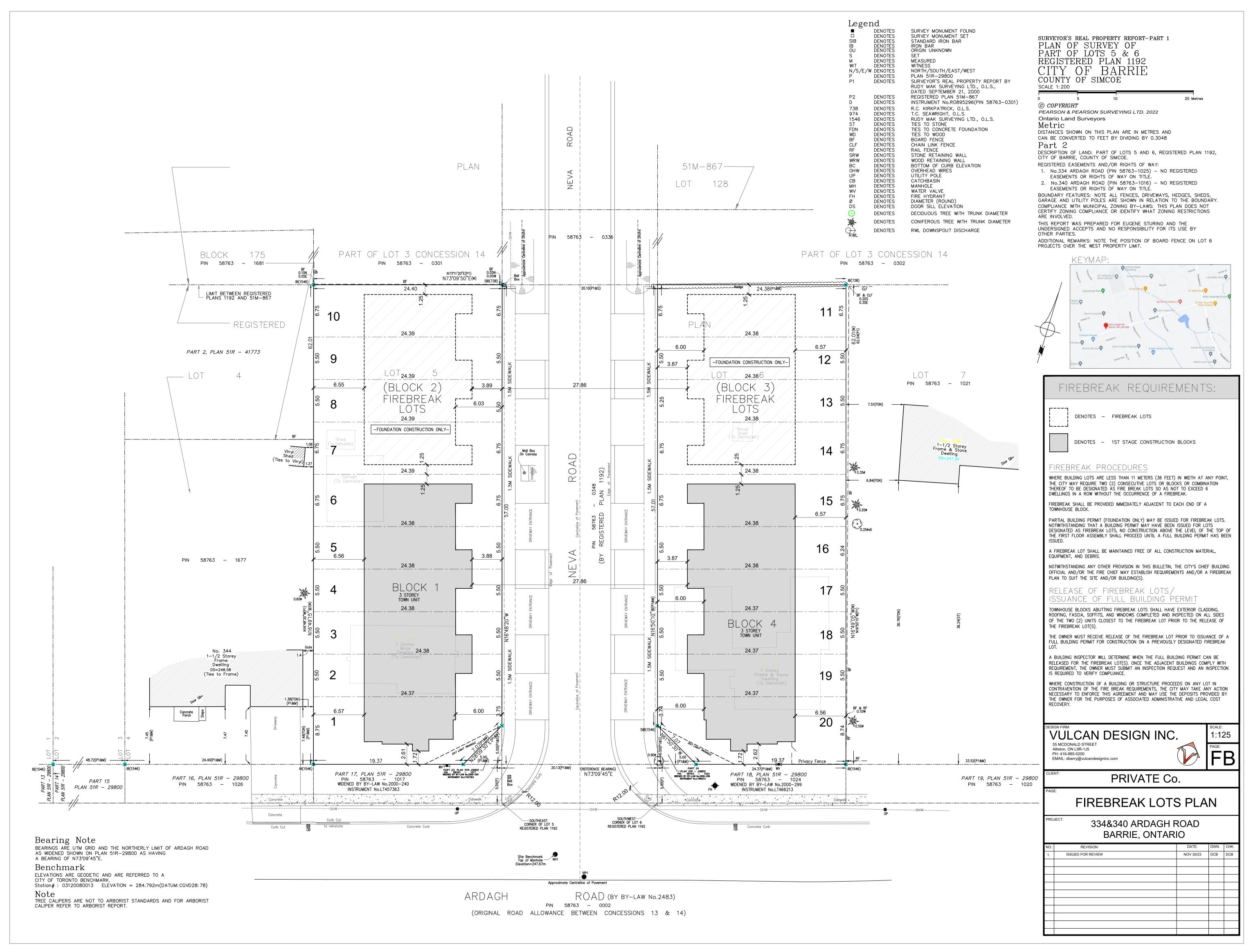
# **APPENDICES**



# APPENDIX A









35 MCDONALD STREET ALLISTON, ON L9R 1J5

PH: 416-885-5200 EMAIL: dberry@vulcandesigninc.com
CONTACT PERSON: DANIEL BERRY

### BCIN DECLARATION:

I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES.

FIRM BCIN: 36501 DESIGNER BCIN: 21107

SIGNATURE: \_ DATE: \_\_\_\_\_

# LEGEND / LINTELS:

	LLOLIND	LIIVILLO.
LIN	ITELS:	ABBREVIATIONS:
<u>WC</u>	OOD	
W1	2/2"X8"	DJ DOUBLE JOIST
W2		TJ TRIPLE JOIST
_	3 2/2"X12"	GT GIRDER TRUSS
W4 W5		'DO' DO OVER
W6		RJ ROOF JOISTS
LVL	. (2.0E)	STL. STEEL
3-LV 2-LV 3-LV 2-LV	'L 9 2- 1 3/4" X 9-1/2" 'L 9 3- 1 3/4" X 9-1/2" 'L 11 2- 1 3/4" X 11-7/8"	SB. SOLID BEARING (SEE 'FRAMING REQ.'S') PL. POINT LOAD (FL.) FLUSH
	'L 11	(DR.) DROPPED
	'L 14 3- 1 3/4" X 14" O. FASTEN MULTIPLE PLY LVL'S	FG. FIXED GLASS
	COWS @ 12" O.C. $3\frac{1}{2}$ " (16d) NAILS SDW SCREWS	FDTN. FOUNDATION
STE	EEL LINTELS	BG. BLACK GLASS
S1	<4'-0" L3-1/2" X 3-1/2" - 1/4"	JT JACK TRUSS
S2	< 6'-0" L4" X 3-1/2" X 1/4"	U/S UNDERSIDE
S3 S4	< 8'-0" L5" X 3-1/2" X 5/16" < 10'-0" L6" X 3-1/2" X 5/16"	T/O TOP OF
	AR LINTELS MIN. 6" EACH END FELS SHALL BE PRIMED TO	FG. FIXED GLASS
	C/CPMA 2-3" ENT ANGLES LONG LEG VERT.	CLG. CEILING
-MA>	K. 1" BRICK OVERHANG EP HOLES @ 56" O.C. MAX	BBFM BEAM BY FLOOR MANUFACTURE
₩ <sup>SB</sup>	SOLID BEARING	© GAS LINE
	SUPPORTED MEMBER)	CABLE LINE
⊗ PL	CONCENTRATED	PHONE JACK
	FIRE PLACE VENT	CEILING EXHAUST FAN
	DRYER VENT	c. PULL CHAIN CLG. LIGH
	STOVE VENT	CEILING LIGHT
	CELLAR VENT	) JEIEN GEIGH

3 WAY SWITCH

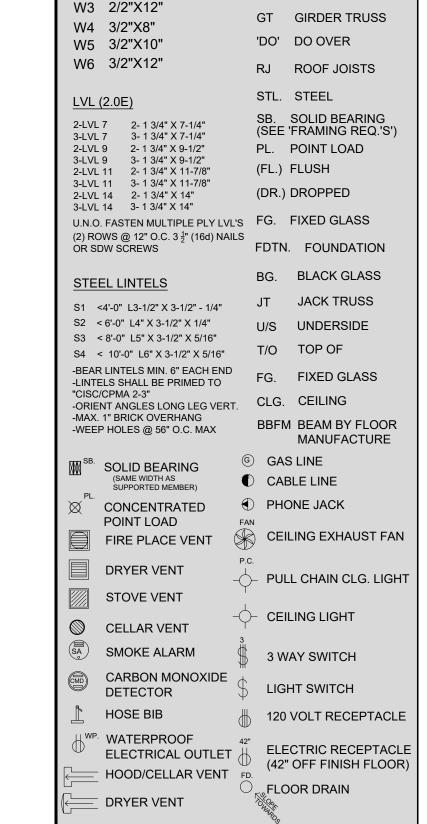
LIGHT SWITCH

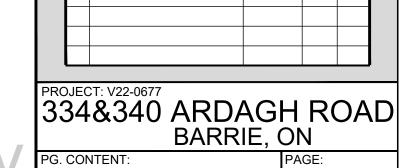
FLOOR DRAIN

DATE: DWN. CHK

MAY04/23 dcb dcb NOV30/23 dcb

120 VOLT RECEPTACLE

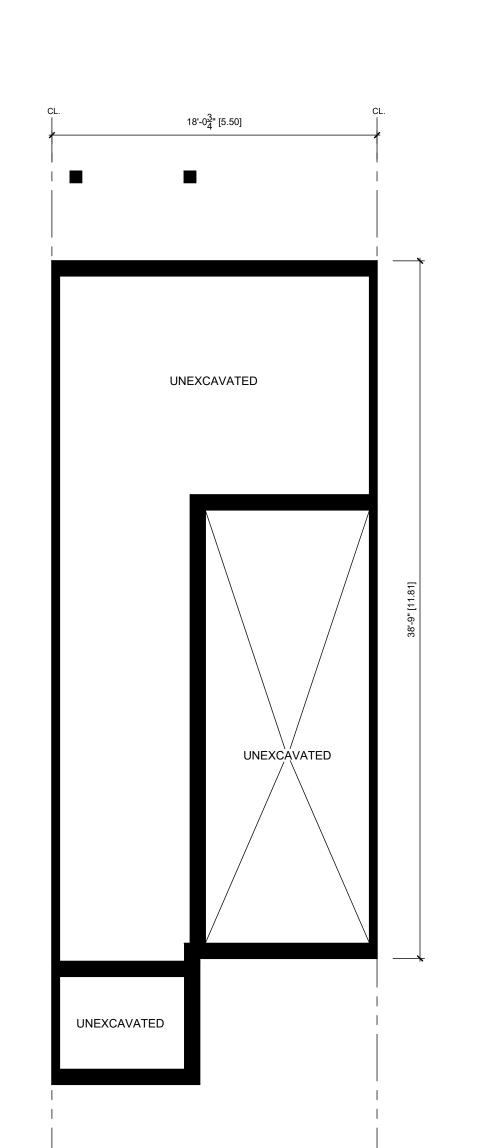




Issued for review

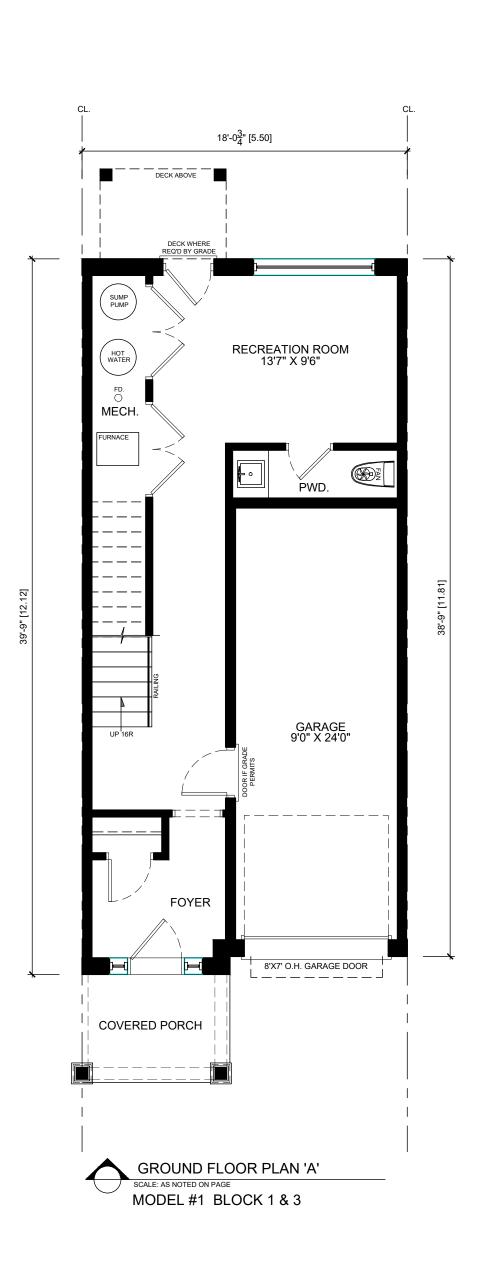
Issued for review

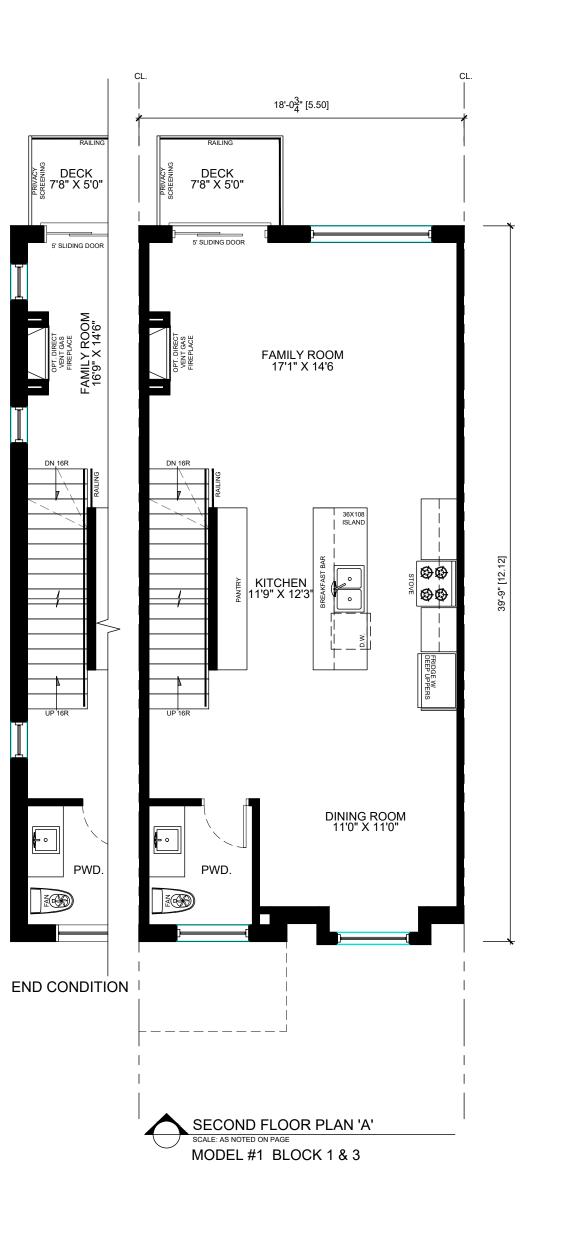
Preliminary
PG. CONTENT:
FLOOR PLANS
SCALE: 3/16" = 1'-0"

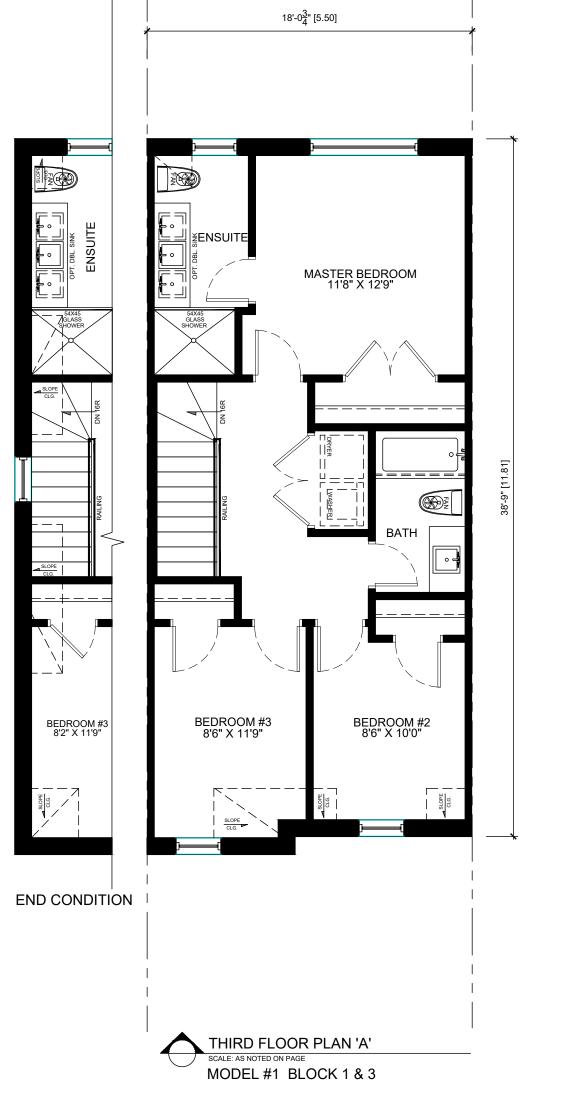


FOUNDATION PLAN 'A'
SCALE: AS NOTED ON PAGE

MODEL #1 BLOCK 1 & 3







AREA CALCULATIONS GROUND sf 470 m2 43.663 | SECOND | sf | 715 | m2 | 66.424 |
THIRD	sf	708	m2	65.773
- O.T.B.	sf	0	m2	0.000
TOTAL	sf	1893	m2	175.860
FIN BASEMENT	sf	0	m2	0.000
TOTAL	sf	1893	m2	175.860

GARAGE sf 237 m2 22.017

FRONT PORCH sf 49 m2 4.552

REAR PORCH sf 0 m2 0.000

COV. W/O PORCH sf 707 m2 65.680

COV. W/ PORCH sf 756 m2 70.232

18'-0<mark>3</mark>" [5.50]

UNEXCÁVATED

FOUNDATION PLAN 'A' & 'B'
SCALE: AS NOTED ON PAGE

MODEL #3 BLOCK 1 & 4

UNEXCAVATED

UNEXC.

### VULCAN DESIGN INC. Professional Service, Exceptional Value

35 MCDONALD STREET ALLISTON, ON L9R 1J5 PH: 416-885-5200

EMAIL: dberry@vulcandesigninc.com CONTACT PERSON: DANIEL BERRY

### **BCIN DECLARATION:**

I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES.

