

158 Ardagh Road

City of Barrie

Traffic Brief for The Hedbern Development Corp.

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1 Introduction

1.1 Background

The Hedbern Development Corp. [The Developer] is proposing a residential development [subject site], municipally known as 158, 162, 166 & 170 Ardagh Road, located north of Ardagh Road, across from Snowshoe Trail, in the City of Barrie [City]. The subject site includes 58 residential townhouse units.

The proposed development will include a single full-movement access driveway [Site Access] onto Ardagh Road, opposite of Snowshoe Trail. The proposed development also provides access via an extension of Bishop Drive. Of the proposed 58 townhouse units, 27 units will have direct driveway access onto the extension of Bishop Drive and 31 units will utilize the Site Access.

The Developer has retained **JD Northcote Engineering Inc.** [JD Engineering] to prepare this traffic brief in support of the proposed residential development.

1.2 Study Area

Figure 1 illustrates the location of the subject site and study area intersections in relation to the surrounding area. The Conceptual Site Plan by IPS Consulting Inc. is attached in **Appendix A**.

The subject site is bound by Ardagh Road to the south and existing residential development to the west, north and east.

Through consultation with the City, the following intersections are included in the Traffic Brief:

- Ardagh Road / Snowshoe Trail & Site Access.

Figure 1 – Proposed Site Location and Study Area



1.3 Study Scope and Objectives

The purpose of this study is to identify the potential impacts to traffic flow at the site access and on the surrounding roadway network. The study analysis includes the following tasks:

- Consult with the City to address any traffic-related issues or concerns they have with the proposed development;
- Determine existing traffic volumes and circulation patterns;
- Estimate future traffic volumes if the proposed development was not constructed, including the impact of additional proposed developments in the area;
- Estimate the amount of traffic that would be generated by the proposed development and assign to the roadway network;
- Complete level-of-service [LOS] analysis of horizon year (with the proposed development) traffic conditions and identify additional operational deficiencies;
- Identify improvement options to address operational deficiencies;
- Calculate lane improvements for the site access driveway based on the Transportation Association of Canada and Ontario Ministry of Transportation guidelines;
- Review the available sight distance at the site access driveway;

- Review and comment on the construction staging plan as it relates to parking of trades people, delivery of construction material, maintenance of adjacent property access, pedestrian movements, City infrastructure, etc.; and
- Document findings and recommendations in a final report.

1.4 Horizon Year and Analysis Periods

The traffic scenario for the City's planning horizon year (2031) was selected for analysis of traffic operations in the study area. The weekday morning [AM] and weekday afternoon [PM] peak hours have been selected as the analysis periods for this study.

2 Information Gathering

2.1 Street and Intersection Characteristics

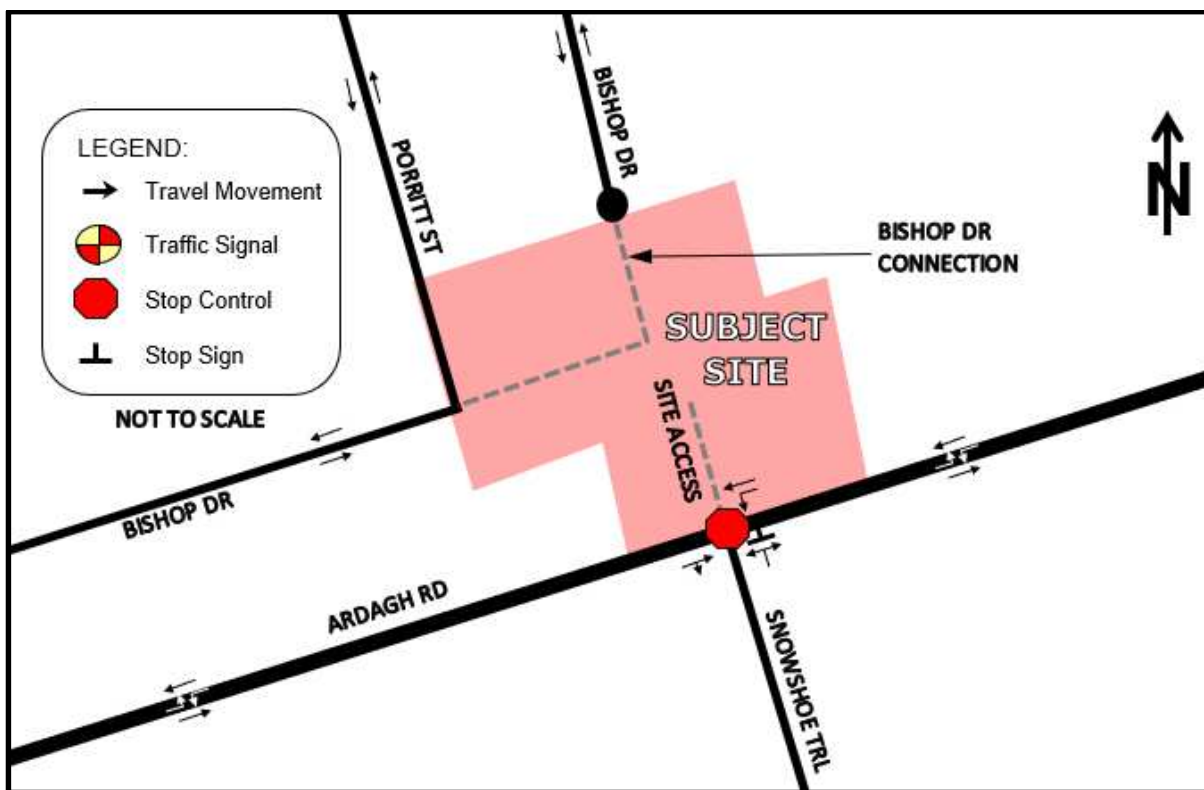
Ardagh Road is a three-lane major arterial road with an urban cross section. Ardagh Road has a bicycle lane and a sidewalk on both sides of the road. Ardagh Road has a posted speed limit of 50km/h and is under the jurisdiction of the City.

Snowshoe Trail is a two-lane local road with an urban cross section. Snowshoe Trail has a sidewalk on the east side of the road. Snowshoe Trail has an assumed (unposted) speed limit of 50km/h and is under the jurisdiction of the City.

Bishop Drive is a two-lane local road with an urban cross section. Bishop Drive has a sidewalk on the north side or east side of the road. Bishop Drive has an assumed (unposted) speed limit of 50km/h and is under the jurisdiction of the City.

The existing intersection spacing and lane configuration within the study area is illustrated in **Figure 2**.

Figure 2 – Existing (2019) Lane Configuration with in Study Area



2.2 Local Transportation Infrastructure Improvements

According to the City of Barrie Multi-Modal Active Transportation Master Plan [MMATMP], the following road improvements were anticipated to be completed prior to 2016, but have not been completed at the time of this study:

- Ardagh Road (within study area)
 - Addition of one lane in both directions

It is noted that the City of Barrie Multi-Modal Active Transportation Master Plan [MMATMP] was completed in January 2014 and at that time Ardagh Road was painted to provide a two-lane cross section. Following the completion of the MMATMP, Ardagh Road was reconfigured to provide a three lane cross-section with painted bike lanes. Since the City's Capital Budget does not identify any road improvements on Ardagh Road, we have assumed that there are no further planned road improvements anticipated on Ardagh Road, for the purpose of the report.

2.3 Transit Access

Barrie Transit provides one bus route within the study area. The No. 7 route provide service between downtown Barrie and the south of City travelling along Ardagh Road within the study area.

The No. 7 route operates between 05:25 – 24:12 on weekdays with service every half hour, between 6:55 – 24:12 on Saturdays with service every half hour and 09:25 – 22:12 on Sundays with service

every hour. The closest bus stop to the subject site for the No. 7 route is located just west of the Ardagh Road / Snowshoe Trail intersection.