FIRM BCIN: 36501 DESIGNER BCIN: 21107

\_ DATE: \_\_\_\_\_ SIGNATURE:

LEGEND / LINTELS:

LINTELS: ABBREVIATIONS: WOOD DJ DOUBLE JOIST W1 2/2"X8" W2 2/2"X10" TJ TRIPLE JOIST W3 2/2"X12" GT GIRDER TRUSS W4 3/2"X8" 'DO' DO OVER W5 3/2"X10" W6 3/2"X12" RJ ROOF JOISTS STL. STEEL LVL (2.0E) SB. SOLID BEARING (SEE 'FRAMING REQ.'S') 2-LVL 7 2- 1 3/4" X 7-1/4"
3-LVL 7 3- 1 3/4" X 7-1/4"
2-LVL 9 2- 1 3/4" X 9-1/2"
3-LVL 9 3- 1 3/4" X 9-1/2"
2-LVL 11 2- 1 3/4" X 11-7/8"
3-LVL 11 3- 1 3/4" X 11-7/8"
2-LVL 14 2- 1 3/4" X 14"
3-LVL 14 3- 1 3/4" X 14" PL. POINT LOAD (FL.) FLUSH

(DR.) DROPPED U.N.O. FASTEN MULTIPLE PLY LVL'S FG. FIXED GLASS
(2) ROWS @ 12" O.C. 3 ½" (16d) NAILS
OR SDW SCREWS FDTN. FOUNDATION FDTN. FOUNDATION

BG. BLACK GLASS STEEL LINTELS JT JACK TRUSS S1 <4'-0" L3-1/2" X 3-1/2" - 1/4" S2 < 6'-0" L4" X 3-1/2" X 1/4" S3 < 8'-0" L5" X 3-1/2" X 5/16" U/S UNDERSIDE T/O TOP OF S4 < 10'-0" L6" X 3-1/2" X 5/16" -BEAR LINTELS MIN. 6" EACH END FG. FIXED GLASS

-LINTELS SHALL BE PRIMED TO "CISC/CPMA 2-3" -ORIENT ANGLES LONG LEG VERT. CLG. CEILING
-MAX. 1" BRICK OVERHANG BBFM BEAM BY FLOOR -WEEP HOLES @ 56" O.C. MAX MANUFACTURE G GAS LINE SB. SOLID BEARING
(SAME WIDTH AS SUPPORTED MEMBER)

CABLE LINE PHONE JACK CONCENTRATED POINT LOAD CEILING EXHAUST FAN FIRE PLACE VENT

STOVE VENT 
← CEILING LIGHT CELLAR VENT

SMOKE ALARM 3 WAY SWITCH CARBON MONOXIDE LIGHT SWITCH DETECTOR HOSE BIB 120 VOLT RECEPTACLE

WP. WATERPROOF
ELECTRICAL OUTLET

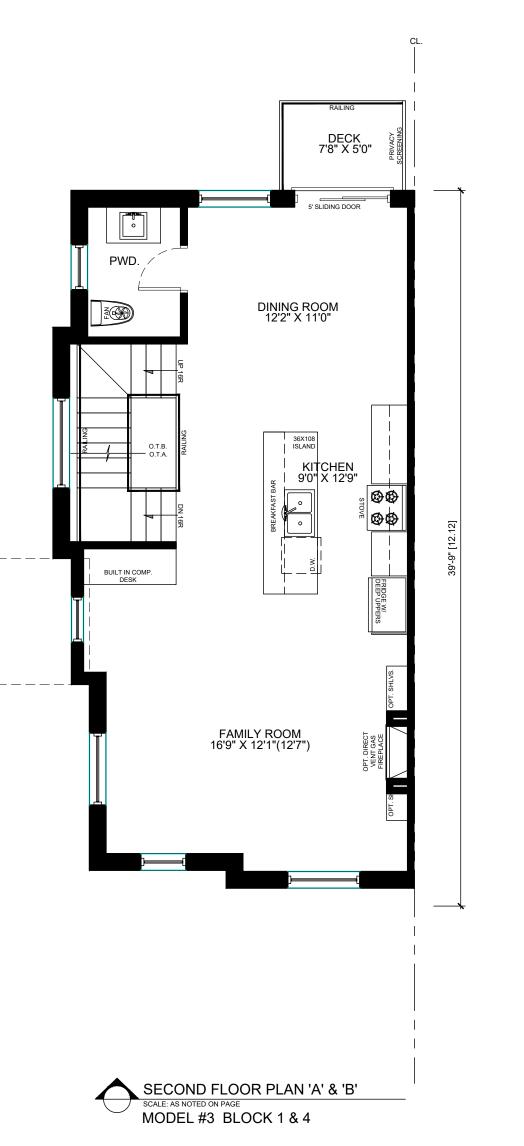
ELECTRIC RECEPTACLE (42" OFF FINISH FLOOR) HOOD/CELLAR VENT FLOOR DRAIN

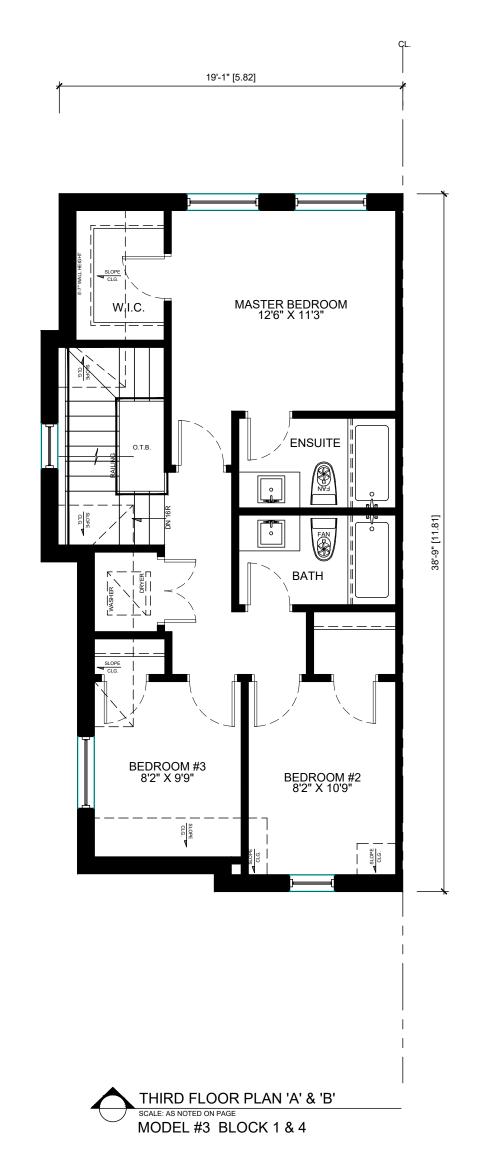
DRYER VENT NO: REVISION: DATE: DWN. CHK Issued for review

NOV14/22 dcb dck NOV30/23 dcb Issued for review

PROJECT: V22-0677 334&340 ARDAGH ROAD BARRIE, ON







EΑ	CALCULATIO	NS	
sf	493	m2	45.800
sf	732	m2	68.003
sf	726	m2	67.445
sf	25	m2	2.323
sf	1926	m2	178.925
sf	0	m2	0.000
sf	1926	m2	178.925
RAG	E CALCULAT	IONS	5
sf	493	m2	45.800
sf	238	m2	22.110
sf	35	m2	3.252
sf	0	m2	0.000
sf	731	m2	67.910
sf	766	m2	71.161
	sf sf sf sf sf sf sf sf	sf 493 sf 732 sf 726 sf 25 sf 1926 sf 0 sf 1926  RAGE CALCULAT sf 493 sf 238 sf 35 sf 0 sf 731	sf         732         m2           sf         726         m2           sf         25         m2           sf         1926         m2           sf         0         m2           sf         1926         m2           RAGE CALCULATIONS         sf         493         m2           sf         238         m2           sf         35         m2           sf         35         m2           sf         731         m2

Preliminary
PG. CONTENT:
FLOOR PLANS
SCALE: 3/16" = 1'-0"

### VULCAN DESIGN INC. Professional Service, Exceptional Value

35 MCDONALD STREET

ALLISTON, ON L9R 1J5

EMAIL: dberry@vulcandesigninc.com CONTACT PERSON: DANIEL BERRY

PH: 416-885-5200

## BCIN DECLARATION:

I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES.

FIRM BCIN: 36501 DESIGNER BCIN: 21107

SIGNATURE: \_ DATE: \_\_\_\_\_

# ELS:

LEGEND /	LINT	ELS:
LINTELS:	ABBRE	VIATIONS:
WOOD		
W1 2/2"X8"	DJ D	OUBLE JOIST
W2 2/2"X10"	TJ T	RIPLE JOIST
W3 2/2"X12"	GT G	IRDER TRUSS
W4 3/2"X8" W5 3/2"X10"	'DO' D	O OVER
W6 3/2"X12"	RJ R	OOF JOISTS
LVL (2.0E)	STL. S	TEEL
2-LVL 7 2- 1 3/4" X 7-1/4" 3-LVL 7 3- 1 3/4" X 7-1/4" 2-LVL 9 2- 1 3/4" X 9-1/2" 3-LVL 9 3- 1 3/4" X 9-1/2"	(SEE 'FF	OLID BEARING RAMING REQ.'S DINT LOAD
2-LVL 11 2- 1 3/4" X 11-7/8" 3-LVL 11 3- 1 3/4" X 11-7/8" 2-LVL 14 2- 1 3/4" X 14" 3-I VI 14 3- 1 3/4" X 14"	, ,	ROPPED
U.N.O. FASTEN MULTIPLE PLY LVL'S	FG. FI	XED GLASS
(2) ROWS @ 12" O.C. $3\frac{1}{2}$ " (16d) NAILS OR SDW SCREWS	FDTN.	FOUNDATION
STEEL LINTELS	BG. E	BLACK GLASS
S1 <4'-0" L3-1/2" X 3-1/2" - 1/4"	JT J	ACK TRUSS
S2 < 6'-0" L4" X 3-1/2" X 1/4"	U/S L	JNDERSIDE
S3 < 8'-0" L5" X 3-1/2" X 5/16" S4 < 10'-0" L6" X 3-1/2" X 5/16"	T/O T	OP OF
-BEAR LINTELS MIN. 6" EACH END -LINTELS SHALL BE PRIMED TO	FG. F	FIXED GLASS
"CISC/CPMA 2-3" -ORIENT ANGLES LONG LEG VERT	CLG. C	CEILING

-ORIENT ANGLES LONG LEG VERT. CLG. CEILING -MAX. 1" BRICK OVERHANG BBFM BEAM BY FLOOR -WEEP HOLES @ 56" O.C. MAX

G GAS LINE SOLID BEARING (SAME WIDTH AS SUPPORTED MEMBER) CABLE LINE PHONE JACK CONCENTRATED POINT LOAD CEILING EXHAUST FAN FIRE PLACE VENT

MANUFACTURE

CEILING LIGHT

STOVE VENT CELLAR VENT

SMOKE ALARM 3 WAY SWITCH CARBON MONOXIDE LIGHT SWITCH DETECTOR 120 VOLT RECEPTACLE

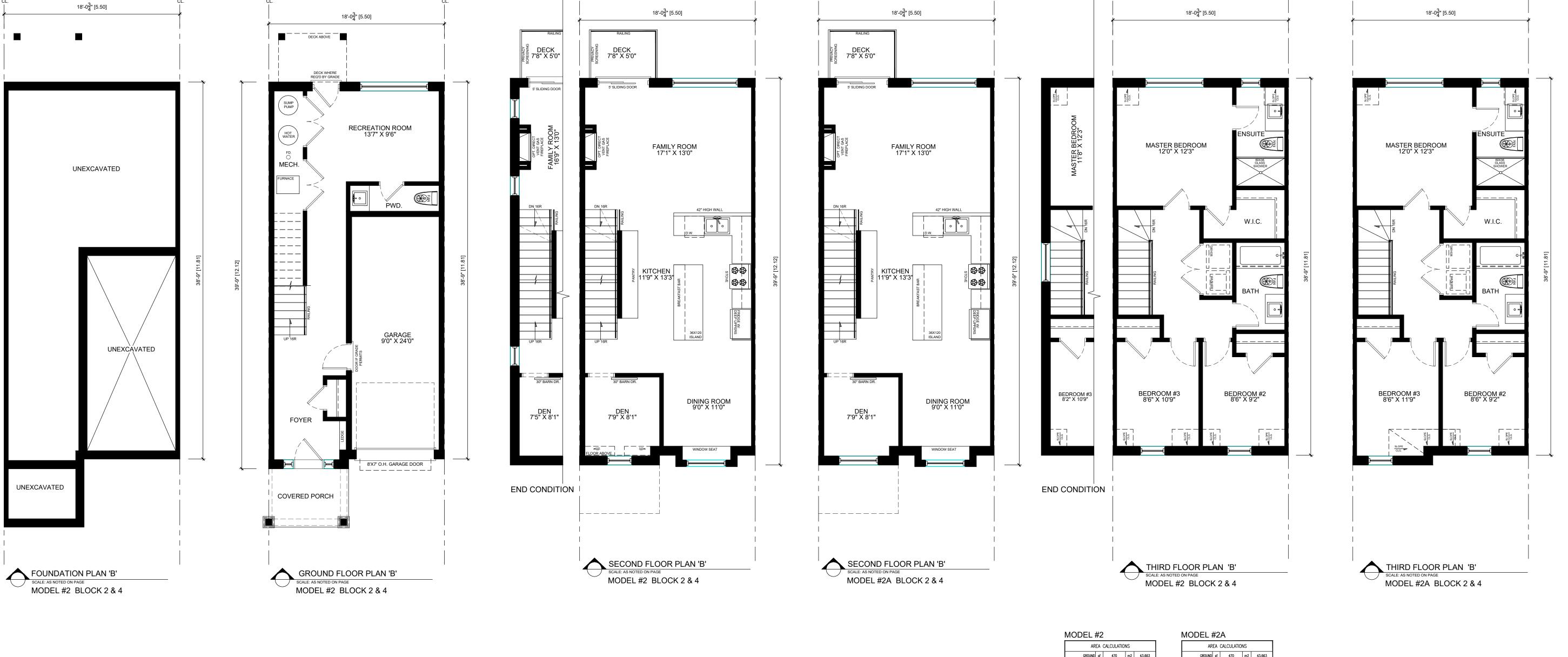
HOSE BIB WP. WATERPROOF
ELECTRICAL OUTLET

ELECTRIC RECEPTACLE

(42" OFF FINISH FLOOR) HOOD/CELLAR VENT FLOOR DRAIN DRYER VENT

DATE: DWN. CHK NO: REVISION: Issued for review NOV14/22 dcb dc NOV30/23 dcb Issued for review

334&340 ARDAGH ROAD
BARRIE, ON



ARCA CALCULATIONS

GROUND sf 470 m2 43.663

SECOND sf 715 m2 66.424

THIRD sf 708 m2 65.773

- 0.T.B. sf 0 m2 0.000

TOTAL sf 1893 m2 175.860

FIN BASEMENT sf 0 m2 0.000

TOTAL sf 1893 m2 175.860 GROUND of 470 m2 43.663

SECOND of 715 m2 66.424

THIRD of 699 m2 64.937

- 0.T.B. of 0 m2 0.000

TOTAL of 1884 m2 175.024

FIN BASEMENT of 0 m2 0.000 TOTAL sf 1884 m2 175.024 COVERAGE CALCULATIONS COVERAGE CALCULATIONS GROUND sf 470 m2 43.663 GROUND sf 470 m2 43.663 GRACHE SF 237 m2 22.017

FRONT PORCH SF 49 m2 4.552

REAR PORCH SF 0 m2 0.000

COV. W/O PORCH SF 707 m2 65.680

COV. W/ PORCH SF 756 m2 70.232 GARAGE sf 237 m2 22.017 FRONT PORCH sf 49 m2 4.552 REAR PORCH sf 0 m2 0.000 COV. W/O PORCH sf 707 m2 65.680 COV. W/ PORCH sf 756 m2 70.232

> Preliminary PG. CONTENT: FLOOR PLANS (Not for Construction)

\*CONSTRUCTION NOTES ARE THE PROPERTY OF VULCAN DESIGN INC. AND MAY NOT BE DUPLICATED OR COPIED WITH OUT THE WRITTEN CONSENT OF VULCAN DESIGN INC. VULCAN DESIGN INC. \*ANY SUBSTITUTE MATERIALS TO NOTES STATED SHALL CONFORM TO O.B.C. REGULATIONS AND MANUFACTURES SPECIFICATIONS SHALL BE SUPPLIED BY BUILDER Professional Service, Exceptional Value \*\*ONLY HEX NOTES IDENTIFIED ON PLANS / ELEVATIONS SHALL APPLY \*NOTES / CONSTRUCTION TO CONFORM TO 2012 ONTARIO BUILDING CODE 35 MCDONALD STREET ALLISTON, ON L9R 1J5 PH: 416-885-5200 EMAIL: dberry@vulcandesigninc.com CONTACT PERSON: DANIEL BERRY ROOF PEAK \_ **BCIN DECLARATION:** ROOF MID-POINT \_\_\_ I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY TOP PLATE \_ \_\_\_\_ FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE TOP OF WINDOW FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES. FIRM BCIN: 36501 DESIGNER BCIN: 21107 SIGNATURE: \_ DATE: \_\_\_\_\_ FINISH THIRD\_ U/S OF FIN. CEILING TOP OF WINDOW FINISH SECOND U/S OF FIN. CEILING \_\_\_\_\_ TOP OF WINDOW, LEGEND / LINTELS: FINISH MAIN LINTELS: **ABBREVIATIONS:** WOOD FINISH GRADE 8'X8' O.H. GARAGE DOOR 8'X8' O.H. GARAGE DOOR 8'X8' O.H. GARAGE DOOR DJ DOUBLE JOIST W1 2/2"X8" W2 2/2"X10" TJ TRIPLE JOIST  $r^{-}$ W3 2/2"X12" U/S OF FOOTING (4'0" MIN.) GT GIRDER TRUSS W4 3/2"X8" 'DO' DO OVER W5 3/2"X10" W6 3/2"X12" BLOCK 1 TOWNHOUSE RJ ROOF JOISTS PROPOSED EAST ELEVATION
SCALE: AS NOTED ON PAGE STL. STEEL LVL (2.0E) SB. SOLID BEARING (SEE 'FRAMING REQ.'S') 2-LVL 7 2- 1 3/4" X 7-1/4" 3-LVL 7 3- 1 3/4" X 7-1/4" 2-LVL 9 2- 1 3/4" X 9-1/2" 3-LVL 9 3- 1 3/4" X 9-1/2" 2-LVL 11 2- 1 3/4" X 11-7/8" 3-LVL 11 3- 1 3/4" X 11-7/8" MODEL #1 PL. POINT LOAD (FL.) FLUSH 2-LVL 14 2- 1 3/4" X 14" 3-LVL 14 3- 1 3/4" X 14" U.N.O. FASTEN MULTIPLE PLY LVL'S FG. FIXED GLASS
(2) ROWS @ 12" O.C. 3 ½" (16d) NAILS
OR SDW SCREWS FDTN. FOUNDATION FDTN. FOUNDATION BG. BLACK GLASS STEEL LINTELS JT JACK TRUSS S1 <4'-0" L3-1/2" X 3-1/2" - 1/4" S2 < 6'-0" L4" X 3-1/2" X 1/4" U/S UNDERSIDE S3 < 8'-0" L5" X 3-1/2" X 5/16" T/O TOP OF S4 < 10'-0" L6" X 3-1/2" X 5/16" -BEAR LINTELS MIN. 6" EACH END FG. FIXED GLASS -LINTELS SHALL BE PRIMED TO -ORIENT ANGLES LONG LEG VERT. CLG. CEILING "CISC/CPMA 2-3" -MAX. 1" BRICK OVERHANG BBFM BEAM BY FLOOR \_\_\_ \_ <u>T</u>O<u>P PLATE</u> \_ \_ <del>\_ \_ </del> -WEEP HOLES @ 56" O.C. MAX MANUFACTURE G GAS LINE TOP OF WINDOW SOLID BEARING (SAME WIDTH AS SUPPORTED MEMBER) CABLE LINE PHONE JACK POINT LOAD CEILING EXHAUST FAN FIRE PLACE VENT PULL CHAIN CLG. LIGHT STOVE VENT U/S OF FIN. CEILING \_\_\_ CELLAR VENT TOP OF WINDOW SMOKE ALARM 3 WAY SWITCH CARBON MONOXIDE LIGHT SWITCH DETECTOR HOSE BIB 120 VOLT RECEPTACLE WP. WATERPROOF ELECTRICAL OUTLET # ELECTRIC RECEPTACLE (42" OFF FINISH FLOOR) FINISH SECOND HOOD/CELLAR VENT FLOOR DRAIN U/S OF FIN. CEILING \_\_\_ DRYER VENT TOP OF WINDOW DATE: DWN. CH NO: REVISION: Issued for review MAY04/23 dcb dc NOV30/23 dcb Issued for review FINISH MAIN 334&340 ARDAGH ROAD
BARRIE, ON BLOCK 1 TOWNHOUSE Preliminary PG. CONTENT: ELEVATIONS PROPOSED WEST ELEVATION
SCALE: AS NOTED ON PAGE MODEL #1 (Not for Construction)



# EMAIL: dberry@vulcandesigninc.com CONTACT PERSON: DANIEL BERRY

# BCIN DECLARATION:

I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES. FIRM BCIN: 36501

DESIGNER BCIN: 21107

SIGNATURE:\_ \_ DATE: \_\_\_\_\_

# LEGEND / LINTELS:

DIST ST USS
ST
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RING EQ.'S')
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ASS
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LOOR TURE
ST FAN
G. LIGH
PTACLE
EPTACLI FLOOR
CHK
dcb
_

PROJECT: V22-0677 334&340 ARDAGH ROAD BARRIE, ON IPAGE:

Preliminary

PG. CONTENT:
ELEVATIONS

SCALE: 3/16" = 1'-0"