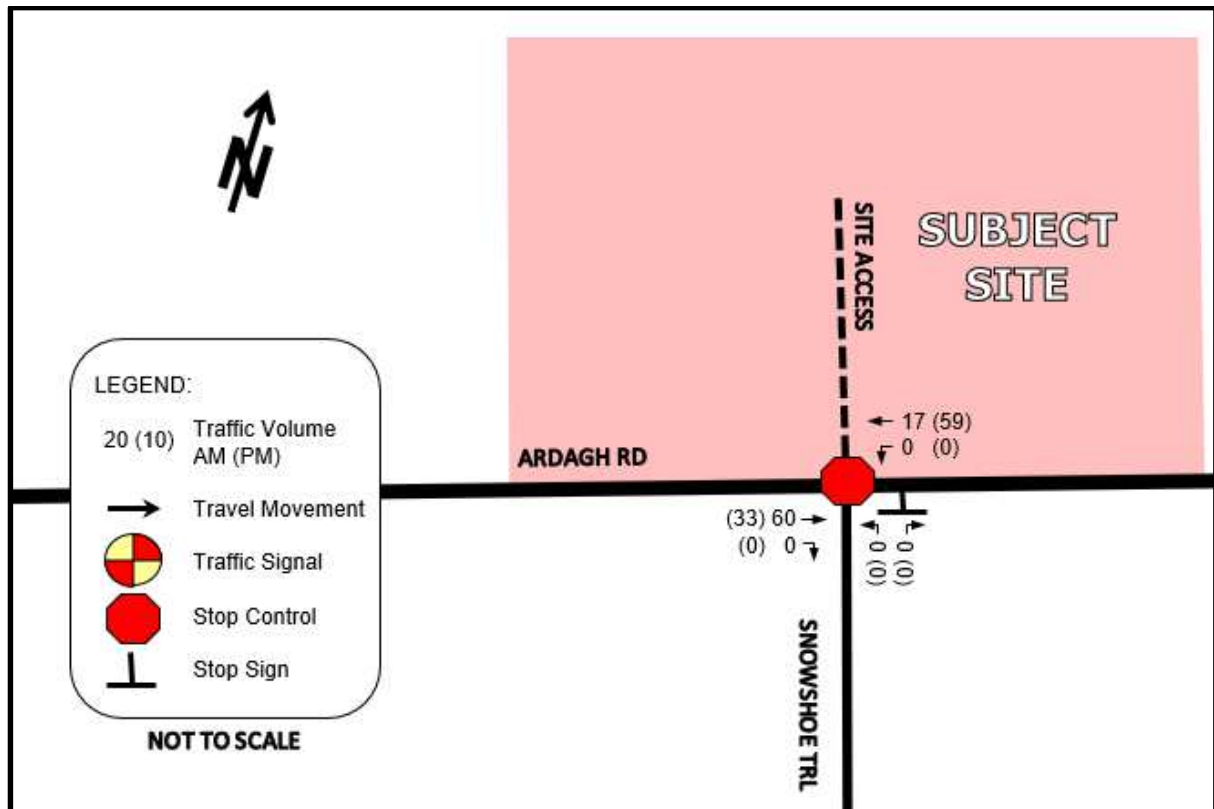
2.4 Other Developments within the Study Area

Based on review of the proposed developments currently in the City's development review process, as identified on the City's website, the Meadows of Bear Creek development is the only site in the study area that will have a notable impact on the local traffic volumes / infrastructure capacity.

The Meadows of Bear Creek development is located north of Ardagh Road between Wright Drive and Mapleton Avenue. This development will include the construction of 78-single detached residential units and a 4.5 hectare medium-density residential block. The specifics of the medium density block are not known at this time; however, it is anticipated that it will include approximately 238 townhouse or walk-up apartment units per the October 2016 traffic impact study completed by JD Engineering [Meadows of Bear Creek TIS]. It has been assumed that this development will be fully occupied prior to the 2031 horizon year.

Traffic generated by the above noted development has been estimated based on the Meadows of Bear Creek TIS (excerpts provided in **Appendix B**). **Figure 3** illustrates the above noted development traffic during within the study area

Figure 3 – Meadows of Bear Creek Traffic Volumes with in Study Area



2.5 Background Traffic Growth

Based on a comparison of the 2011 and 2031 traffic volumes provided in the EMME travel demand model used in the MMATMP, the anticipated background traffic growth on Ardagh Road, in the study area, will be negligible. In order to be conservative, a 2% per annum background traffic growth rate has been applied to the traffic volumes within the study area.

2.6 Traffic Counts

Detailed traffic and pedestrian counts were completed at the Ardagh Road / Snowshoe Trail intersection. **Table 1** summarizes the traffic count data collection information.

Table 1 – Traffic Count Data

Location	Count Date	AM Peak Hour	PM Peak Hour	Completed By
Ardagh Road / Snowshoe Trail	Thursday, February 21 st , 2019	07:30 - 08:30	16:15 - 17:15	JD Eng.*

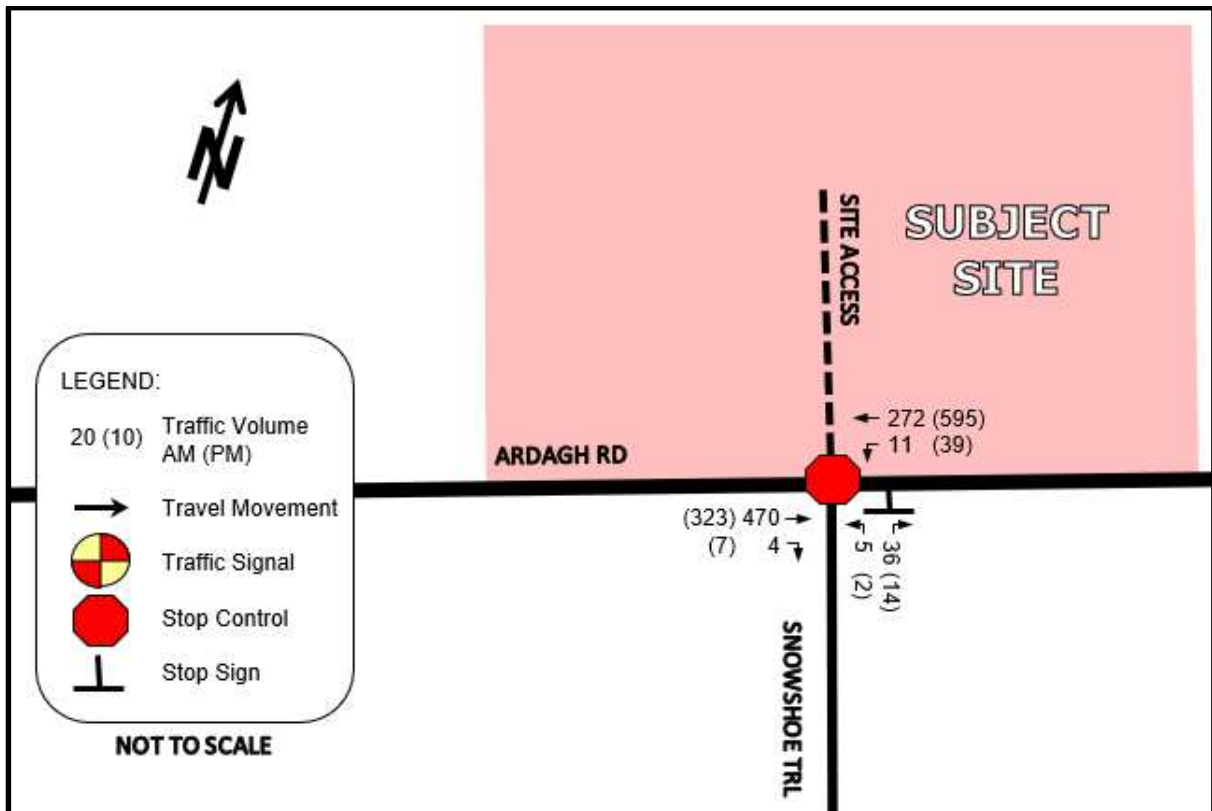
* Traffic counts were completed by Ontario Traffic Inc. on behalf of JD Engineering

Detailed traffic count data can be found in **Appendix C**. The peak hours of traffic generation for the Ardagh Road / Snowshoe Trail intersection generally aligned with the anticipated peak hour of traffic generation by the proposed development.

Heavy vehicle percentages from the traffic count data have also been included in the Synchro analysis.

Figure 4 illustrates the existing (2019) AM and PM peak hour traffic volumes within the study area.