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(2) ROWS @ 12" O.C. 3 ½" (16d) NAILS
OR SDW SCREWS FDTN. FOUNDATION FDTN. FOUNDATION BG. BLACK GLASS STEEL LINTELS JT JACK TRUSS S1 <4'-0" L3-1/2" X 3-1/2" - 1/4" S2 < 6'-0" L4" X 3-1/2" X 1/4" U/S UNDERSIDE S3 < 8'-0" L5" X 3-1/2" X 5/16" T/O TOP OF 2:12 S4 < 10'-0" L6" X 3-1/2" X 5/16" -BEAR LINTELS MIN. 6" EACH END FG. FIXED GLASS -LINTELS SHALL BE PRIMED TO -ORIENT ANGLES LONG LEG VERT. CLG. CEILING "CISC/CPMA 2-3" -MAX. 1" BRICK OVERHANG BBFM BEAM BY FLOOR -WEEP HOLES @ 56" O.C. MAX \_\_TOP PLATE\_\_\_\_\_ MANUFACTURE G GAS LINE SOLID BEARING TOP OF WINDOW (SAME WIDTH AS SUPPORTED MEMBER) CABLE LINE PHONE JACK CONCENTRATED POINT LOAD CEILING EXHAUST FAN FIRE PLACE VENT \_FINISH THIRD\_ STOVE VENT CEILING LIGHT \_\_\_\_\_ <u>U/S OF FIN. CEI</u>LIN<u>G</u>\_\_\_\_ CELLAR VENT TOP OF WINDOW SMOKE ALARM 3 WAY SWITCH CARBON MONOXIDE LIGHT SWITCH DETECTOR HOSE BIB 120 VOLT RECEPTACLE WP. WATERPROOF ELECTRICAL OUTLET # ELECTRIC RECEPTACLE (42" OFF FINISH FLOOR) HOOD/CELLAR VENT FINISH SECOND FLOOR DRAIN DRYER VENT DATE: DWN. CHK NO: REVISION: Issued for review MAY04/23 dcb dc NOV30/23 dcb Issued for review FINISH MAIN

\_\_\_\_\_\_

**BLOCK 2 TOWNHOUSE** 

PROPOSED WEST ELEVATION 'B'
SCALE: AS NOTED ON PAGE
MODEL #2

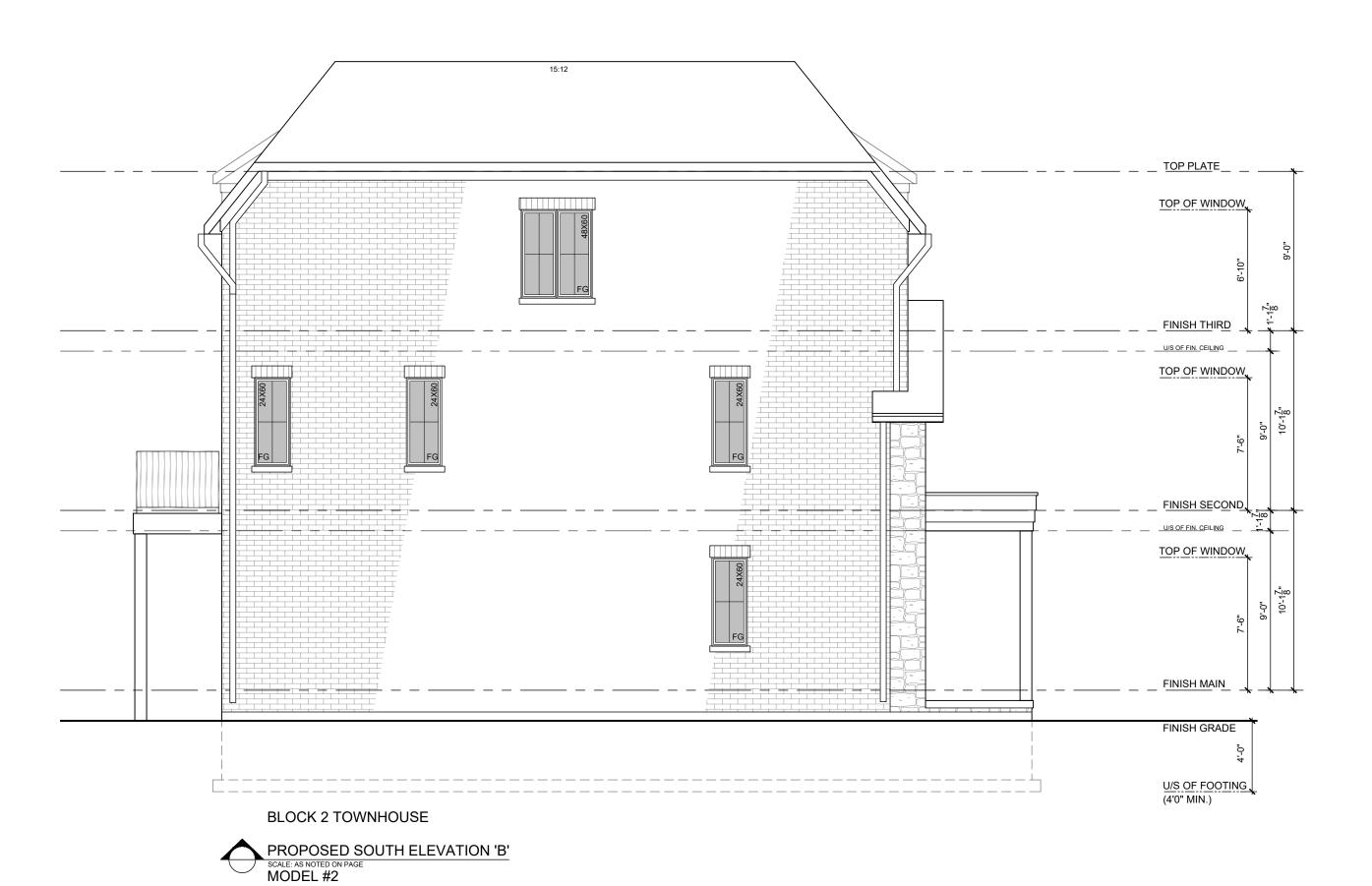
U/S OF FOOTING

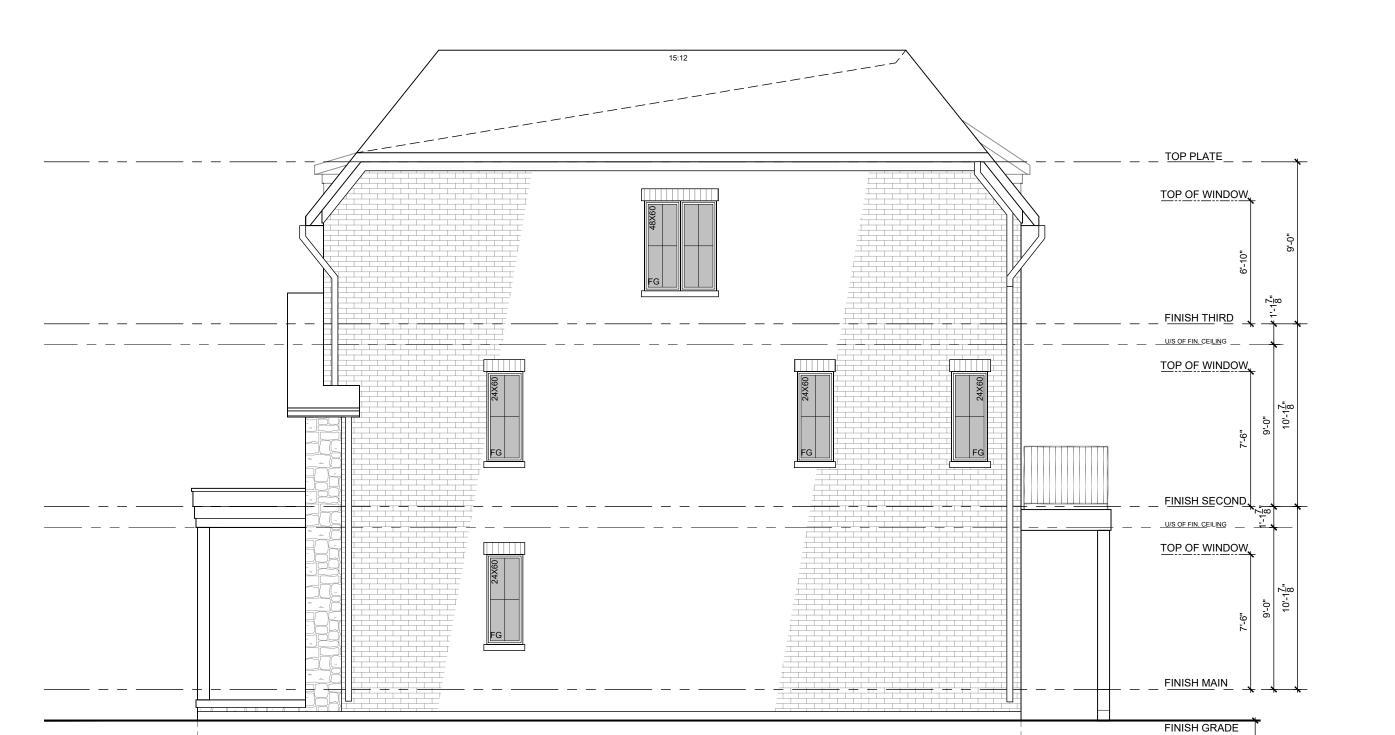
(4'0" MIN.)

(Not for Construction)

334&340 ARDAGH ROAD
BARRIE, ON
IPAGE:

PG. CONTENT:
ELEVATIONS





BLOCK 2 TOWNHOUSE

PROPOSED NORTH ELEVATION 'B'
SCALE: AS NOTED ON PAGE
MODEL #2

Preliminary
(Not for Construction)

BARRIE,

PG. CONTENT:
ELEVATIONS

SCALE: 3/16" = 1'-0"

U/S OF FOOTING (4'0" MIN.)

# VULCAN DESIGN INC. Professional Service, Exceptional Value

35 MCDONALD STREET

ALLISTON, ON L9R 1J5 PH: 416-885-5200

# BCIN DECLARATION:

EMAIL: dberry@vulcandesigninc.com
CONTACT PERSON: DANIEL BERRY

I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES. FIRM BCIN: 36501

DESIGNER BCIN: 21107

SIGNATURE:\_ \_ DATE: \_\_\_\_\_

# LEGEND / LINTELS:

<u>LINTELS:</u>	ABBREVIATIONS:
WOOD	
W1 2/2"X8"	DJ DOUBLE JOIST
W2 2/2"X10"	TJ TRIPLE JOIST
W3 2/2"X12"	GT GIRDER TRUSS
W4 3/2"X8" W5 3/2"X10"	'DO' DO OVER
W6 3/2"X12"	RJ ROOF JOISTS
	STL. STEEL
LVL (2.0E)	SB. SOLID BEARING
2-LVL 7 2- 1 3/4" X 7-1/4" 3-LVL 7 3- 1 3/4" X 7-1/4"	(SEE 'FRAMING REQ.'S'
2-LVL 9 2- 1 3/4" X 9-1/2" 3-LVL 9 3- 1 3/4" X 9-1/2"	PL. POINT LOAD
2-LVL 11 2- 1 3/4" X 11-7/8" 3-LVL 11 3- 1 3/4" X 11-7/8"	(FL.) FLUSH
2-LVL 14 2- 1 3/4" X 14"	(DR.) DROPPED
U.N.O. FASTEN MULTIPLE PLY LVL'S	FG. FIXED GLASS
(2) ROWS @ 12" O.C. 3 ½" (16d) NAILS OR SDW SCREWS	FDTN. FOUNDATION
STEEL LINTELS	BG. BLACK GLASS
S1 <4'-0" L3-1/2" X 3-1/2" - 1/4"	JT JACK TRUSS
S2 < 6'-0" L4" X 3-1/2" X 1/4"	U/S UNDERSIDE
S3 < 8'-0" L5" X 3-1/2" X 5/16"	T/O TOP OF
S4 < 10'-0" L6" X 3-1/2" X 5/16" -BEAR LINTELS MIN. 6" EACH END	
-LINTELS SHALL BE PRIMED TO "CISC/CPMA 2-3"	FG. FIXED GLASS
-ORIENT ANGLES LONG LEG VERTMAX. 1" BRICK OVERHANG	CLG. CEILING
-WEEP HOLES @ 56" O.C. MAX	BBFM BEAM BY FLOOF MANUFACTURE
SB. SOLID BEARING	GAS LINE
SUPPORTED MEMBER)	CABLE LINE
	PHONE JACK
POINT LOAD FAI	N CEILING EXHAUST FA
P.0	) ).
DRYER VENT	PULL CHAIN CLG. LIG
STOVE VENT	CEILING LIGHT
© CELLAR VENT	OLILING LIGHT
SMOKE ALARM	3 WAY SWITCH
CARBON MONOXIDE \$	LIGHT SWITCH
↑ HOSE BIB	
WP. WATERPROOF 42"	
ELECTRICAL OUTLET	
HOOD/CELLAR VENT FD	).
DRYER VENT	FLOOR DRAIN
NO: REVISION :	DATE: DWN.CHK
1 Issued for review	MAY04/23 dcb dcb
2 Issued for review	NOV30/23 dcb
	10.00,20
	<del>                                     </del>

PROJECT: V22-0677 334&340 ARDAGH ROAD BARRIE, ON IPAGE:

8'X8' O.H. GARAGE DOOR

\_\_\_\_\_

8'X8' O.H. GARAGE DOOR

8'X8' O.H. GARAGE DOOR

8'X8' O.H. GARAGE DOOR

BLOCK 3 TOWNHOUSE

PROPOSED WEST ELEVATION
SCALE: AS NOTED ON PAGE

**BLOCK 3 TOWNHOUSE** 

PROPOSED EAST ELEVATION
SCALE: AS NOTED ON PAGE
MODEL #1

MODEL #1

### VULCAN DESIGN INC. Professional Service, Exceptional Value

35 MCDONALD STREET

ALLISTON, ON L9R 1J5 PH: 416-885-5200

EMAIL: dberry@vulcandesigninc.com
CONTACT PERSON: DANIEL BERRY

ROOF PEAK \_ \_\_\_

ROOF MID-POINT

\_\_ <u>TOP PLATE</u> \_ \_ <del>\_\_\_</del>

FINISH THIRD

FINISH SECOND

TOP PLATE\_

FINISH THIRD 1

TOP OF WINDOW

FINISH SECOND

\_\_\_\_\_ FINISH MAIN

FINISH GRADE

U/S OF FOOTING (4'0" MIN.)

TOP OF WINDOW

FINISH GRADE

\_\_ U/S OF FIN. CEILING\_\_\_ \_

TOP OF WINDOW

\_\_ U/S OF FIN. CEILING \_\_\_ TOP OF WINDOW,

TOP OF WINDOW

## **BCIN DECLARATION:**

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FIRM BCIN: 36501 DESIGNER BCIN: 21107

SIGNATURE: \_ DATE: \_\_\_\_\_

## LEGEND / LINTELS:

ABBREVIATIONS:

LINTELS.	ADDE	\⊏ V I	ATIO	<u> </u>	
WOOD					
W1 2/2"X8"	DJ	DOU	BLE J	DIST	
W2 2/2"X10" W3 2/2"X12"	TJ	TRIP	LE JO	IST	
W4 3/2"X8"	GT	GIRE	ER TE	RUSS	
W5 3/2"X10"	'DO'	DO C	VER		
W6 3/2"X12"	RJ	ROO	F JOIS	STS	
LVL (2.0E)	STL.	STEE	ΞL		
2-LVL 7 2- 1 3/4" X 7-1/4" 3-LVL 7 3- 1 3/4" X 7-1/4" 2-LVL 9 2- 1 3/4" X 9-1/2" 3-LVL 9 3- 1 3/4" X 9-1/2" 2-LVL 11 2- 1 3/4" X 11-7/8"	SB. S (SEE 'I PL. I (FL.) I	FRAM POIN	IING F	REQ.'S	;')
3-LVL 11 3- 1 3/4" X 11-7/8" 2-LVL 14 2- 1 3/4" X 14"	(DR.) I				
3-LVL 14 3- 1 3/4" X 14"  U.N.O. FASTEN MULTIPLE PLY LVL'S	FG. I	FIXED	GLAS	ss	
(2) ROWS @ 12" O.C. 3 ½" (16d) NAILS OR SDW SCREWS	FDTN.	FO	UNDA	TION	
STEEL LINTELS	BG.	BLA	CK GL	ASS	
S1 <4'-0" L3-1/2" X 3-1/2" - 1/4"	JT	JAC	K TRU	SS	
S2 < 6'-0" L4" X 3-1/2" X 1/4" S3 < 8'-0" L5" X 3-1/2" X 5/16"	U/S	UND	ERSIE	ÞΕ	
S4 < 10'-0" L6" X 3-1/2" X 5/16"	T/O	TOP	OF		
-BEAR LINTELS MIN. 6" EACH END -LINTELS SHALL BE PRIMED TO	FG.	FIXE	D GLA	ASS	
"CISC/CPMA 2-3" -ORIENT ANGLES LONG LEG VERT.	CLG.	CEIL	ING		
-MAX. 1" BRICK OVERHANG -WEEP HOLES @ 56" O.C. MAX	BBFM		M BY I		
WW SOLID BEAKING	GAS CAB	LINE			
	рно	NE J	ACK		
POINT LOAD FA	_	ING E	EXHAL	JST F	Α
DRYER VENT		L CHA	AIN CL	G. LIC	Эŀ
STOVE VENT	⊢ CEII	ING I	₋IGHT		
© CELLAR VENT		10 1			
SMOKE ALARM		AY SV	VITCH		
CARBON MONOXIDE DETECTOR	LIGH	IT SW	/ITCH		
L HOSE BIB	120 \	VOLT	RECE	PTAC	)L
WP. WATERPROOF 42 ELECTRICAL OUTLET	( ELE		C REC		
HOOD/CELLAR VENT FI	D. (42	OR D		20	
DRYER VENT	FLO		•		
NO.: REVISION :		TE:	DWN.	CHK	
1 Issued for review		04/23		dcb	
2 Issued for review	NOV	30/23	dcb		
				I	1

334&340 ARDAGH ROAD
BARRIE, ON
IPAGE: PG. CONTENT:
ELEVATIONS

(Not for Construction)

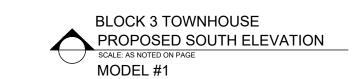


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BLOCK 3 TOWNHOUSE PROPOSED NORTH ELEVATION

MODEL #1





Preliminary

PG. CONTENT:
ELEVATIONS

SCALE: 3/16" = 1'-0"

U/S OF FOOTING (4'0" MIN.)

### VULCAN DESIGN INC. Professional Service, Exceptional Value

35 MCDONALD STREET

ALLISTON, ON L9R 1J5 PH: 416-885-5200

EMAIL: dberry@vulcandesigninc.com
CONTACT PERSON: DANIEL BERRY

## BCIN DECLARATION:

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DESIGNER BCIN: 21107

SIGNATURE:\_ \_ DATE: \_\_\_\_\_

# LEGEND / LINTELS:

	LLOLIND	, L	_			
LIN	TELS:		ABBR	REVI	ATIO	NS:
WO	<u>DC</u>					
	2/2"X8"		DJ	DOU	BLE JO	DIST
W2	2/2"X10" 2/2"X12"		TJ	TRIP	LE JO	IST
W4			GT	GIRE	ER TE	RUSS
W5			'DO'	DO C	VER	
W6	3/2"X12"		RJ	ROO	F JOIS	STS
LVL	(2.0E)		STL.	STE	ΞL	
3-LVL 2-LVL 3-LVL 2-LVL	. 9 3- 1 3/4" X 9-1/2" . 11 2- 1 3/4" X 11-7/8"		SB. S (SEE 'I PL. F (FL.) F	FRAN POIN	IING F	REQ.'S
3-LVL 2-LVL	. 11 3- 1 3/4" X 11-7/8" . 14 2- 1 3/4" X 14"		(DR.) [	DROF	PPED	
3-LVL U.N.C	. 14       3- 1 3/4" X 14" ). FASTEN MULTIPLE PLY LVI	L'S	FG. F	FIXED	GLAS	SS
	DWS @ 12" O.C. 3 ½" (16d) NA DW SCREWS	ILS	FDTN.	FO	UNDA	TION
STE	EL LINTELS		BG.	BLA	CK GL	ASS
	<4'-0" L3-1/2" X 3-1/2" - 1/4"		JT	JAC	K TRU	SS
S2 ·	< 6'-0" L4" X 3-1/2" X 1/4"		U/S	UND	ERSIE	DΕ
	< 8'-0" L5" X 3-1/2" X 5/16" < 10'-0" L6" X 3-1/2" X 5/16"		T/O	TOP	OF	
	R LINTELS MIN. 6" EACH END ELS SHALL BE PRIMED TO	)	FG.	FIXE	D GLA	ASS
	/CPMA 2-3" ENT ANGLES LONG LEG VER	Т.	CLG.	CEIL	ING	
	. 1" BRICK OVERHANG P HOLES @ 56" O.C. MAX		BBFM		M BY I	
SB.	SOLID BEARING	<b>©</b>	GAS	LINE		
PL.	(SAME WIDTH AS SUPPORTED MEMBER)	O	CAB	LE LI	NE	
X	CONCENTRATED POINT LOAD	FAN		NE J	ACK	
	FIRE PLACE VENT		CEIL	ING I	EXHAU	JST F
	DRYER VENT STOVE VENT	P.C.		_ CHA	AIN CL	G. LIG
	CELLAR VENT	-	- CEIL	ING I	₋IGHT	
SA	SMOKE ALARM	3 \$	3 W.A	AY SV	VITCH	
CMD	CARBON MONOXIDE DETECTOR	\$	LIGH	IT SW	VITCH	
	HOSE BIB		120 \	/OLT	RECE	PTAC
₩P.	WATERPROOF ELECTRICAL OUTLET	42"			C REC FINISH	
	HOOD/CELLAR VENT	FD.	`	OR D		
_	DRYER VENT		OWAGOS			
N	O.: REVISION :		DA	TE:	DWN.	CHK
	·		MAY			dcb
	2 Issued for review		NOV3	30/23	dcb	
	i i		1		1	

PROJECT: V22-0677 334&340 ARDAGH ROAD BARRIE, ON

\*CONSTRUCTION NOTES ARE THE PROPERTY OF VULCAN DESIGN INC. AND MAY NOT BE DUPLICATED OR COPIED WITH OUT THE WRITTEN CONSENT OF VULCAN DESIGN INC. VULCAN DESIGN INC. \*ANY SUBSTITUTE MATERIALS TO NOTES STATED SHALL CONFORM TO O.B.C. REGULATIONS AND MANUFACTURES SPECIFICATIONS SHALL BE SUPPLIED BY BUILDER Professional Service, Exceptional Value \*\*ONLY HEX NOTES IDENTIFIED ON PLANS / ELEVATIONS SHALL APPLY \*NOTES / CONSTRUCTION TO CONFORM TO 2012 ONTARIO BUILDING CODE 35 MCDONALD STREET ALLISTON, ON L9R 1J5 PH: 416-885-5200 EMAIL: dberry@vulcandesigninc.com CONTACT PERSON: DANIEL BERRY ROOF PEAK \_ \_ **BCIN DECLARATION:** ROOF MID-POINT I DANIEL BERRY DECLARE THAT I HAVE REVIEWED & TAKE DESIGN RESPONSIBILITY TOP\_PLATE\_ FOR THE DESIGN WORK UNDER DIVISION C, PART 3 SECTION 3.2 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE TOP OF WINDOW FIRM IS REGISTERED IN THE APPROPRIATE CATEGORIES. FIRM BCIN: 36501 DESIGNER BCIN: 21107 SIGNATURE: \_ DATE: \_\_\_\_\_ TOP OF WINDOW FINISH SECOND U/S OF FIN. CEILING TOP OF WINDOW LEGEND / LINTELS: LINTELS: **ABBREVIATIONS:** WOOD FINISH GRADE 8'X8' O.H. GARAGE DOOR DJ DOUBLE JOIST W1 2/2"X8" W2 2/2"X10" TJ TRIPLE JOIST W3 2/2"X12" U/S OF FOOTING GT GIRDER TRUSS W4 3/2"X8" (4'0" MIN.) 'DO' DO OVER **BLOCK 4 TOWNHOUSE** W5 3/2"X10" W6 3/2"X12" RJ ROOF JOISTS PROPOSED WEST ELEVATION 'B' scale: AS NOTED ON PAGE MODEL #2 STL. STEEL LVL (2.0E) SB. SOLID BEARING (SEE 'FRAMING REQ.'S') 2-LVL 7 2- 1 3/4" X 7-1/4" 3-LVL 7 3- 1 3/4" X 7-1/4" 2-LVL 9 2- 1 3/4" X 9-1/2" 3-LVL 9 3- 1 3/4" X 9-1/2" 2-LVL 11 2- 1 3/4" X 11-7/8" 3-LVL 11 3- 1 3/4" X 14" 2-LVL 14 2- 1 3/4" X 14" PL. POINT LOAD (FL.) FLUSH (DR.) DROPPED 3-LVL 14 3- 1 3/4" X 14" U.N.O. FASTEN MULTIPLE PLY LVL'S FG. FIXED GLASS (2) ROWS @ 12" O.C.  $3\frac{1}{2}$ " (16d) NAILS OR SDW SCREWS FDTN. FOUNDATION FDTN. FOUNDATION BG. BLACK GLASS STEEL LINTELS JT JACK TRUSS S1 <4'-0" L3-1/2" X 3-1/2" - 1/4" S2 < 6'-0" L4" X 3-1/2" X 1/4" U/S UNDERSIDE S3 < 8'-0" L5" X 3-1/2" X 5/16" T/O TOP OF 2:12 S4 < 10'-0" L6" X 3-1/2" X 5/16" -BEAR LINTELS MIN. 6" EACH END FG. FIXED GLASS -LINTELS SHALL BE PRIMED TO "CISC/CPMA 2-3" -ORIENT ANGLES LONG LEG VERT. CLG. CEILING
-MAX. 1" BRICK OVERHANG BBFM BEAM BY FLOOR -WEEP HOLES @ 56" O.C. MAX TOP PLATE \_ \_\_\_\_ MANUFACTURE G GAS LINE SOLID BEARING TOP OF WINDOW (SAME WIDTH AS SUPPORTED MEMBER) CABLE LINE PHONE JACK POINT LOAD CEILING EXHAUST FAN FIRE PLACE VENT - PULL CHAIN CLG. LIGHT FINISH THIRD STOVE VENT CELLAR VENT TOP OF WINDOW SMOKE ALARM 3 WAY SWITCH CARBON MONOXIDE LIGHT SWITCH DETECTOR HOSE BIB 120 VOLT RECEPTACLE WP. WATERPROOF ELECTRICAL OUTLET # ELECTRIC RECEPTACLE (42" OFF FINISH FLOOR) HOOD/CELLAR VENT FINISH SECOND FLOOR DRAIN DRYER VENT TOP OF WINDOW DATE: DWN. CH NO: REVISION: Issued for review MAY04/23 dcb dc NOV30/23 dcb \_\_\_\_\_ FINISH MAIN \_ FINISH GRADE 334&340 ARDAGH ROAD BARRIE, ON BLOCK 4 TOWNHOUSE \_\_\_\_\_ PG. CONTENT:
ELEVATIONS PROPOSED EAST ELEVATION 'B'
SCALE: AS NOTED ON PAGE
MODEL #2 (Not for Construction)

TYPICAL BUILDING SECTION
SCALE: AS NOTED ON PAGE

FINISH THIRD

FINISH SECOND

FINISH MAIN



35 MCDONALD STREET ALLISTON, ON L9R 1J5 PH: 416-885-5200

EMAIL: dberry@vulcandesigninc.com
CONTACT PERSON: DANIEL BERRY

## **BCIN DECLARATION:**

I DANIEL BERRY DECLARE THAT I HAVE
REVIEWED & TAKE DESIGN RESPONSIBILITY
FOR THE DESIGN WORK UNDER DIVISION C,
PART 3 SECTION 3.2 OF THE ONTARIO
BUILDING CODE. I AM QUALIFIED AND THE
FIRM IS REGISTERED IN THE APPROPRIATE
CATEGORIES.

FIRM BCIN: 36501

DESIGNER BCIN: 21107

SIGNATURE: DATE: \_\_\_\_\_

LEGEND / LINTELS:

	2"X8" DJ 2"X10" TJ 2"X12" GT 2"X12" RJ 2"X12" RJ 2"X12" RJ 2"X12" RJ 3-1 3/4" X 7-1/4" SB. 3-1 3/4" X 9-1/2" PL. 3-1 3/4" X 9-1/2" PL. 3-1 3/4" X 11-7/8" (FL 3-1 3/4" X 14" (DF 3-1 3/4" X 11-7/8" (DF 3-1 3/4" X 11-7/8			
LINTEL	<u>S:</u>		ABBF	REVIATIONS:
WOOD				
W1 2/2	"X8"		DJ	DOUBLE JOIST
W2 2/2			TJ	TRIPLE JOIST
_			GT	GIRDER TRUSS
W4 3/2 W5 3/2			'DO'	DO OVER
W6 3/2			RJ	ROOF JOISTS
LVL (2.0E	·)		STL.	STEEL
2-LVL 7 3-LVL 7 2-LVL 9	2- 1 3/4" X 7-1/4" 3- 1 3/4" X 7-1/4" 2- 1 3/4" X 9-1/2"		(SEE '	SOLID BEARING FRAMING REQ.'S' POINT LOAD
			(FL.)	FLUSH
2-LVL 14	2- 1 3/4" X 14"		(DR.)	DROPPED
	ΓΕΝ MULTIPLE PLY LVI		FG.	FIXED GLASS
(2) ROWS @ OR SDW SC		LS	FDTN.	FOUNDATION
STEEL LI	NTELS		BG.	BLACK GLASS
S1 <4'-0"	L3-1/2" X 3-1/2" - 1/4"		JT	JACK TRUSS
			U/S	UNDERSIDE
			T/O	TOP OF
		1	FG.	FIXED GLASS
"CISC/CPMA -ORIENT AN		Т.	CLG.	CEILING
			BBFM	BEAM BY FLOOF
SB. SOL	ID READING	G	GAS	SLINE
(0/1	ME WIDTH AS	0	CAB	BLE LINE
PL.		•	PHC	NE JACK
		~		LING EXHAUST FA
DRY	YER VENT			L CHAIN CLG. LIG
STO	OVE VENT		OFII	INCLICUT
	LAR VENT	3	- CEII	LING LIGHT
SA SMO	OKE ALARM	\$	3 W	AY SWITCH
CAF		\$	LIGH	HT SWITCH
DET	20.0.0			
⊥ HOS			120	VOLT RECEPTAC
HOS	SE BIB TERPROOF	42"		VOLT RECEPTAC
HOS WA	SE BIB TERPROOF CTRICAL OUTLET	42"	ELE	

NO: REVISION:

Issued for review

Issued for review

DATE: DWN.CHK

MAY04/23 dcb dcb NOV30/23 dcb

Preliminary

Project: V22-0677
334&340 ARDAGH ROAD
BARRIE, ON

PG. CONTENT:
ELEVATIONS
SCALE: 3/16" = 1'-0"

A 11



PROPOSED NORTH ELEVATION 'B'
SCALE: AS NOTED ON PAGE
MODEL #2

TOP PLATE\_\_\_\_

FINISH THIRD

FINISH SECOND

U/S OF FIN. CEILING

FINISH MAIN

FINISH GRADE

TOP OF WINDOW

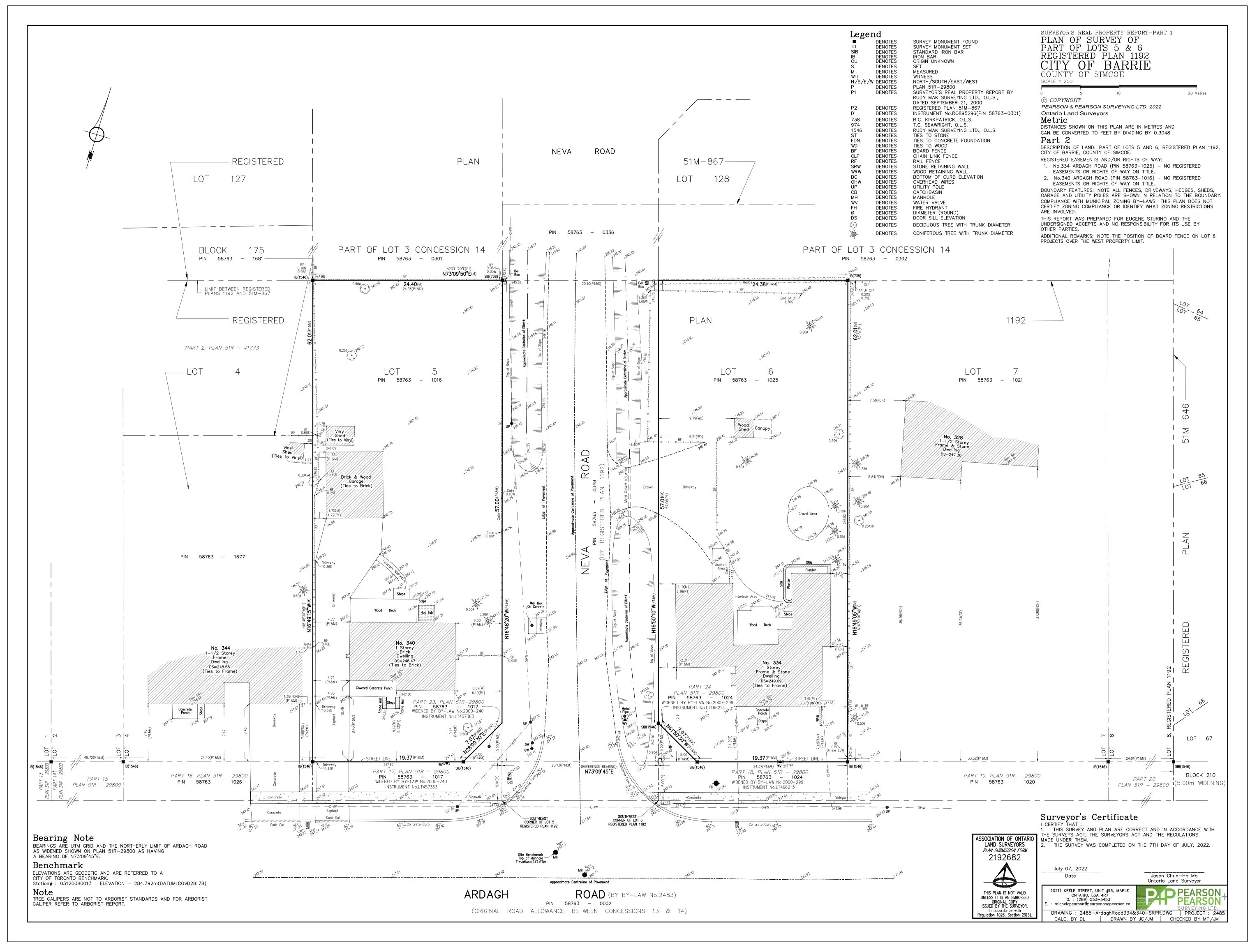
TOP OF WINDOW

\_\_\_\_\_U/S OF FIN. CEILING\_\_\_

TOP OF WINDOW



Prelimina (Not for Construe



# APPENDIX B



### **Drawing** Notes on Sample Descriptions

1. All sample descriptions included in this report follow the Canadian Foundations Engineering Manual soil classification system. This system follows the standard proposed by the International Society for Soil Mechanics and Foundation Engineering. Laboratory grain size analyses provided by Sirati & Partners Consultants Limited also follow the same system. Different classification systems may be used by others; one such system is the Unified Soil Classification. Please note that, with the exception of those samples where a grain size analysis has been made, all samples are classified visually. Visual classification is not sufficiently accurate to provide exact grain sizing or precise differentiation between size classification systems.

### ISSMFE SOIL CLASSIFICATION GRAVE MEDIUM 0.006 0.02 0.002 0.06 0.6 2.0 6.0 200 EQUIVALENT GRAIN DIAMETER IN MILLIMETRES CLAY (PLASTIC) TO FINE MEDIUM FINE COARSE

UNIFIED SOIL CLASSIFICATION

SILT (NONPLASTIC)

- 2. Fill: Where fill is designated on the borehole log it is defined as indicated by the sample recovered during the boring process. The reader is cautioned that fills are heterogeneous in nature and variable in density or degree of compaction. The borehole description may therefore not be applicable as a general description of site fill materials. All fills should be expected to contain obstruction such as wood, large concrete pieces or subsurface basements, floors, tanks, etc., none of these may have been encountered in the boreholes. Since boreholes cannot accurately define the contents of the fill, test pits are recommended to provide supplementary information. Despite the use of test pits, the heterogeneous nature of fill will leave some ambiguity as to the exact composition of the fill. Most fills contain pockets, seams, or layers of organically contaminated soil. This organic material can result in the generation of methane gas and/or significant ongoing and future settlements. Fill at this site may have been monitored for the presence of methane gas and, if so, the results are given on the borehole logs. The monitoring process does not indicate the volume of gas that can be potentially generated nor does it pinpoint the source of the gas. These readings are to advise of the presence of gas only, and a detailed study is recommended for sites where any explosive gas/methane is detected. Some fill material may be contaminated by toxic/hazardous waste that renders it unacceptable for deposition in any but designated land fill sites; unless specifically stated the fill on this site has not been tested for contaminants that may be considered toxic or hazardous. This testing and a potential hazard study can be undertaken if requested. In most residential/commercial areas undergoing reconstruction, buried oil tanks are common and are generally not detected in a conventional geotechnical site investigation.
- 3. Till: The term till on the borehole logs indicates that the material originates from a geological process associated with glaciation. Because of this geological process the till must be considered heterogeneous in composition and as such may contain pockets and/or seams of material such as sand, gravel, silt or clay. Till often contains cobbles (60 to 200 mm) or boulders (over 200 mm). Contractors may therefore encounter cobbles and boulders during excavation, even if they are not indicated by the borings. It should be appreciated that normal sampling equipment cannot differentiate the size or type of any obstruction. Because of the horizontal and vertical variability of till, the sample description may be applicable to a very limited zone; caution is therefore essential when dealing with sensitive excavations or dewatering programs in till materials.



### **LOG OF BOREHOLE BH/MW-01**

PROJECT: Geotechnical and Hydrogeologival Investigations

CLIENT: Evans Planning

PROJECT LOCATION: 334 Ardagh Road, Barrie, ON

DATUM: Geodetic

DRILLING DATA

Method: Solid Stem Auger

Diameter: 150 mm REF. NO.: SP22-01117-00 A

Date: Jan-20-2023 ENCL NO.: 2

	SOIL PROFILE		S	SAMPL	.ES			DYN/ RESI	AMIC CO STANCE	NE PEN PLOT	NETRA	TION		DI ACT	. NATI	JRAL	HOUR		F	RE	MARK	۲S
(m) ELEV DEPTH	DESCRIPTION	STRATA PLOT	NUMBER	ТҮРЕ	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION	SHE	20 4 AR ST JNCONF QUICK T	10 6 RENG	0 8 TH (kF + ×	0 10	.NE ity NE	W <sub>P</sub> WA	C NATU MOIS CON V TER CC	NTENT	LIQUID LIMIT W <sub>L</sub> T (%)	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m³)	GR	AND AIN SI RIBUT (%)	IZ TIC
2 <u>48:9</u> 0.3	TOPSOIL: 305 mm FILL: sand, trace to some silt, trace	<u>11/2</u>	1	SS	4		247								0							_
	organics, trace rootlets, brown, moist, loose trace gravel, very loose		2	SS	2		246							0								
45.3			3	SS	15		240							0								
1.8	<b>SAND:</b> trace silt, trace gravel, brown, moist, compact				10		245											-				
	dense, trace cobbles		4	SS	20		· 244							0								
	delise, trace consider		5	SS	43									0								
42.6							243	_					•									
4.6	SILTY SAND WITH GRAVEL: brown, wet, compact	. 4	6	SS	20		W. L.				1	<b>(</b>			0					23 3	8 38	3
		0 0					· · · · · · · · · · · · · · · · · · ·	, 202 [ - - -	.5		كه											
6.1 40.5	SAND: trace to some silt, trace gravel, brown, wet, very dense	ر از م	7	ss	65		241			V	`— 				0			-				
	Borehole was open upon completion of drilling.     Groundwater level was encountered at 4.57 mbgs upon completion of drilling.     Monitoring well was installed in the borehole upon completion of drilling.     Groundwater level was measured at 4.9 mbgs on February 27, 2023.																					