Figure 4 – Existing (2019) Traffic Volumes

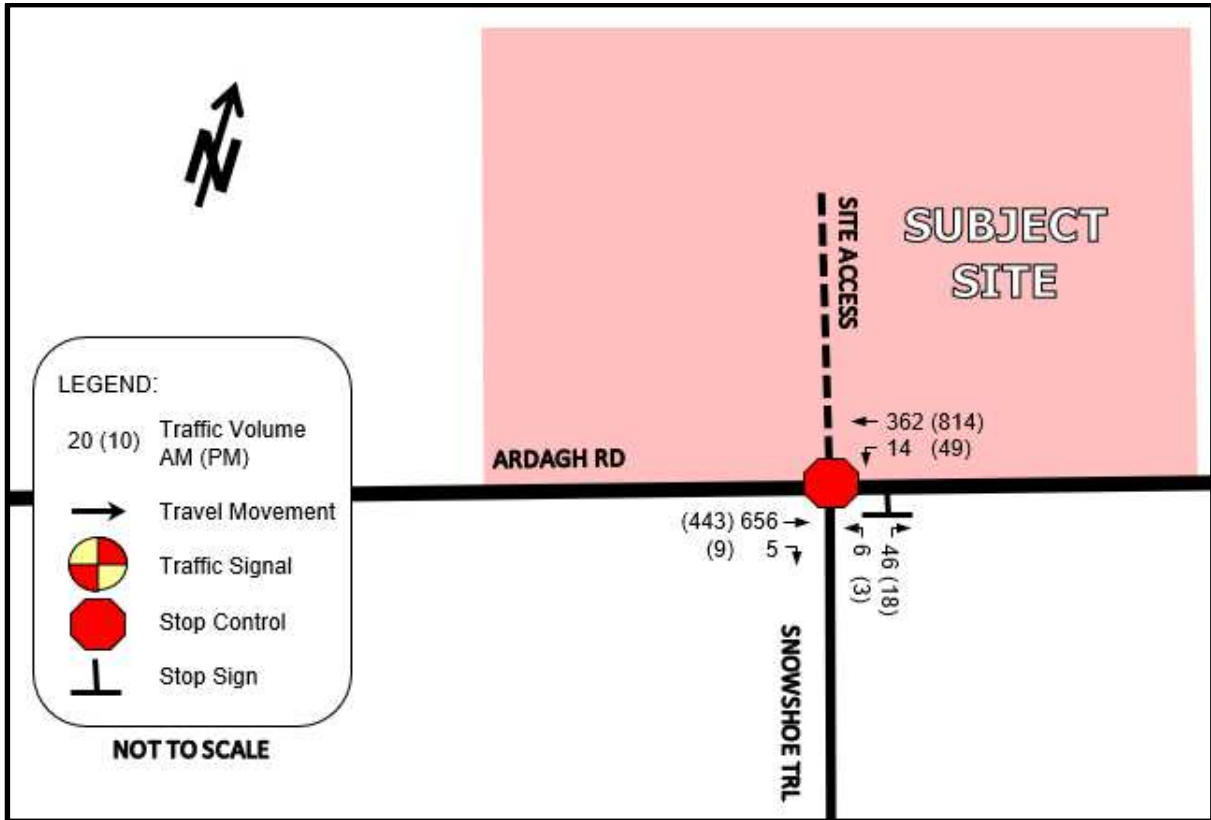


2.7 Horizon Year Traffic Volumes

In addition to the adjacent development traffic volumes outlined in Section 2.4, the background traffic growth rate discussed in Section 2.5 has also been applied to the existing (2019) traffic volumes to estimate the background (2031) horizon year traffic volumes.

Figure 5 illustrates the background (2031) horizon year AM and PM peak hour traffic volumes in the study area.

Figure 5 – Background (2031) Traffic Volumes



3 Proposed Development Traffic Generation and Assignment

3.1 Traffic Generation

The traffic generation for the proposed development has been based on the Institute of Transportation Engineers [ITE] *Trip Generation Manual* (10th Edition) [ITE Trip Generation Manual]. The following ITE land uses have been applied to estimate the traffic from the proposed development:

- ITE land use 220 (Multifamily Housing (Low-Rise)) – General Urban / Suburban Setting

The estimated trip generation of the proposed development is illustrated below in **Table 2**. The AM and PM peak traffic generation for the proposed development is not expected to exactly align with the AM and PM peak hour in the traffic counts; consequently, we have applied the peak hour of adjacent street traffic values provided in the ITE Trip Generation Manual.

Table 2 – Estimated Traffic Generation of Proposed Development

Land Use	Access Location	Size	AM Peak Hour			PM Peak Hour		
			IN	OUT	TOTAL	IN	OUT	TOTAL
Multifamily Housing (Low-Rise) ITE Land Use: 220	Site Access	31 units	3	11	14	11	6	17
	Bishop Drive	27 units	3	10	13	9	6	15
TOTAL TRIP GENERATION		58 units	6	21	27	20	12	32

In order to be conservative, no transportation modal split has been applied to the above-noted traffic generation calculation.

3.2 Traffic Assignment

For the purposes of this study, it has been assumed that all traffic generated by the proposed development will be new traffic and would not be in the study area if the development was not constructed.

The ITE data provides the anticipated percentage of new traffic entering and exiting during the peak hour. The distribution of residential traffic has been calculated based on the 2016 Transportation Tomorrow Survey [TTS] data for traffic zone 8522, retrieved using the TTS Internet Data Retrieval System [IDRS] (output attached as **Appendix D**). TTS data provides historical origin and destination work trip percentages for specific areas within the City and southern Ontario.

Traffic distribution for the trips generated by the residential component of the proposed development during the AM and PM peak hour is expected to generally follow commuter travel patterns. Our analysis is based on egress traffic during the AM peak hour. Logically, the distribution of ingress traffic will follow the inverse of the exiting traffic distribution. For each of the individual areas identified in the TTS data, we have selected the probable route of travel, assuming that people will select their route primarily based on travel time.