### LOG OF BOREHOLE BH/MW-02A

PROJECT: Geotechnical and Hydrogeologival Investigations

CLIENT: Evans Planning

PROJECT LOCATION: 334 Ardagh Road, Barrie, ON

DATUM: Geodetic

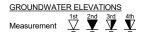
DRILLING DATA

Method: Solid Stem Auger

Diameter: 150 mm REF. NO.: SP22-01117-00 A

Date: Jan-20-2023 ENCL NO.: 3

	SOIL PROFILE		s	AMPL	.ES			DYNA RESIS	AMIC CO STANCE	NE PEN E PLOT	NETRA	TION		DI ASTI	C NATI	URAL	LIQUID		5	REN	MARKS
(m)		٦				] [] [			20 4	40 6	0 8	30 10	00	L	CON	TENT	LIMIT	a) EN	NTN.		AND
LEV	DESCRIPTION	STRATA PLOT	_		BLOWS 0.3 m	GROUND WATER				RENG	TH (k	Pa)	ANIE	W <sub>P</sub>	\ 	<i>w</i> o	W <sub>L</sub>	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (KN/m³)	GRA DISTR	IN SIZ RIBUTI
EPTH	DESCRIPTION	ATA	/BEF	ш	BLO 0.3	INC.	CONDITION		INCONF	INED RIAXIAL	+	FIELD VA & Sensiti	Vity	WA-	TER CC	ONTEN	Γ(%)	90°	PTUT PTUT		(%)
46.4		STR	NUMBER	TYPE	ż	GRC		1					00	1			30		_	GR SA	A SI
46.4	CLEAR STONE: 50 mm	X	1	SS	5	• •															
	<b>FILL:</b> sand, some silt, trace gravel, dark brown, moist, loose	$\bigotimes$		33	3		246	-						0				1			
	trace silt, trace rootlets, very loose	$\bowtie$				1		-													
		$\bowtie$	2	SS	1										0						
44.6		$\bowtie$	$\vdash$			H	245											1			
1.8	SAND: trace silt, trace gravel,	(X)	3	SS	6			E						٥							
	brown, moist, loose							Ē													
			4	SS	21		244							0							
						ı		-													
	trace cobbles, compact becoming poorly graded sand with		5	SS	35		0.40	Ē													
	gravel, trace cobbles, trace silt, dense		Ľ				243											1			
	2550							Ė													
							242	<u> </u>				L .	_					]			
	trace cobbles wet				-			ŧ				<b>/</b>	\								
	trace cobbles, wet		6	SS	30	J∷E		Ē			_	Ś	•		0						
						]: [	241		1		~ \	<b>`</b> _						1			
								F	 m		$\leftarrow$	-									
0.3 6.1	INTERBEDDED LAYERS OF	7.77				<b>∤:</b>  E	Feb 2	7, 202 Г	3	()	•										
0.1	LEAN CLAY AND SILTY SAND:		7	SS	18	ŀΕ	240	<u> </u>		~					-	0	-	-		0 9	50
	trace gravel, brown, moist to wet, compact/ very stiff		$\vdash$			╂┋		E													
						E		Ē													
							239	_	1									1			
			8	SS	18	]	· .	Ē													
			Ľ		10	器	8	ŧ													
						R	238	<u> </u>										1			
						88	<b>5</b>	E													
	trace cobbles, very dense/ hard		9	SS	50/	88		Ė								0				1 61	1 36
9.5	END OF BOREHOLE:	1/47	٦	00	76mn	PQC	237	H										╁		1 0	1 30
3.5	Borehole was open upon																				
	completion of drilling.  2. Water level was encountered at																				
	4.72 mbgs upon completion of																				
	drilling. 3. Monitoring well installed at the																				
	BH/MW-02Ă . 4. Groundwater level was measured																				
	at 4.9 mbgs on February 27, 2023.																				
- 1																					
					1	1	1	I		1	1	1	1	I	1	1	1	1	i	1	



GRAPH NOTES +  $^3$  , imes  $^3$  : Numbers refer to Sensitivity

 $\bigcirc$  8=3% Strain at Failure

REF. NO.: SP22-01117-00 A



### **LOG OF BOREHOLE BH-02**

PROJECT: Geotechnical and Hydrogeologival Investigations

CLIENT: Evans Planning

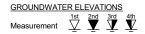
PROJECT LOCATION: 334 Ardagh Road, Barrie, ON

DRILLING DATA

Method: Solid Stem Auger

Diameter: 150 mm

	SOIL PROFILE		8	SAMPL	ES			DYNAMIC RESISTAN	CONE PEI	NETRAT	TION			NATUR	AL .			_	REMAF	٩K۶
(m) ELEV EPTH	DESCRIPTION	STRATA PLOT	NUMBER	ТҮРЕ	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION	20 SHEAR S O UNCO • QUICK 20	40 6 STRENG NFINED TRIAXIAL	TH (kF	0 10	NE ity NE	PLASTIC IMIT W <sub>P</sub> WATE		TENT (%	RUID IMIT W∟ -1 (6)	POCKET PEN. (Cu) (KPa)		ANE GRAIN S DISTRIBL (%)	) SIZ JTI
0.0 0.2 0.6	TOPSOIL: 200 mm  FILL: sand, trace gravel, trace silt, trace organics, brown, moist  SAND: some gravel, trace silt, brown, moist	X	2	GRAB GRAB	3								0		0					
1.1	END OF BOREHOLE:  1. Borehole was advanced by hand augering and was open and dry upon termination.								5	52										









### LOG OF BOREHOLE BH/MW-03

PROJECT: Geotechnical and Hydrogeologival Investigations

CLIENT: Evans Planning

PROJECT LOCATION: 334 Ardagh Road, Barrie, ON

DATUM: Geodetic

DRILLING DATA

Method: Solid Stem Auger

Diameter: 150 mm REF. NO.: SP22-01117-00 A

Date: Jan-20-2023 ENCL NO.: 5

SH LO	CATION: See Drawing 1 N 4911854.3	10 E				1	1	DYNA	MIC CO	NE PEI	VETRA	TION								
	SOIL PROFILE	SAMPLES (				DYNAMIC CONE PENETRATION RESISTANCE PLOT						PLASTI	C NATU	JRAL TURE	LIQUID	-i	TW.	REMARK		
(m)		10.			_ ω 	GROUND WATER CONDITIONS	_		1	1	1	1	00	LIMIT W <sub>P</sub>	CON	TENT	LIMIT W <sub>L</sub>	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m³)	AND GRAIN SI
LEV EPTH	DESCRIPTION	STRATA PLOT	띪		BLOWS 0.3 m	N V V	ELEVATION	SHE	AR ST	RENG INED RIAXIAL	1H (kl +	Pa)	ANE	—		<b>—</b>	—	OCKE (Cu) (F	URAL (KN/r	DISTRIBUT
		RA	NUMBER	TYPE	M	JON O	Α̈́							l .	TER CC		. ,	ď	NAT	(%)
47.2 4 <b>0.2</b>	ASPHALT: 40 mm	ı SXX	ž	, UGE		<u> </u>		╁	20 4	40 6	80 8	30 1	00	1	0 2	0 3	80			GR SA SI
40.Q	GRANULAR FILL: sand and		1	SS	27		247							0						
	gravel, 200 mm  FILL: sand, trace to some silt, trace	$\bigotimes$						Ė												
	gravel, trace organics, brown, moist, loose	$\bowtie$	2	SS	4		246	<u>.</u>							0					
	10036	$\bowtie$					240	<u>'</u>												
45.4 1.8	CAND: trace to some gravel trace	$\bowtie$	3	ss	41			Ē						0						
1.0	<b>SAND:</b> trace to some gravel, trace silt, brown, moist, dense, trace						245	;=												
	cobbles becoming poorly graded sand with		4	SS	54			Ē						0						36 57 6
	gravel, trace silt, very dense		Ľ		0.7			Ē						ľ						00 01 0
	dense		<u> </u>				244	Ē.												
			5	SS	33			E						0						
								Ē												
							243	\$ <del>[</del>				<u> </u>								
	wet		<u> </u>			$\{ \mid \mid \mid : \mid$		Ē												
			6	SS	47		 W. L.	<u>-</u> 242.3	m m			Ś	•	ĺ	0					
							Feb 2			<del>                                     </del>	7	>								
							:	E			L	•								
41.1			L					ŧ	•		\									
6.1	<b>SILTY SAND:</b> trace gravel, trace clay, brown, very moist to wet, very		7	SS	74	1	241	F		† <b>V</b>					0					1 68 30
	dense		Ĺ		′ -	:目:		Ė												1 00 0
	grey	陆	1					E												
							240	)=												
			ł				<u>'</u>	Ē												
		Hi	8	SS	50/			F							0					
38.9 8.3	END OF BOREHOLE:	j l j	Ů	- 00	100mr		239													
	Borehole was open upon																			
	completion of drilling.																			
	<ul><li>2. Groundwater was encountered at</li><li>4.57 mbgs upon completion of</li></ul>																			
	drilling. 3. Monitoring well was installed in																			
	the borehole upon completion of																			
	drilling. 4. Groundwater level																			
		1	I				1	1		1		1	1	I		1	1	1		



### LOG OF BOREHOLE BH/MW-04

PROJECT: Geotechnical and Hydrogeologival Investigations

CLIENT: Evans Planning

PROJECT LOCATION: 334 Ardagh Road, Barrie, ON

DATUM: Geodetic

DRILLING DATA

Method: Solid Stem Auger

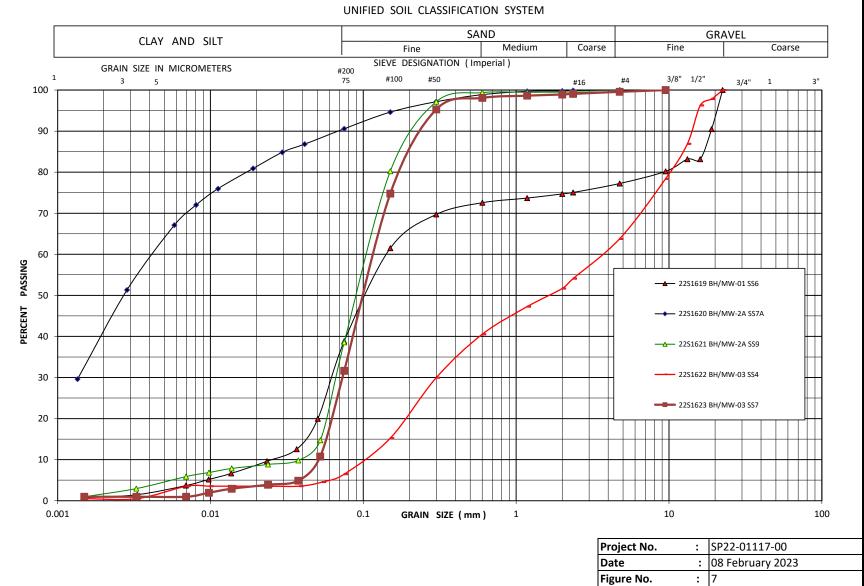
Diameter: 150 mm REF. NO.: SP22-01117-00 A

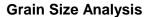
Date: Jan-20-2023 ENCL NO.: 6

	SOIL PROFILE		S	SAMPL	.ES			DYNA	AMIC CO	ONE PEI	NETRA	IION								5	44 D.C.	
(m)		PLOT		uvii L		WATER ONS	NO NO	DYNAMIC CONE PENETRATION RESISTANCE PLOT  20 40 60 80 100  SHEAR STRENGTH (kPa)  O UNCONFINED + FIELD VANE O UNCONFINED + & Sensitivity OUICK TRIAXIAL × LAB VANE						PLASTIC MOISTURE LIMIT CONTENT W <sub>P</sub> W			LIQUID LIMIT W.	POCKET PEN. (Cu) (kPa)	NATURAL UNIT WT (kN/m³)	REMARKS AND GRAIN SIZE DISTRIBUTIO		
EPTH 246.6		STRATA PLOT	NUMBER	TYPE	"N" BLOWS 0.3 m	GROUND WATER CONDITIONS	ELEVATION					FIÉLD VANE & Sensitivity LAB VANE 80 100		WATER CONTEN			NT (%)		NATUR S)	(%) GR SA SI		
4 <b>6.4</b> 0.2	TOPSOIL: 230 mm  FILL: sand, trace to some silt, trace gravel, trace organics, trace rootlets, brown, moist, loose		1	SS	1		246								0							
	brown, most, loose		2	SS	1																	
	gravelly sand, trace silt		3	SS	3		245							0								
3.9	POORLY GRADED SAND WITH	$\bowtie$	4	SS	10		244							-								
	<b>GRAVEL:</b> trace cobbles, trace silt, brown, moist, dense trace cobbles		5	SS	37		243							0				-		37 57	<b>7</b> (	
	very dense		6	SS	53		242					4	_	0				-				
0.5	wet						W. L. 2 Feb 27 241	241.6 7, 202	m 23		5/2	<b>`</b>						-				
6.1	<b>LEAN CLAY WITH SAND:</b> trace gravel, grey, wet, very stiff		7	SS	21		240									•		-		1 25	5 44	
	stiff						239											-		0.0		
38.4 8.2	END OF BOREHOLE:		8	SS	14			-							0					29	57	
	Borehole was open upon completion of drilling.     Groundwater was encountered at 5.79 mbgs upon completion of drilling.     Monitoring well was installed in the borehole upon completion of drilling.     Groundwater level																					



### **GRAIN SIZE DISTRIBUTION**







(Granular Material)

MTO LS-602

FIGURE NUMBER:

SAMPLE NUMBER: 22S1624 Date Tested: 3 Feb 2023

PROJECT NUMBER: SP22-1117-00

PROJECT NAME:

CLIENT: 334 – 340 Ardagh Road, Barrie, ON

PROJECT LOCATION:

SAMPLED BY: DATE SAMPLED:

SUPPLIER:

SAMPLE LOCATION:

DESCRIPTION: Sample BH/MW-04 SS5
SPECIFICATION: OPSS 1010 Granular A

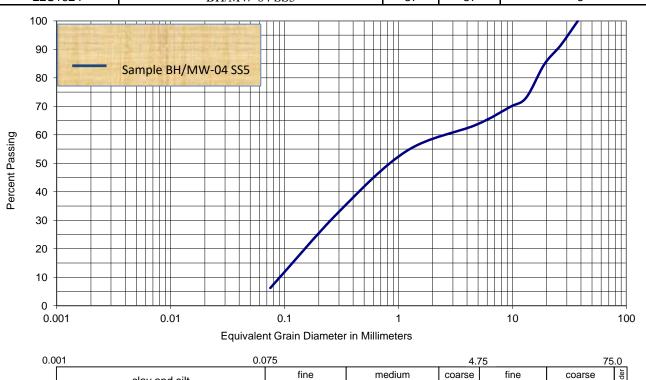
clay and silt

**COMMENTS:** 

SIEVE SIZE	PERCENT PASSING	SPECIFICATIONS
37.5 mm	100.00	Upper limit
26.5 mm	91.44	100
19.0 mm	84.70	85-100
13.2 mm	73.06	65-90
9.5 mm	69.77	50-73
4.75 mm	63.45	35-55
1.18 mm	54.35	15-40
300 µm	32.98	5-22
75 µm	6.23	2-8

gravel

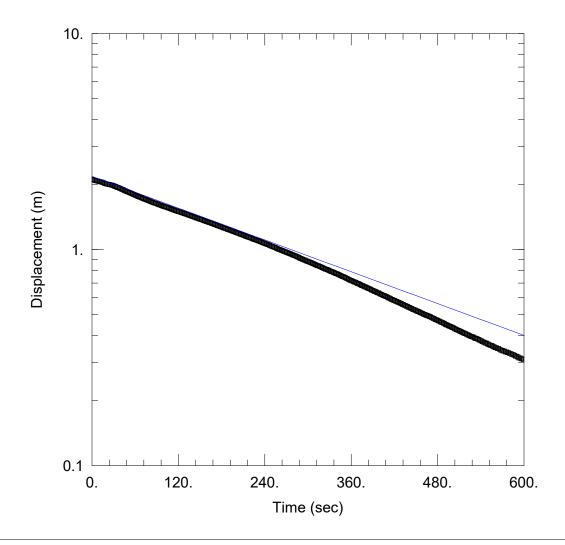
			Pei	rcentage of
Sample No.	BH-SS	Gravel	Sand	Fines(Silt and Clay)
22S1624	BH/MW-04 SS5	37	57	6



sand

# APPENDIX C





# RISING HEAD

Data Set: Z:\...\BH MW-02A.aqt

Date: 03/02/23 Time: 10:11:28

# PROJECT INFORMATION

Company: SIRATI & PARTNERS

Project: <u>SP22-01117-00</u>

Location: 334 Ardagh Rd., Barrie, ON

Test Well: BH/MW-02A
Test Date: 2023-February-27

## **AQUIFER DATA**

Saturated Thickness: 2.61 m Anisotropy Ratio (Kz/Kr): 1.

# WELL DATA (BH/MW-02A)

Initial Displacement: 2.115 m

Total Well Penetration Depth: 7.92 m

Casing Radius: 0.0508 m

Water level below top of screen

Static Water Column Height: 2.61 m

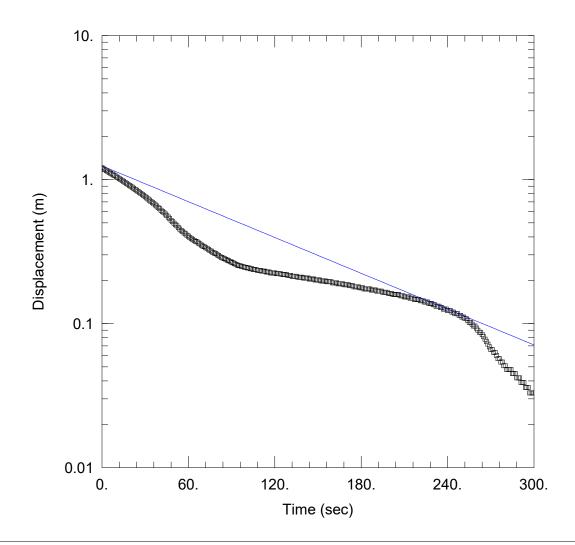
Screen Length: 3.04 m Well Radius: 0.0508 m

# **SOLUTION**

Aquifer Model: Unconfined

Solution Method: Hvorslev

K = 7.404E-6 m/sec y0 = 2.183 m



# RISING HEAD

Data Set: Z:\...\BH MW-04.aqt

Date: 03/02/23 Time: 10:41:43

# PROJECT INFORMATION

Company: SIRATI & PARTNERS

Project: SP22-01117-00

Location: 340 Ardagh Rd., Barrie, ON

Test Well: BH/MW-04

Test Date: 2023-February-27

## **AQUIFER DATA**

Saturated Thickness: 2.53 m Anisotropy Ratio (Kz/Kr): 1.

# WELL DATA (BH/MW-04)

Initial Displacement: 1.206 m

Static Water Column Height: 2.53 m

Total Well Penetration Depth: 7.57 m

Screen Length: 3. m Well Radius: 0.0508 m

Casing Radius: 0.0508 m

# **SOLUTION**

Aquifer Model: Unconfined

Solution Method: Hvorslev

K = 2.579E-5 m/sec

y0 = 1.245 m

# APPENDIX D





CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD 160 KONRAD CRESCENT UNIT 4 MARKHAM, ON L3R 9T9

(905) 833-1582

ATTENTION TO: Hiva Elhami

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528

MISCELLANEOUS ANALYSIS REVIEWED BY: Amanjot Bhela, Lab Operation Manager

TRACE ORGANICS REVIEWED BY: Radhika Chakraberty, Trace Organics Lab Manager

WATER ANALYSIS REVIEWED BY: Yris Verastegui, Inorganic Team Lead

DATE REPORTED: May 21, 2024

PAGES (INCLUDING COVER): 15 VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*Notes	

#### Disclaimer:

- All work conducted herein has been done using accepted standard protocols, and generally accepted practices and methods. AGAT test methods may
  incorporate modifications from the specified reference methods to improve performance.
- All samples will be disposed of within 30 days after receipt unless a Long Term Storage Agreement is signed and returned. Some specialty analysis may
  be exempt, please contact your Client Project Manager for details.
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  third party. Unless expressly agreed otherwise in writing, AGAT's liability is limited to the actual cost of the specific analysis or analyses included in the
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- This Certificate shall not be reproduced except in full, without the written approval of the laboratory.
- The test results reported herewith relate only to the samples as received by the laboratory.
- Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, warranties of
  merchantability, fitness for a particular purpose, or non-infringement. AGAT assumes no responsibility for any errors or omissions in the guidelines
  contained in this document.
- All reportable information as specified by ISO/IEC 17025:2017 is available from AGAT Laboratories upon request.
- For environmental samples in the Province of Quebec: The analysis is performed on and results apply to samples as received. A temperature above 6°C upon receipt, as indicated in the Sample Reception Notification (SRN), could indicate the integrity of the samples has been compromised if the delay between sampling and submission to the laboratory could not be minimized.

AGAT Laboratories (V1)

Page 1 of 15

Member of: Association of Professional Engineers and Geoscientists of Alberta (APEGA)

Western Enviro-Agricultural Laboratory Association (WEALA) Environmental Services Association of Alberta (ESAA) AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation. AGAT Laboratories (Mississauga) is also accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) for specific drinking water tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. Measurement Uncertainty is not taken into consideration when stating conformity with a specified requirement.



SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

# **Certificate of Analysis**

**AGAT WORK ORDER: 24T148528** 

PROJECT: SP22-01117-00

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

**ATTENTION TO: Hiva Elhami SAMPLED BY:Hiva** 

# **Barrie Sanitary - Organics**

DATE RECEIVED: 2024-05-08								<b>DATE REPORTED: 2024-05-21</b>
Parameter	Unit		CRIPTION: PLE TYPE: SAMPLED:	BH/MW-01 Water 2024-05-07 11:40 5846407	BH/MW-02A Water 2024-05-07 12:48 5846873	BH/MW-03 Water 2024-05-07 13:50 5846874	BH/MW-04 Water 2024-05-07 15:10 5846875	
Oil and Grease (animal/vegetable) in water	mg/L	150	0.5	0.77	0.63	0.63	0.50	
Oil and Grease (mineral) in water	mg/L	15	0.5	<0.5	<0.5	<0.5	<0.5	
Dichloromethane	mg/L	0.09	0.0003	< 0.0003	< 0.0003	< 0.0003	<0.0003	
Benzene	mg/L	0.01	0.0002	< 0.0002	< 0.0002	< 0.0002	<0.0002	
Trichloroethylene	mg/L	0.05	0.0002	0.0019	0.0004	0.0004	0.0009	
Toluene	mg/L	0.02	0.0002	< 0.0002	0.0004	0.0005	<0.0002	
Tetrachloroethene	mg/L	0.06	0.0002	< 0.0002	< 0.0002	< 0.0002	<0.0002	
Ethylbenzene	mg/L	0.06	0.0001	< 0.0001	< 0.0001	0.0002	<0.0001	
1,1,2,2-Tetrachloroethane	mg/L	0.06	0.0001	<0.0001	< 0.0001	<0.0001	<0.0001	
1,4-Dichlorobenzene	mg/L	0.08	0.0001	< 0.0001	< 0.0001	< 0.0001	<0.0001	
1,2-Dichlorobenzene	mg/L	0.05	0.0001	< 0.0001	< 0.0001	< 0.0001	<0.0001	
Xylenes (Total)	mg/L	0.3	0.0002	< 0.0002	0.0003	0.0004	<0.0002	
Hexachlorobenzene	mg/L	0.0001	0.00001	< 0.00001	<0.00001	< 0.00001	<0.00001	
Naphthalene	mg/L		0.0003	< 0.0003	< 0.0003	< 0.0003	<0.0003	
Acenaphthylene	mg/L		0.00011	< 0.00011	< 0.00011	< 0.00011	<0.00011	
Acenaphthene	mg/L		0.00010	< 0.00010	< 0.00010	< 0.00010	<0.00010	
Fluorene	mg/L		0.0002	< 0.0002	< 0.0002	<0.0002	<0.0002	
Phenanthrene	mg/L		0.00011	< 0.00011	< 0.00011	< 0.00011	<0.00011	
Anthracene	mg/L		0.0001	< 0.0001	< 0.0001	<0.0001	<0.0001	
Fluoranthene	mg/L		0.00012	< 0.00012	< 0.00012	< 0.00012	<0.00012	
Pyrene	mg/L		0.00012	< 0.00012	< 0.00012	0.00042	<0.00012	
Benzo(a)anthracene	mg/L		0.0001	<0.0001	<0.0001	<0.0001	<0.0001	
Chrysene	mg/L		0.0001	<0.0001	<0.0001	0.0003	<0.0001	
Benzo(b)fluoranthene	mg/L		0.0001	<0.0001	<0.0001	0.0003	<0.0001	
Benzo(k)fluoranthene	mg/L		0.0001	<0.0001	<0.0001	<0.0001	<0.0001	
Benzo(a)pyrene	mg/L		0.001	<0.001	<0.001	<0.001	<0.001	
Indeno(1,2,3-cd)pyrene	mg/L		0.0001	<0.0001	<0.0001	<0.0001	<0.0001	
Dibenzo(a,h)anthracene	mg/L		0.0001	<0.0001	<0.0001	<0.0001	<0.0001	
Benzo(ghi)perylene	mg/L		0.0001	< 0.0001	< 0.0001	< 0.0001	<0.0001	

Certified By:

R. Chakraberty



SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

**Certificate of Analysis** 

AGAT WORK ORDER: 24T148528

PROJECT: SP22-01117-00

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

ATTENTION TO: Hiva Elhami

**SAMPLED BY:Hiva** 

### **Barrie Sanitary - Organics**

						0.900		
DATE RECEIVED: 2024-05-08								DATE REPORTED: 2024-05-21
	S	AMPLE DES	CRIPTION:	BH/MW-01	BH/MW-02A	BH/MW-03	BH/MW-04	
		SAM	PLE TYPE:	Water	Water	Water	Water	
		DATE	SAMPLED:	2024-05-07 11:40	2024-05-07 12:48	2024-05-07 13:50	2024-05-07 15:10	
Parameter	Unit	G/S	RDL	5846407	5846873	5846874	5846875	
Total PAHs	mg/L	0.005	0.0003	<0.0003	< 0.0003	0.0010	<0.0003	
Surrogate	Unit	Acceptal	ole Limits					
Toluene-d8	% Recovery	50-	140	95	93	101	94	
4-Bromofluorobenzene	% Recovery	50-	140	100	96	103	98	
тсмх	%	50-	140	97	104	86	84	
2-Fluorophenol	%	50-	140	79	74	70	85	
phenol-d6 surrogate	%	50-	140	71	85	73	74	
2,4,6-Tribromophenol	%	50-	140	88	81	85	77	
Chrysene-d12	%	50-	140	89	88	99	88	

Comments:

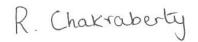
RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Limits for Sanitary Sewer Discharge - The City of Barrie - By-Law No. 2012-172

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5846407-5846875 Oil and Grease animal/vegetable is a calculated parameter. The calculated value is the difference between Total O&G and Mineral O&G. Xylenes total is a calculated parameter. The calculated value is the sum of m&p-Xylene and o-Xylene.

Analysis performed at AGAT Toronto (unless marked by \*)

Certified By:





**Certificate of Analysis** 

**AGAT WORK ORDER: 24T148528** 

PROJECT: SP22-01117-00

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

**ATTENTION TO: Hiva Elhami** 

SAMPLING SITE:334 and 340 A	Ardagh Rd.	Barrie		SAMPLED BY:Hiva								
			BOD5									
DATE RECEIVED: 2024-05-08									DATE REPORTED: 2024-05-21			
		SAMPLE DE		BH/MW-01	BH/MW-02A	BH/MW-03	BH/MW-04					
				MPLE TYPE: SAMPLED:	Water 2024-05-07 11:40	Water 2024-05-07 12:48	Water 2024-05-07 13:50	Water 2024-05-07 15:10				
Parameter	Unit	G / S: A	G / S: B	RDL	5846407	5846873	5846874	5846875				
Biochemical Oxygen Demand, Total	mg/L	300	15	2	<2[ <b]< td=""><td>&lt;2[<b]< td=""><td>&lt;2[<b]< td=""><td>&lt;2[<b]< td=""><td></td></b]<></td></b]<></td></b]<></td></b]<>	<2[ <b]< td=""><td>&lt;2[<b]< td=""><td>&lt;2[<b]< td=""><td></td></b]<></td></b]<></td></b]<>	<2[ <b]< td=""><td>&lt;2[<b]< td=""><td></td></b]<></td></b]<>	<2[ <b]< td=""><td></td></b]<>				

Comments:

RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Limits for Sanitary Sewer Discharge - The City of Barrie - By-Law No. 2012-172, B Refers to Limits for Storm Sewer Discharge -The City of Barrie - By-Law No. 2012-172

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

Analysis performed at AGAT Halifax (unless marked by \*)

Certified By:





SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

# **Certificate of Analysis**

AGAT WORK ORDER: 24T148528

PROJECT: SP22-01117-00

ATTENTION TO: Hiva Elhami

**SAMPLED BY:Hiva** 

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

# Barrie Sanitary/Storm Sewer Use By-law - Inorganics

DATE RECEIVED: 2024-05-08	3								DATE REPORTE	D: 2024-05-21	
				SCRIPTION: IPLE TYPE: SAMPLED:	BH/MW-01 Water 2024-05-07 11:40		BH/MW-02A Water 2024-05-07 12:48		BH/MW-03 Water 2024-05-07 13:50		BH/MW-04 Water 2024-05-07 15:10
Parameter	Unit	G / S: A	G/S:B	RDL	5846407	RDL	5846873	RDL	5846874	RDL	5846875
pH	pH Units	6.0-9.5	6.0-9.5	NA	8.08	NA	7.83	NA	7.83	NA	7.99
Chemical Oxygen Demand	mg/L	600		5	14[ <a]< td=""><td>5</td><td>6[<a]< td=""><td>5</td><td>12[<a]< td=""><td>5</td><td>10[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	5	6[ <a]< td=""><td>5</td><td>12[<a]< td=""><td>5</td><td>10[<a]< td=""></a]<></td></a]<></td></a]<>	5	12[ <a]< td=""><td>5</td><td>10[<a]< td=""></a]<></td></a]<>	5	10[ <a]< td=""></a]<>
Total Suspended Solids	mg/L	350	15	10	4030[>A]	10	10700[>A]	10	9020[>A]	10	4500[>A]
Total Phosphorus	mg/L	10		0.06	1.93[ <a]< td=""><td>0.06</td><td>3.72[<a]< td=""><td>0.06</td><td>3.64[<a]< td=""><td>0.06</td><td>3.74[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.06	3.72[ <a]< td=""><td>0.06</td><td>3.64[<a]< td=""><td>0.06</td><td>3.74[<a]< td=""></a]<></td></a]<></td></a]<>	0.06	3.64[ <a]< td=""><td>0.06</td><td>3.74[<a]< td=""></a]<></td></a]<>	0.06	3.74[ <a]< td=""></a]<>
Total Kjeldahl Nitrogen	mg/L	100		0.10	<0.10[ <a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.10	<0.10[ <a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""></a]<></td></a]<></td></a]<>	0.10	<0.10[ <a]< td=""><td>0.10</td><td>&lt;0.10[<a]< td=""></a]<></td></a]<>	0.10	<0.10[ <a]< td=""></a]<>
Fluoride	mg/L	10		0.05	<0.05[ <a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.05	<0.05[ <a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""></a]<></td></a]<></td></a]<>	0.05	<0.05[ <a]< td=""><td>0.05</td><td>&lt;0.05[<a]< td=""></a]<></td></a]<>	0.05	<0.05[ <a]< td=""></a]<>
Chloride	mg/L	1500		0.10	90.0[ <a]< td=""><td>0.49</td><td>513[<a]< td=""><td>0.24</td><td>436[<a]< td=""><td>0.12</td><td>237[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.49	513[ <a]< td=""><td>0.24</td><td>436[<a]< td=""><td>0.12</td><td>237[<a]< td=""></a]<></td></a]<></td></a]<>	0.24	436[ <a]< td=""><td>0.12</td><td>237[<a]< td=""></a]<></td></a]<>	0.12	237[ <a]< td=""></a]<>
Sulphate	mg/L	1500		0.10	11.1[ <a]< td=""><td>0.38</td><td>27.1[<a]< td=""><td>0.19</td><td>29.8[<a]< td=""><td>0.10</td><td>20.3[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.38	27.1[ <a]< td=""><td>0.19</td><td>29.8[<a]< td=""><td>0.10</td><td>20.3[<a]< td=""></a]<></td></a]<></td></a]<>	0.19	29.8[ <a]< td=""><td>0.10</td><td>20.3[<a]< td=""></a]<></td></a]<>	0.10	20.3[ <a]< td=""></a]<>
Sulphide	mg/L	1.0		0.01	<0.01[ <a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.01	<0.01[ <a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""></a]<></td></a]<></td></a]<>	0.01	<0.01[ <a]< td=""><td>0.01</td><td>&lt;0.01[<a]< td=""></a]<></td></a]<>	0.01	<0.01[ <a]< td=""></a]<>
PhenoIs	mg/L	0.1		0.002	0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<>	0.002	<0.002[ <a]< td=""></a]<>
Cyanide, SAD	mg/L	1.2		0.002	<0.002[ <a]< td=""><td>0.002</td><td>0.004[<a]< td=""><td>0.002</td><td>0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.002	0.004[ <a]< td=""><td>0.002</td><td>0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<>	0.002	0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<>	0.002	<0.002[ <a]< td=""></a]<>
Total Aluminum	mg/L	50		0.20	94.4[>A]	0.20	98.8[>A]	0.20	83.0[>A]	0.20	42.8[ <a]< td=""></a]<>
Total Antimony	mg/L	5.0		0.003	<0.003[ <a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.003	<0.003[ <a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""></a]<></td></a]<></td></a]<>	0.003	<0.003[ <a]< td=""><td>0.003</td><td>&lt;0.003[<a]< td=""></a]<></td></a]<>	0.003	<0.003[ <a]< td=""></a]<>
Total Arsenic	mg/L	1.0		0.06	<0.06[ <a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.06	<0.06[ <a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""></a]<></td></a]<></td></a]<>	0.06	<0.06[ <a]< td=""><td>0.06</td><td>&lt;0.06[<a]< td=""></a]<></td></a]<>	0.06	<0.06[ <a]< td=""></a]<>
Total Barium	mg/L	5.0		0.04	0.96[ <a]< td=""><td>0.04</td><td>1.22[<a]< td=""><td>0.04</td><td>1.06[<a]< td=""><td>0.04</td><td>0.57[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	1.22[ <a]< td=""><td>0.04</td><td>1.06[<a]< td=""><td>0.04</td><td>0.57[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	1.06[ <a]< td=""><td>0.04</td><td>0.57[<a]< td=""></a]<></td></a]<>	0.04	0.57[ <a]< td=""></a]<>
Total Bismuth	mg/L	5.0		0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<>	0.04	<0.04[ <a]< td=""></a]<>
Total Cadmium	mg/L	0.7	0.001	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<>	0.002	<0.002[ <a]< td=""></a]<>
Total Chromium	mg/L	2.0	0.08	0.06	0.12[B-A]	0.06	0.14[B-A]	0.06	0.13[B-A]	0.06	<0.06[ <b]< td=""></b]<>
Total Cobalt	mg/L	5.0		0.010	0.053[ <a]< td=""><td>0.010</td><td>0.043[<a]< td=""><td>0.010</td><td>0.049[<a]< td=""><td>0.010</td><td>0.035[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.010	0.043[ <a]< td=""><td>0.010</td><td>0.049[<a]< td=""><td>0.010</td><td>0.035[<a]< td=""></a]<></td></a]<></td></a]<>	0.010	0.049[ <a]< td=""><td>0.010</td><td>0.035[<a]< td=""></a]<></td></a]<>	0.010	0.035[ <a]< td=""></a]<>
Total Copper	mg/L	2.0	0.01	0.04	0.12[B-A]	0.04	0.10[B-A]	0.04	0.15[B-A]	0.04	0.05[B-A]
Total Iron	mg/L	50		1.00	112[>A]	1.00	115[>A]	1.00	127[>A]	1.00	56.5[>A]
Total Lead	mg/L	0.7	0.05	0.010	0.038[ <b]< td=""><td>0.010</td><td>0.040[<b]< td=""><td>0.010</td><td>0.046[<b]< td=""><td>0.010</td><td>0.020[<b]< td=""></b]<></td></b]<></td></b]<></td></b]<>	0.010	0.040[ <b]< td=""><td>0.010</td><td>0.046[<b]< td=""><td>0.010</td><td>0.020[<b]< td=""></b]<></td></b]<></td></b]<>	0.010	0.046[ <b]< td=""><td>0.010</td><td>0.020[<b]< td=""></b]<></td></b]<>	0.010	0.020[ <b]< td=""></b]<>
Total Manganese	mg/L	5.0		0.04	3.37[ <a]< td=""><td>0.04</td><td>3.59[<a]< td=""><td>0.04</td><td>3.58[<a]< td=""><td>0.04</td><td>2.19[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	3.59[ <a]< td=""><td>0.04</td><td>3.58[<a]< td=""><td>0.04</td><td>2.19[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	3.58[ <a]< td=""><td>0.04</td><td>2.19[<a]< td=""></a]<></td></a]<>	0.04	2.19[ <a]< td=""></a]<>
Total Mercury	mg/L	0.01		0.0002	<0.0002[ <a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.0002	<0.0002[ <a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""></a]<></td></a]<></td></a]<>	0.0002	<0.0002[ <a]< td=""><td>0.0002</td><td>&lt;0.0002[<a]< td=""></a]<></td></a]<>	0.0002	<0.0002[ <a]< td=""></a]<>
Total Molybdenum	mg/L	5.0		0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<>	0.04	<0.04[ <a]< td=""></a]<>
Total Nickel	mg/L	2.0	0.05	0.06	0.12[B-A]	0.06	0.14[B-A]	0.06	0.27[B-A]	0.06	0.07[B-A]
Total Selenium	mg/L	1.0		0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>0.05[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>0.05[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>0.05[<a]< td=""></a]<></td></a]<>	0.04	0.05[ <a]< td=""></a]<>
Total Silver	mg/L	0.4		0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<></td></a]<>	0.002	<0.002[ <a]< td=""><td>0.002</td><td>&lt;0.002[<a]< td=""></a]<></td></a]<>	0.002	<0.002[ <a]< td=""></a]<>
Total Tin	mg/L	5.0		0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	<0.04[ <a]< td=""><td>0.04</td><td>&lt;0.04[<a]< td=""></a]<></td></a]<>	0.04	<0.04[ <a]< td=""></a]<>

Certified By:

Inis Verastegui



SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

**Certificate of Analysis** 

**AGAT WORK ORDER: 24T148528** 

PROJECT: SP22-01117-00

ATTENTION TO: Hiva Elhami

**SAMPLED BY:Hiva** 

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

# Barrie Sanitary/Storm Sewer Use By-law - Inorganics

				•		,	•				
DATE RECEIVED: 2024-05-08									DATE REPORTE	D: 2024-05-2	
			SAMPLE DE	SCRIPTION:	BH/MW-01		BH/MW-02A		BH/MW-03		BH/MW-04
			SAM	MPLE TYPE:	Water		Water		Water		Water
			DATE	SAMPLED:	2024-05-07 11:40		2024-05-07 12:48		2024-05-07 13:50		2024-05-07 15:10
Parameter	Unit	G / S: A	G / S: B	RDL	5846407	RDL	5846873	RDL	5846874	RDL	5846875
Total Vanadium	mg/L	5.0		0.04	0.20[ <a]< td=""><td>0.04</td><td>0.15[<a]< td=""><td>0.04</td><td>0.16[<a]< td=""><td>0.04</td><td>0.09[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	0.04	0.15[ <a]< td=""><td>0.04</td><td>0.16[<a]< td=""><td>0.04</td><td>0.09[<a]< td=""></a]<></td></a]<></td></a]<>	0.04	0.16[ <a]< td=""><td>0.04</td><td>0.09[<a]< td=""></a]<></td></a]<>	0.04	0.09[ <a]< td=""></a]<>
Total Zinc	mg/L	2.0	0.04	0.40	<0.40[ <a]< td=""><td>0.40</td><td>0.40[B-A]</td><td>0.40</td><td>&lt;0.40[<a]< td=""><td>0.40</td><td>&lt;0.40[<a]< td=""></a]<></td></a]<></td></a]<>	0.40	0.40[B-A]	0.40	<0.40[ <a]< td=""><td>0.40</td><td>&lt;0.40[<a]< td=""></a]<></td></a]<>	0.40	<0.40[ <a]< td=""></a]<>

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Limits for Sanitary Sewer Discharge - The City of Barrie - By-Law No. 2012-172, B Refers to Limits for Storm Sewer Discharge -

The City of Barrie - By-Law No. 2012-172

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5846407-5846875 Dilution required, RDL has been increased accordingly.