The distribution of trips is illustrated in **Table 3** using the methodology outlined above.

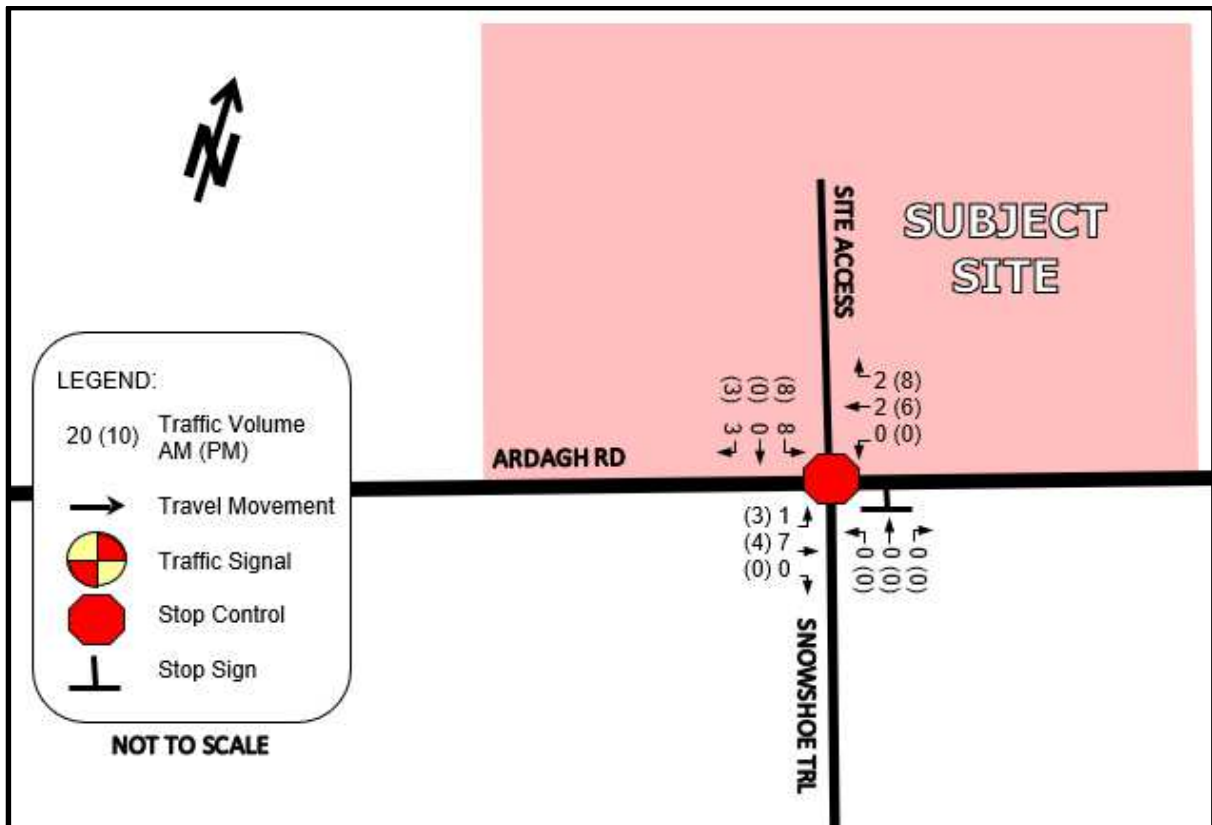
Table 3 – Proposed Development Traffic Distribution

Travel Direction (to / from)	Percentage of Total Traffic Generation	
	Bishop Drive Townhomes	Site Access Townhomes
East via Ardagh Road	70%	70%
West via Ardagh Road	-	30%
Outside Study Area	30%	-
TOTAL	100%	100%

It is noted that the traffic generated from the townhomes with direct access onto Bishop Drive will only utilize the Ardagh Road / Snowshoe Trail & Site Access intersection if travelling to the east via Ardagh Road.

Using the traffic distributions pattern noted above, the traffic assignment for the proposed development was calculated for the AM and PM peak hour and is illustrated in **Figure 6**.