Analysis performed at AGAT Toronto (unless marked by \*)

Certified By:

Yrus Verastegui



# **Exceedance Summary**

AGAT WORK ORDER: 24T148528

**ATTENTION TO: Hiva Elhami** 

**Total Suspended Solids** 

PROJECT: SP22-01117-00

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

**ANALYSIS PACKAGE UNIT GUIDEVALUE RESULT** SAMPLEID SAMPLE TITLE **GUIDELINE PARAMETER** 5846407 BH/MW-01 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Chromium** mg/L 0.08 0.12 5846407 BH/MW-01 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics mg/L 0.01 0.12 **Total Copper** 5846407 BH/MW-01 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Nickel mg/L 0.05 0.12 5846407 BH/MW-01 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Suspended Solids** mg/L 15 4030 5846407 BH/MW-01 ON Barrie SN **Total Aluminum** 50 Barrie Sanitary/Storm Sewer Use By-law - Inorganics mg/L 94.4 50 5846407 BH/MW-01 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Iron mg/L 112 350 5846407 BH/MW-01 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Suspended Solids** mg/L 4030 5846873 BH/MW-02A ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Chromium** mg/L 0.08 0.14 BH/MW-02A ON Barrie SM 0.01 0.10 5846873 Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Copper** mg/L 5846873 BH/MW-02A ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Nickel mg/L 0.05 0.14 5846873 BH/MW-02A ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Suspended Solids** 15 10700 mg/L 5846873 BH/MW-02A ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Zinc mg/L 0.04 0.40 5846873 BH/MW-02A ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Aluminum mg/L 50 98.8 5846873 BH/MW-02A ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Iron 50 115 mg/L ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Suspended Solids** 350 10700 5846873 BH/MW-02A mg/L 5846874 BH/MW-03 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Chromium** mg/L 0.08 0.13 0.01 5846874 BH/MW-03 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Copper** mg/L 0.15 5846874 BH/MW-03 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Nickel mg/L 0.05 0.27 BH/MW-03 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Suspended Solids** 15 9020 5846874 mg/L 5846874 BH/MW-03 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Aluminum 50 83.0 mg/L Total Iron 50 127 5846874 BH/MW-03 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics mg/L 5846874 BH/MW-03 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Suspended Solids 350 9020 mg/L BH/MW-04 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics **Total Copper** 0.01 0.05 5846875 mg/L 5846875 BH/MW-04 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Nickel mg/L 0.05 0.07 Total Suspended Solids 15 4500 5846875 BH/MW-04 ON Barrie SM Barrie Sanitary/Storm Sewer Use By-law - Inorganics mg/L 50 5846875 BH/MW-04 ON Barrie SN Barrie Sanitary/Storm Sewer Use By-law - Inorganics Total Iron ma/L 56.5

Barrie Sanitary/Storm Sewer Use By-law - Inorganics

BH/MW-04

ON Barrie SN

5846875

4500

350

mg/L



# **Quality Assurance**

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528
ATTENTION TO: Hiva Elhami

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

SAMPLED BY:Hiva

PARAMETER   Batch   Sample   Dup #1   Dup #2   RPD   Blank   Measured Park   Limits   Dup #2   RPD   Blank   Measured Park   Dup #2   RPD   Dup #2   RPD   Blank   Measured Park   Dup #2   RPD   Dup #2   Dup #	Trace Organics Analysis															
PARAMETER   Batch   Sample   Dup #1   Dup #2   RPD   Blank   Mesure   Value   Limits   Low   Upper   Limits   Value   Upper	RPT Date: May 21, 2024			С	DUPLICATE			REFERE	NCE MA	TERIAL	METHOD BLANK SPIKE			MATRIX SPIKE		
Barrie Sanitary - Organics   Organics   Sanitary - Organics   Or	PARAMETER	Batch		Dup #1	Dup #2	RPD					Recovery			Recovery		
Coli and Grease (animal/vegetable)   5845047			Ia	·				value	Lower	Upper		Lower	Upper		Lower	Upper
Nater   Coli and Grease (mineral) in water   5845047   Col. 5   Col. 5   NA   Col. 5   96%   70%   130%   92%   70%   130%   93%   70%   120	Barrie Sanitary - Organics															
Dichloromethane   5847389	,	5845047		< 0.5	< 0.5	NA	< 0.5	88%	70%	130%	103%	70%	130%	105%	70%	130%
Benzene   S847389	Oil and Grease (mineral) in water	5845047		< 0.5	< 0.5	NA	< 0.5	96%	70%	130%	92%	70%	130%	93%	70%	130%
Trichloroethylene 5847389	Dichloromethane	5847389		<0.0003	< 0.0003	NA	< 0.0003	71%	50%	140%	102%	60%	130%	91%	50%	140%
Toluene 5847389	Benzene	5847389		<0.0002	< 0.0002	NA	< 0.0002	69%	50%	140%	76%	60%	130%	79%	50%	140%
Tetrachloroethene	Trichloroethylene	5847389		<0.0002	<0.0002	NA	< 0.0002	72%	50%	140%	85%	60%	130%	74%	50%	140%
Ethylbenzene 5847389	Toluene	5847389		<0.0002	<0.0002	NA	< 0.0002	73%	50%	140%	79%	60%	130%	78%	50%	140%
1,1,2,2-Tetrachloroethane 5847389	Tetrachloroethene	5847389		0.0020	0.0018	10.0%	< 0.0002	65%	50%	140%	68%	60%	130%	76%	50%	140%
1,4-Dichlorobenzene       5847389       <0.0001	Ethylbenzene	5847389		<0.0001	<0.0001	NA	< 0.0001	71%	50%	140%	74%	60%	130%	74%	50%	140%
1,2-Dichlorobenzene	1,1,2,2-Tetrachloroethane	5847389		<0.0001	<0.0001	NA	< 0.0001	85%	50%	140%	74%	60%	130%	71%	50%	140%
Hexachlorobenzene 5841788	1,4-Dichlorobenzene	5847389		<0.0001	<0.0001	NA	< 0.0001	78%	50%	140%	79%	60%	130%	83%	50%	140%
Naphthalene 5848458	1,2-Dichlorobenzene	5847389		<0.0001	<0.0001	NA	< 0.0001	86%	50%	140%	81%	60%	130%	86%	50%	140%
Acenaphthylene 5848458	Hexachlorobenzene	5841788		< 0.00001	< 0.00001	NA	< 0.00001	98%	50%	140%	99%	50%	140%	82%	50%	140%
Acenaphthene 5848458	Naphthalene	5848458		< 0.0003	< 0.0003	NA	< 0.0003	108%	50%	140%	73%	50%	140%	74%	50%	140%
Fluorene 5848458 < 0.0001 < 0.0002	Acenaphthylene	5848458		< 0.00011	< 0.00011	NA	< 0.00011	94%	50%	140%	82%	50%	140%	85%	50%	140%
Phenanthrene         5848458         < 0.00011 < 0.00011         NA         < 0.00011 103%         50% 140%         71%         50% 140%         89%         50% 14           Anthracene         5848458         < 0.00007 < 0.00007	Acenaphthene	5848458	•	< 0.00010	< 0.00010	NA	< 0.00010	95%	50%	140%	74%	50%	140%	99%	50%	140%
Anthracene 5848458	Fluorene	5848458		< 0.0002	< 0.0002	NA	< 0.0002	103%	50%	140%	77%	50%	140%	85%	50%	140%
Fluoranthene 5848458	Phenanthrene	5848458		< 0.00011	< 0.00011	NA	< 0.00011	I 103%	50%	140%	71%	50%	140%	89%	50%	140%
Pyrene         5848458         < 0.00012 < 0.00012         NA         < 0.00012         110%         50%         140%         71%         50%         140%         85%         50%         14           Benzo(a)anthracene         5848458         < 0.00008 < 0.00008	Anthracene	5848458		< 0.00007	< 0.00007	NA	< 0.00007	106%	50%	140%	73%	50%	140%	85%	50%	140%
Benzo(a)anthracene 5848458	Fluoranthene	5848458		< 0.00012	< 0.00012	NA	< 0.00012	113%	50%	140%	73%	50%	140%	96%	50%	140%
Chrysene 5848458 < 0.00005 < 0.00005 NA < 0.00005 111% 50% 140% 100% 50% 140% 74% 50% 1408 Penzo(b)fluoranthene 5848458 < 0.00003 < 0.00003 NA < 0.00003 101% 50% 140% 99% 50% 140% 81% 50% 1408 Penzo(k)fluoranthene 5848458 < 0.00006 < 0.00006 NA < 0.00006 111% 50% 140% 100% 50% 140% 85% 50% 1408 Penzo(a)pyrene 5848458 < 0.001 < 0.001 NA < 0.001 100% 50% 140% 69% 50% 140% 88% 50% 1408 Penzo(a)pyrene 5848458 < 0.00003 < 0.00003 NA < 0.00003 82% 50% 140% 93% 50% 140% 96% 50% 140% Penzo(a)pyrene 5848458 < 0.00003 < 0.00003 NA < 0.00003 82% 50% 140% 93% 50% 140% 96% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 Penzo(a)pyrene 50% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 NA < 0.00009 Penzo(a)pyrene 50% Penzo(a)pyrene 5848458 < 0.00009 < 0.00009 Penzo(a)pyrene 50% Penzo(a)pyrene 5848458 < 0.00009 Penzo(a)pyrene 50% Penzo(a)p	Pyrene	5848458	•	< 0.00012	< 0.00012	NA	< 0.00012	110%	50%	140%	71%	50%	140%	85%	50%	140%
Benzo(b)fluoranthene         5848458         < 0.00003 < 0.00003	Benzo(a)anthracene	5848458		< 0.00008	< 0.00008	NA	< 0.00008	8 83%	50%	140%	71%	50%	140%	85%	50%	140%
Benzo(k)fluoranthene       5848458       < 0.00006	Chrysene	5848458		< 0.00005	< 0.00005	NA	< 0.00005	111%	50%	140%	100%	50%	140%	74%	50%	140%
Benzo(a)pyrene 5848458 < 0.001 < 0.001 NA < 0.001 100% 50% 140% 69% 50% 140% 88% 50% 140% 140% 140% 140% 140% 140% 140% 14	Benzo(b)fluoranthene	5848458		< 0.00003	< 0.00003	NA	< 0.00003	3 101%	50%	140%	99%	50%	140%	81%	50%	140%
Indeno(1,2,3-cd)pyrene 5848458 < 0.00003 < 0.00003 NA < 0.00003 82% 50% 140% 93% 50% 140% 96% 50% 140	Benzo(k)fluoranthene	5848458		< 0.00006	< 0.00006	NA	< 0.00006	111%	50%	140%	100%	50%	140%	85%	50%	140%
Dibenzo(a,h)anthracene 5848458 < 0.00009 < 0.00009 NA < 0.00009 96% 50% 140% 67% 50% 140% 85% 50% 140%	Benzo(a)pyrene	5848458		< 0.001	< 0.001	NA	< 0.001	100%	50%	140%	69%	50%	140%	88%	50%	140%
	Indeno(1,2,3-cd)pyrene	5848458		< 0.00003	< 0.00003	NA	< 0.00003	82%	50%	140%	93%	50%	140%	96%	50%	140%
Benzo(ghi)perylene 5848458 < 0.00006 < 0.00006 NA < 0.00006 98% 50% 140% 76% 50% 140% 74% 50% 14	Dibenzo(a,h)anthracene	5848458		< 0.00009	< 0.00009	NA	< 0.00009	96%	50%	140%	67%	50%	140%	85%	50%	140%
	Benzo(ghi)perylene	5848458	•	< 0.00006	< 0.00006	NA	< 0.00006	98%	50%	140%	76%	50%	140%	74%	50%	140%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

R. Chakraberty



# **Quality Assurance**

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528 ATTENTION TO: Hiva Elhami

**SAMPLED BY:Hiva** 

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

				Wate	er Ar	nalysi	is								
RPT Date: May 21, 2024				UPLICATE	•		REFEREN	ICE MA	TERIAL	METHOD BLANK SPIKE			MATRIX SPIKE		KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured		eptable mits	Recovery	Lin	ptable nits	Recovery		eptable mits
		ld	-	-			Value	Lower	Upper	,	Lower	Upper	er	Lower	Upper
Barrie Sanitary/Storm Sewer U	Jse By-law - Ir	norganics													
рН	5845142		7.63	7.69	0.8%	NA	99%	90%	110%						
Chemical Oxygen Demand	5847984		49	48	2.1%	< 5	101%	90%	110%	100%	90%	110%	87%	70%	130%
Total Suspended Solids	5847931		<10	<10	NA	< 10	100%	80%	120%						
Total Phosphorus	5852462		0.13	0.13	0.0%	< 0.02	104%	70%	130%	102%	80%	120%	NA	70%	130%
Total Kjeldahl Nitrogen	5838267		0.13	0.14	NA	< 0.10	97%	70%	130%	101%	80%	120%	89%	70%	130%
Fluoride	5844099		<0.05	<0.05	NA	< 0.05	100%	70%	130%	101%	80%	120%	98%	70%	130%
Chloride	5844099		123	123	0.0%	< 0.10	94%	70%	130%	103%	80%	120%	103%	70%	130%
Sulphate	5844099		39.0	39.1	0.3%	< 0.10	97%	70%	130%	102%	80%	120%	98%	70%	130%
Sulphide	5860288		<0.01	< 0.01	NA	< 0.01	102%	90%	110%	101%	90%	110%	101%	80%	120%
Phenols	5844490		<0.002	<0.002	NA	< 0.002	101%	90%	110%	98%	90%	110%	102%	80%	120%
Cyanide, SAD	5843428		0.009	0.008	NA	< 0.002	105%	70%	130%	91%	80%	120%	100%	70%	130%
Total Aluminum	5838284		0.153	0.145	5.4%	< 0.010	120%	70%	130%	108%	80%	120%	109%	70%	130%
Total Antimony	5838284		< 0.003	< 0.003	NA	< 0.003	100%	70%	130%	100%	80%	120%	100%	70%	130%
Total Arsenic	5838284		< 0.003	< 0.003	NA	< 0.003	98%	70%	130%	97%	80%	120%	101%	70%	130%
Total Barium	5838284		0.027	0.026	3.8%	< 0.002	100%	70%	130%	101%	80%	120%	99%	70%	130%
Total Bismuth	5838284		<0.002	<0.002	NA	< 0.002	95%	70%	130%	99%	80%	120%	94%	70%	130%
Total Cadmium	5838284		<0.0001	<0.0001	NA	< 0.0001	98%	70%	130%	102%	80%	120%	95%	70%	130%
Total Chromium	5838284		<0.003	< 0.003	NA	< 0.003	98%	70%	130%	99%	80%	120%	103%	70%	130%
Total Cobalt	5838284		<0.0005	< 0.0005	NA	< 0.0005	106%	70%	130%	101%	80%	120%	105%	70%	130%
Total Copper	5838284		<0.002	<0.002	NA	< 0.002	99%	70%	130%	99%	80%	120%	99%	70%	130%
Total Iron	5838284		0.186	0.196	NA	< 0.050	106%	70%	130%	102%	80%	120%	106%	70%	130%
Total Lead	5838284		<0.0005	<0.0005	NA	< 0.0005	93%	70%	130%	96%	80%	120%	93%	70%	130%
Total Manganese	5838284		0.094	0.111	16.6%	< 0.002	107%	70%	130%	94%	80%	120%	114%	70%	130%
Total Mercury	5846273		<0.0002	< 0.0002	NA	< 0.0002	103%	70%	130%	104%	80%	120%	97%	70%	130%
Total Molybdenum	5838284		<0.002	0.003	NA	< 0.002	100%	70%	130%	106%	80%	120%	113%	70%	130%
Total Nickel	5838284		0.003	0.003	NA	< 0.003	105%	70%	130%	98%	80%	120%	102%	70%	130%
Total Selenium	5838284		<0.002	< 0.002	NA	< 0.002	102%	70%	130%	97%	80%	120%	94%	70%	130%
Total Silver	5838284		<0.0001	<0.0001	NA	< 0.0001	104%	70%	130%	101%	80%	120%	107%	70%	130%
Total Tin	5838284		<0.002	< 0.002	NA	< 0.002	100%	70%	130%	96%	80%	120%	98%	70%	130%
Total Vanadium	5838284		<0.002	<0.002	NA	< 0.002	110%	70%	130%	102%	80%	120%	103%	70%	130%
Total Zinc	5838284		<0.020	<0.020	NA	< 0.020	100%	70%	130%	100%	80%	120%	101%	70%	130%

Comments: NA signifies Not Applicable.

Duplicate NA: results are under 5X the RDL and will not be calculated.

Matrix spike NA: Spike level < native concentration. Matrix spike acceptance limits do not apply and are not calculated.

BOD5

Biochemical Oxygen Demand, Total 5850501 6 6 NA < 2 105% 70% 130%

Comments: If RPD value is NA, the results of the duplicates are less than 5x the RDL and the RPD will not be calculated.

# AGAT QUALITY ASSURANCE REPORT (V1)

Page 9 of 15



# **Quality Assurance**

CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528 ATTENTION TO: Hiva Elhami

SAMPLED BY:Hiva

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

Water Analysis (Continued)															
RPT Date: May 21, 2024				UPLICAT	E		REFEREN	ICE MATE	ERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Blank Measured Limits				Acceptable Limits		Recovery	Lin	ptable nits
		ld					Value	Lower U	Jpper	,	Lower	Upper		Lower	Upper

Certified By:

Inis Verástegui

# **Method Summary**

CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD

PROJECT: SP22-01117-00
SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

AGAT WORK ORDER: 24T148528 ATTENTION TO: Hiva Elhami SAMPLED BY:Hiva

PARAMETER	AGAT S.O.P	ANALYTICAL TECHNIQUE			
Trace Organics Analysis					
Oil and Grease (animal/vegetable) in water	VOL-91-5011	EPA SW-846 3510C & SM 5520	BALANCE		
Oil and Grease (mineral) in water	VOL-91-5011	EPA SW-846 3510C & SM 5520	BALANCE		
Dichloromethane	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Benzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Trichloroethylene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Toluene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Tetrachloroethene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Ethylbenzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
1,1,2,2-Tetrachloroethane	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
1,4-Dichlorobenzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
1,2-Dichlorobenzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
Xylenes (Total)	VOL-91-5001	modified from EPA 5030B & EPA 8260D	CALCULATION		
Hexachlorobenzene	ORG-91-5112	EPA SW846 3510C & 8081B	GC/ECD		
Toluene-d8	VOL-91- 5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
4-Bromofluorobenzene	VOL-91- 5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS		
TCMX	ORG-91-5112	modified from EPA SW-846 3510C & 8081B	GC/ECD		
Naphthalene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Acenaphthylene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Acenaphthene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Fluorene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Phenanthrene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Anthracene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Fluoranthene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Pyrene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Benzo(a)anthracene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Chrysene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Benzo(b)fluoranthene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Benzo(k)fluoranthene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		
Benzo(a)pyrene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS		



# **Method Summary**

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528
ATTENTION TO: Hiva Elhami
SAMPLED BY:Hiva

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Indeno(1,2,3-cd)pyrene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS
Dibenzo(a,h)anthracene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS
Benzo(ghi)perylene	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS
Total PAHs	ORG-91-5114	modified from EPA 3510C and EPA 8270E	CALCULATION
2-Fluorophenol	ORG-91-5114	modified from EPA 3510C, 8270E & ON MOECC E3265	GC/MS
phenol-d6 surrogate	ORG-91-5114	modified from EPA 3510C, 8270E & ON MOECC E3265	GC/MS
2,4,6-Tribromophenol	ORG-91-5114	modified from EPA 3510C, 8270E & ON MOECC E3265	GC/MS
Chrysene-d12	ORG-91-5114	modified from EPA 3510C and EPA 8270E	GC/MS

# **Method Summary**

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528 ATTENTION TO: Hiva Elhami SAMPLED BY:Hiva

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Water Analysis	•		
Biochemical Oxygen Demand, Total	INOR-121-6023	SM 5210 B	INCUBATOR
рН	INOR-93-6000	modified from SM 4500-H+ B	PC TITRATE
Chemical Oxygen Demand	INOR-93-6042	modified from SM 5220 A and SM 5220 D	SPECTROPHOTOMETER
Total Suspended Solids	INOR-93-6028	modified from EPA 1684,ON MOECC E3139,SM 2540C,D	BALANCE
Total Phosphorus	INOR-93-6022	modified from SM 4500-P B and SM 4500-P E	SPECTROPHOTOMETER
Total Kjeldahl Nitrogen	INOR-93-6048	modified from EPA 351.2 and SM 4500-NORG D	LACHAT FIA
Fluoride	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Chloride	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Sulphate	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Sulphide	INOR-93-6054	modified from SM 4500 S2- D	SPECTROPHOTOMETER
Phenols	INOR-93-6072	modified from SM 5530 D	LACHAT FIA
Cyanide, SAD	INOR-93-6051	modified from MOECC E3015; SM 4500-CN- A, B, & C	SEGMENTED FLOW ANALYSIS
Total Aluminum	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Antimony	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Arsenic	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Barium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Bismuth	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cadmium	MET -93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Chromium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cobalt	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Copper	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Iron	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Lead	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Manganese	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Mercury	MET-93-6100	modified from EPA 245.2 and SM 3112 B	<sup>2</sup> CVAAS
Total Molybdenum	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Nickel	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Selenium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Silver	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Tin	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS



# **Method Summary**

**CLIENT NAME: SIRATI & PARTNERS CONSULTANTS LTD** 

PROJECT: SP22-01117-00

AGAT WORK ORDER: 24T148528 ATTENTION TO: Hiva Elhami SAMPLED BY:Hiva

SAMPLING SITE:334 and 340 Ardagh Rd. Barrie

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Total Vanadium	ME 1-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Zinc	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS

# AGAT Laboratories

**Chain of Custody Record** 

Report Information:

**Project Information:** 

Company: Contact:

Address:

Phone:

1. Email:

2 Email

Project:

( 1 Yellow Copy - AGAT

Site Location:

Reports to be sent to:

Have feedback? Scan here for a quick survey!