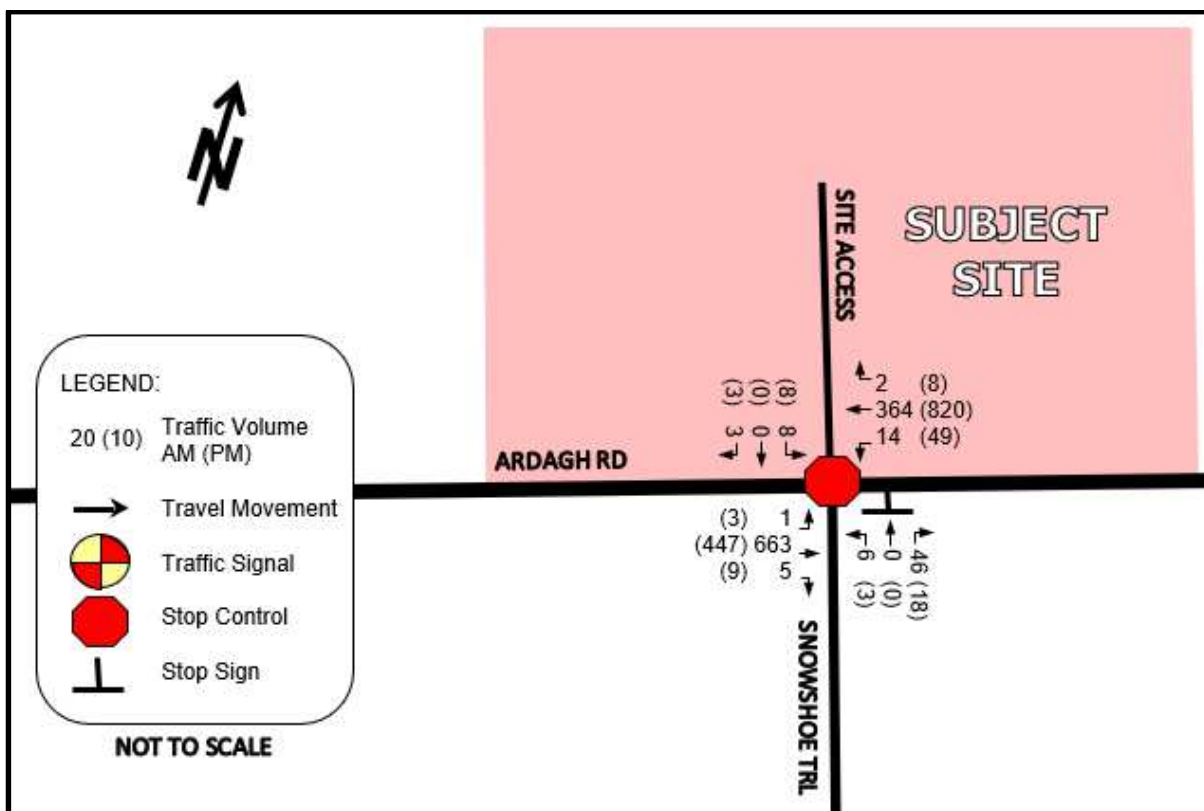
Figure 6 – Subject Site Traffic Assignment



3.3 Total Horizon Year Traffic Volumes with the Proposed Development

For the total (2031) horizon year traffic volumes, the proposed development traffic was added to the background (2031) traffic volumes. The resulting total (2031) horizon year traffic volume for the AM and PM peak hour are illustrated in **Figure 7**.

Figure 7 – Total (2031) Traffic Volumes



4 Intersection Operation with Proposed Development

4.1 Introduction

Traffic operations within the study area were evaluated using the horizon year traffic volumes with the existing road configuration and traffic control. The intersection performance was measured using the traffic analysis software, Synchro 10, a deterministic model that employs Highway Capacity Manual and Intersection Capacity Utilization methodologies for analyzing intersection operations. These procedures are accepted by provincial and municipal agencies throughout North America.

Synchro 10 enables the study area to be graphically defined in terms of streets and intersections, along with their geometric and traffic control characteristics. The user is able to evaluate both signalized and unsignalized intersections in relation to each other, thus not only providing level of service for the individual intersections, but also enabling an assessment of the impact the various intersections in a network have on each other in terms of spacing, traffic congestion, delay, and queuing.

Individual turning movements with a volume-to-capacity [V/C] ratio of 0.85 or greater are considered to be critical movements and have been highlighted in the LOS tables.

The intersection operations were also evaluated in terms of the LOS. LOS is a common measure of the quality of performance at an intersection and is defined in terms of vehicular delay. This delay includes deceleration delay, queue move-up time, stopped delay, and acceleration delay. LOS is expressed on a scale of A through F, where LOS A represents very little delay (i.e. less than 10 seconds per vehicle) and LOS F represents very high delay (i.e. greater than 50 seconds per vehicle for a stop sign controlled intersection and greater than 80 seconds per vehicle for a signalized intersection).