**Regulatory Requirements:** 

Is this submission for a Record

of Site Condition (RSC)?

☐ No

Regulation 153/04 Regulation 406

□Ind/Com

☐Res/Park

Agriculture

Regulation 558

CCME

If this is a Drinking Water sample, please use Drinking Water Chain of Custody Form (potable water consumed by humans)

(Please check all applicable boxes)

Table \_\_\_\_\_\_indicate One

☐Ind/Com

☐Res/Park

Agriculture

Coarse

☐ Yes

Soil Texture (Check One)

5835 Coopers Avenue Mississauga, Ontario L4Z 1Y2 Ph: 905.712.5100 Fax: 905.712,5122 webearth agallabs.com

Prov. Water Quality

**Report Guideline on** 

**Certificate of Analysis** 

□ No

Other

☐ Yes

Objectives (PWOO)

Laboratory	Use Only		
Work Order #:	241	1485	28

Cooler Quantity:	21		
Arrival Temperatures:	3.9	14-7	4.9
Depot Temperatures:	6.3	15-9	4-8
Custody Seal Intact:	□Yes	□No	DIMA

Turnaround Tin	ne (TAT) Required:	
Regular TAT	5 to 7 Business Days	

Rush TAT (Rush Surcharges Apply)

	3 Business Days	1-1	2 Business Days	_	Next Business
ш	Days	1	Days		Next Business Day

Please provide prior notification for rush TAT
\*TAT is exclusive of weekends and statutory holidays

For 'Same Day' analysis, please contact your AGAT CSR

Sampled By:							
AGAT Quote #: P0:		_ Legal Sample □	0.	Reg 153	0. Reg 406	0, Reg 558	2
Please note: If quotation number is not provided, client will	l be billed full price for analysis	Legal Sample	orvi, b		kage □ oc	". 88	λ
Invoice Information:  Company: Contact:	Bill To Same: Yes-M No □	GW Ground Water SD Sediment	Metals, Hg, Cr	HWSB	Bracterization Package 11-F4  F Rainwater Leach  Vocs □ Svocs □ oc	aracterization TCD:  DABNS D Bajp D POBS  Sture D Sulphide  SULP LAC	eh Concentrati
Address:		O Oil SW Surface Water P Paint R Rock/Shele	2 2	H B	1 F4	aracteriz	P P P
Email: Hivand Stratica N	ick@ Svoti co	S Soil	eld Filten Inorgan	□ crvi, □	406 SPLEX, FI	Dvocal Charles	ndous
Firuze@ Siratio		I to the second	Field & Inc		on 406 on 406 on 406	IIII Dispo	v Hb2z
Sample Identification Date Sampled	Time # of Containers	Sample Comments/ Matrix Special Instructions	Metals N/A	Metals BTEX, F VOC VOC	Regulation 406 Chapter, Pec. SAR Regulation 406 SPL mSPLP   Metals	Corrosivity:	Potential
1 BH/MW_OI May 07,22	11:40 9 21	GW	N				
2. BH/MW-02A "	12:48 21	N	N				
	13:50 4 21	v	N				
4. BH(MW-04 ~	15:10 # 21	"	N				
5.	AM PM						
6.	AM PM						
7.	AM PM						
8.	AM PM						
9.	- AM PM				186		
10.	AM PM						
11,	AM	24			THE TOTAL		
Samples Relinquished By (Print Name and Sign):  History Ellips in American Sign):  "adulished by (Print Name and Sign):	May of, 2024 16	Samples Received by (Print Name and Sign); Samples Received By (Print Name and Sign);		Date	y8 3p	Page	of

uished Br (Print Name and Sistn):

Samples Received By (Print Name and Sign)

# APPENDIX E



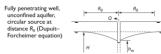
#### Short Term Dewatering Calculations for Block 1 - 340 Ardagh Rd.

Groundwater Dewatering Calculation (Q1)

	B - Assumed	C - Target	D - Assumed
A- Initial Water	Aquifer	Water Level	Bottom of
<b>Level Elevation</b>	Bottom	Elevation	Dewatering
(m)	Elevation (m)	(m)	Well (m)
242.3		241.1	240.1

Partially Penetrating Well Method

									Zone of	Zone of		
									Influence from	Influence		
									Center of	from Edge of	No safety	safety factor
						Eq. 7.1	Eq. 7.2	Average	Excavation	Excavation	factor	of 3.0
H-s = A-D (m)	t =C-D (m)	s=D-B (m)	k (m/s)	a (m)	b (m)	re (m)	re (m)	re (m)	Ro (m)	R	Q1 (L/day)	Q1 (L/day)
2.2	1		1.66E-05	35	14	6.3	12.5	9.4	24	15	18,378	55,133



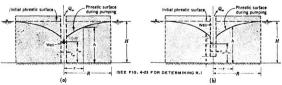
k = soil permeability;

H = initial water table level in aquifer;

 $h_{\rm w}$  = lowered water level in equivalent well;

 $r_{\rm e}$  = equivalent radius of well;

 $R_0$  = radius of influence.



#### FULLY PENETRATING WELL

FLOW,  $Q_w$  , OR DRAWDOWN,  $H^2\sim h^2$ , NEGLECTING HEIGHT OF FREE DISCHARGE,  $h^1$  (CONDITION (d)).

$$Q_{w} = \frac{\pi k(H^2 - h^2)}{\ln (R/r)}$$

 $Q_{w} = \frac{\pi k \left(H^{2} - h_{w}^{2}\right)}{\ln \left(R/r_{w}\right)}$ 

" | h(R/r) |

FLOW, Q., TAKING h' INTO ACCOUNT (b) CAN BE ESTIMATED ACCURATELY FROM EQ 2 USING HEIGHT OF WATER, 1+5(5=0 FOR FULLY PENETRATING WELL), FOR THE TERM h...

#### FULLY OR PARTIALLY PENETRATING WELL

FLOW, Q , FOR ANY GRAVITY WELL WITH A CIRCULAR SOURCE

$$Q_{w} = \frac{\pi k \left[ (H - s)^{2} - t^{2} \right]}{\ln (R/r_{w})} \left[ 1 + \left( 0.30 + \frac{10r_{w}}{H} \right) \sin \frac{1.8s}{H} \right]$$





Figure 7.5 Equivalent radius of arrays of wells. (a) Circular system of radius  $r_{\rm e}$ . (b) Rectangular system.

plan dimensions a by b, the equivalent radius can be estimated by assuming a well of equal perimeter

$$r_{c} = \frac{\left(a + b\right)}{\pi} \tag{7.1}$$

or equal area

$$r_e = \sqrt{\frac{ab}{\pi}}$$
(7.2)

#### 2) Stormwater runoff as per 20 mm per day

Precipitation	Site Area	Q2
m/day	m2	L/day
0.02	490	9800

Total Dewatering Volume Q = Q1 + Q2

Q=

**64,933** L/day

RADIUS OF INFLUENCE, R, CAN BE ESTIMATED FOR BOTH ARTESIAN AND GRAVITY FLOWS BY

R = C (H - h<sub>w</sub>) 
$$\sqrt{k}$$

WHERE R, H, AND h, ARE DEFINED PREVIOUSLY AND EXPRESSED IN FEET. COEFFICIENT OF PERMEABILITY, k, IS EXPRESSED IN 10<sup>-4</sup> CM/SEC.

AND C = 3 FOR ARTESIAN AND GRAVITY FLOWS TO A WELL.

C = 1.5 TO 2.0 FOR A SINGLE LINE OF WELLPOINTS.

# Long Term Dewatering Calculations - for Block 1- 340 Ardagh Rd. Scenario 1

Initial water level (mAMSL) =	242.3
Average sub-drainage level (mAMSL) =	242.2
Height of wall drain or drawdown (m)=	0.1
Underground level area (m2) =	490

$$Q' = \frac{K(h_0^2 - h_L^2)}{2L}$$

k	1.66E-05 m/s
R=3000*h*SQRT(K)	1.2 m
h0	1.1 m
hL	1 m
L = R/2	0.6 m
Q (Flux)	2.85E-06 m2/s
Equivalent Perimeter Length	88.5 m

Equivalent i crimeter Length	00.5 111
Q (Subdrain)	21,825 L/d
Q (Sub Drain)	0.3 L/s
Saftey Factor	1.5
Q (Subdrain)	32,738 L/d
Q (Sub Drain)	0.4 L/s

# APPENDIX F



# **DETAILED WATER BALANCE CALCULATIONS**

334 Ardagh Road, Barrie, ON

## 1 Climate Information

Precipitation	999 mm/a
Actual Evapotranspiration	531 mm/a
Water Surplus	468 mm/a

### 2 Infiltration Rates

## **Table 2 Approach - Infiltration factors**

Topography: Rolling land	0.25
Soil Type: Glacial Till (Predominantly sand and gravel)	0.4
Cover: Cultivated Land	0.1
Total	0.75

Infiltration (0.75 x 387) 351 mm/a Run-off (468-351) 117 mm/a

# **Table 3 Approach - Typical Recharge Rates**

Coarse Sand and Gravel	>250	mm/a
Fine to medium sand	200-250	mm/a
Silty sand to sandy silt	150-200	mm/a
Silt	125-150	mm/a
Clayey Silt	100- 125	mm/a
Clay	<100	mm/a
Site development area is underlain predominantly by SILTY	SAND to SA	NDY SILT

soils

Based on the above, the recharge rate is typically 150-200 mm/a

3 Pre-Development Property Statistics	ha	m2
Paved Area	0.002875	28.75
Roof Area	0.028488	284.88
Landscape Area	0.118543	1,185
Total	0.149906	1,499

4 Post-Development Property Statistics	ha	m2
Paved Area	0.020197	202
Roof Area	0.070323	703
Landscape Area	0.059383	594
Total Land Area	0.149903	1,499

# 5. Annual Pre-Development Water Balance

La	nd Use	Area (m²)	Precipitation (m³)	Evapotranspiration (m3)	Infiltration (m³)	Run-off (m³)
	Paved Area	28.75	29	3	0	26
Impervious Areas	Building/Roof Area	284.88	285	28	0	256
Pervious Areas	Landscape Area	1,185	1,184	629	416	139
		1,499	1,498	661	416	421

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

### 6. Annual Post-Development Water Balance

La	nd Use	Area (m²)	Precipitation (m³)	Evapotranspiration (m3)	Infiltration (m³)	Run-off (m³)
	Paved Area	202	202	20	0	182
Impervious Areas	Building/Roof Area	703	703	70	0	632
Pervious Areas	Landscape Area	594	593	315	208	69
		1,499	1,498	406	208	883

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and general roof areas.

## 7. Comparision of Pre- and Post -Development

<u>.</u>	Precipitation (m³)	Evapotranspiration (m3)	Infiltration (m³)	Run-off (m³)
Pre-Development	1,498	661	416	421
Post-Development	1,498	406	208	883
Change in Volume		-255	-208	463
Change in %			-50	110

# 8. Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration	661
Volume of Post-Development Infiltration	208
Deficit from Pre to Post Development Infiltration	452
Percentage of Roof Runoff required to match the pre-development infiltration	72

# **DETAILED WATER BALANCE CALCULATIONS**

340 Ardagh Road, Barrie, ON

## 1 Climate Information

Precipitation	999 mm/a
Actual Evapotranspiration	531 mm/a
Water Surplus	468 mm/a

### 2 Infiltration Rates

## **Table 2 Approach - Infiltration factors**

Topography: Rolling land	0.25
Soil Type: Glacial Till (Predominantly sand and gravel)	0.4
Cover: Cultivated Land	0.1
Total	0.75

Infiltration (0.75 x 468) 351 mm/a Run-off (468-351) 117 mm/a

# Table 3 Approach - Typical Recharge Rates

Coarse Sand and Gravel	>250	mm/a
Fine to medium sand	200-250	mm/a
Silty sand to sandy silt	150-200	mm/a
Silt	125-150	mm/a
Clayey Silt	100- 125	mm/a
Clay	<100	mm/a
Site development area is underlain by silt and clavey silt m	naterial with	

some sand and gravel

Based on the above, the recharge rate is typically 150-200 mm/a

3 Pre-Development Property Statistics	ha	m2
Paved Area	0.0157	157
Roof Area	0.0223	223
Landscape Area	0.111832	1,118
Total	0.149832	1,498

4 Post-Development Property Statistics	ha	m2
Paved Area	0.020133	201
Roof Area	0.070323	703
Landscape Area	0.059376	594
Total Land Area	0.149832	1,498

# 5. Annual Pre-Development Water Balance

Land Use		Area (m²)	Precipitation (m³)	Evapotranspiration (m3)	Infiltration (m³)	Run-off (m³)
	Paved Area	157	157	16	0	141
Impervious Areas	Building/Roof Area	223	223	22	0	200
Pervious Areas	Landscape Area	1,118	1,117	594	393	131
		1,498	1,497	632	393	473

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

### 6. Annual Post-Development Water Balance

La	Land Use		Area (m²) Precipitation (m³) Evapotranspiration (m3)		Infiltration (m³)	Run-off (m³)
	Paved Area	201	201	20	0	181
Impervious Areas	Building/Roof Area	703	703	70	0	632
Pervious Areas	Landscape Area	594	593	315	208	69
		1,498	1,497	406	208	883

Assuming no infiltration occurring in paved and roof areas, 10% of precipitation to be evaporated from paved and general roof areas.

## 7. Comparision of Pre- and Post -Development

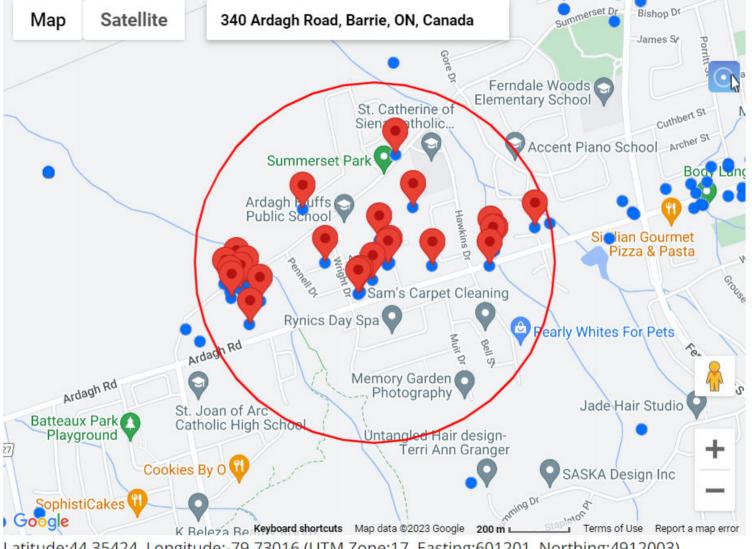
	Precipitation (m³)	Evapotranspiration (m3)	Infiltration (m³)	Run-off (m³)
Pre-Development	1,497	632	393	473
Post-Development	1,497	406	208	883
Change in Volume		-226	-184	410
Change in %			-47	87

# 8. Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration	632
Volume of Post-Development Infiltration	208
Deficit from Pre to Post Development Infiltration	423
Percentage of Roof Runoff required to match the pre-development infiltration	67

# APPENDIX G





Latitude:44.35424, Longitude:-79.73016 (UTM Zone:17, Easting:601201, Northing:4912003)

Well ID	Well Record Information	Well Tag # (since 2003) \$	Audit# \$	Contractor Lic# \$	Well Depth (m) \$
5701514	PDF HTML	N/A	N/A	4715	18.3
5701688	PDF HTML	N/A	N/A	4753	9.1
5709611	PDF HTML	N/A	N/A	4608	7.0
5710470	PDF HTML	N/A	N/A	4608	7.6
5719410	PDF HTML	N/A	N/A	1467	27.1
5719579	PDF HTML	N/A	N/A	1467	26.8
5728987	PDF HTML	N/A	111289	3602	25.9
5731300	PDF HTML	N/A	148244	3602	21.9
5734834	PDF HTML	N/A	156610	2801	94.5
5738948	PDF HTML	A010794	Z11826	6607	3.0
7149889	PDF HTML	A097054	Z111899	7219	N/A
7150340	PDF HTML	N/A	Z103426	2513	N/A
7303756	PDF HTML	A217625	Z274175	7241	4.9

7303756	PDF HTML	A217625	Z274175	7241	4.9	
7303914	PDF HTML	A217647	Z274174	7241	4.6	
7333788	HTML	A139489	Z265166	7314	2.3	
7384091	PDF HTML	A312273	Z354319	7744	N/A	
7384093	PDF HTML	A312275	Z354338	7744	N/A	
7384094	PDF HTML	A312272	Z354322	7744	N/A	
7401741	PDF HTML	A240942	EO8O5MTA	7732	N/A	
7401767	PDF HTML	_NO_TAG	X99N6RXV	7732	N/A	
7401770	PDF HTML	_NO_TAG	IESUHAK5	7732	N/A	
7401773	PDF HTML	_NO_TAG	ON9TI9CG	7732	N/A	
7420205	PDF HTML	A352844	IPBDBXI4	7360	7.6	
7420206	PDF HTML	A352846	OERUEGBN	7360	10.7	
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Showing 1 to	Showing 1 to 24 of 24 entries First Previous 1 Next Last					

# APPENDIX H



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Professional judgement was exercised in gathering and analyzing data and formulation of recommendations using current industry guidelines and standards. Similar to all professional persons rendering advice, SIRATI cannot act as absolute insurer of the conclusion we have reached. No additional warranty or representation, expressed or implied, is included or intended in this report other than stated herein the report.

The assessment should not be considered a comprehensive audit that eliminates all risks of encountering environmental problems. The information presented herein this report is primarily based on information collected during the hydrogeological study based on the condition of the Property at the time of site inspection/drilling followed by a review of historical data, as appended to this report.

In assessing the environmental setting of the Property, SIRATI has solely relied upon information supplied by others in good faith and has therefore assumed that the information supplied is factual and accurate. We accept no responsibility for any inaccurate information, misrepresentation or for any deficiency of the information supplied by any third party.

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In the event that during future work new information regarding the environmental/hydrogeological condition of the Property is encountered, or in the event that the outstanding responses from the regulatory agencies indicate outstanding issues on file with respect to the Property, SIRATI should be notified in order that we may re-evaluate the findings of this assessment and provide amendments, as required.