The LOS criteria for signalized and stop sign controlled intersections are shown in **Table 4**. A description of traffic performance characteristics is included for each LOS.

Table 4 – Level of Service Criteria for Intersections

LOS	LOS Description	Control Delay (seconds per vehicle)	
		Signalized Intersections	Stop Controlled Intersections
A	Very low delay; most vehicles do not stop (Excellent)	less than 10.0	less than 10.0
B	Higher delay; more vehicles stop (Very Good)	between 10.0 and 20.0	between 10.0 and 15.0
C	Higher level of congestion; number of vehicles stopping is significant, although many still pass through intersection without stopping (Good)	between 20.0 and 35.0	between 15.0 and 25.0
D	Congestion becomes noticeable; vehicles must sometimes wait through more than one red light; many vehicles stop (Satisfactory)	between 35.0 and 55.0	between 25.0 and 35.0
E	Vehicles must often wait through more than one red light; considered by many agencies to be the limit of acceptable delay	between 55.0 and 80.0	between 35.0 and 50.0
F	This level is considered to be unacceptable to most drivers; occurs when arrival flow rates exceed the capacity of the intersection (Unacceptable)	greater than 80.0	greater than 50.0

4.2 Total (2031) Intersection Operation

The results of the LOS analysis under total (2031) traffic volumes during the AM and PM peak hour can be found below in **Table 5**. The existing intersection geometry and traffic control has been utilized for this scenario. Detailed output of the Synchro analysis can be found in **Appendix E**.

Table 5 – Total (2031) LOS

Location (E-W Street / N-S Street)	Weekday AM Peak Hour			Weekday PM Peak Hour		
	V/C	Delay (s)	LOS	V/C	Delay (s)	LOS
Ardagh Road / Snowshoe Trail & Site Access	-	1.0	A	-	0.7	B
NB	0.13	14.8	B	0.04	12.6	B
SB	0.03	15.3	C	0.04	18.9	C

The results of the LOS analysis indicate that the Ardagh Road / Snowshoe Trail & Site Access intersection is operating within the typical design limits noted in Section 4.1.

An analysis was completed for right turn movements at the Ardagh Road / Snowshoe Trail & Site Access intersection. Based on the LOS analysis results, no right turn lanes are warranted at the Ardagh Road / Snowshoe Trail & Site Access intersection.

Based on the Ontario Traffic Manual Book 12 *Signal Justification*, traffic signals are not warranted at the Ardagh Road / Snowshoe Trail & Site Access intersection (results are provided in **Appendix F**).

No infrastructure improvements are recommended within the study area.

4.3 Site Access

The Site Access will operate efficiently as a full-movement access, with two-way stop control for the northbound and southbound movements. No lane improvements are recommended on Ardagh Road and Snowshoe Trail at the Site Access. A single northbound and two southbound lanes (one right turn lane and one through-left turn lane) at the Site Access driveway will provide the necessary capacity to service the proposed development.

4.4 Sight Distance Review

A review of the available sight distance for the proposed Site Access was completed as part of this analysis.

The sight distance east and west of the Site Access is greater than the minimum stopping sight distance requirements as identified in the Transportation Association of Canada *Design Guide for Canadian Roads* (2017) [TAC Guidelines] for a design speed of 60km/h (85 meters).

Consequently, there are no issues with the sight distance available for the proposed Site Access.

4.5 Construction Parking

Based on our correspondence with the design team for this project, the Developer is committed to the following arrangement for construction parking:

The parking of construction vehicles and vehicles owned by staff and subcontractors employed by the Developer will be formally directed (in writing where possible) to avoid parking on municipal streets surrounding the subject site and avoid parking within private parking lots surrounding the subject site, without consent from the owner of the parking lots.

5 Summary

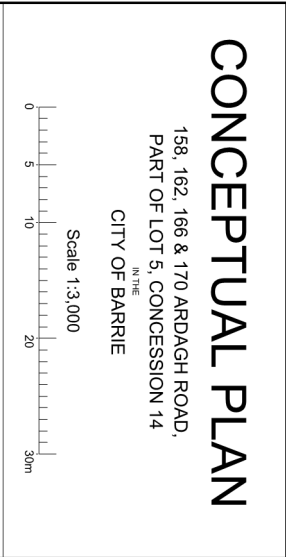
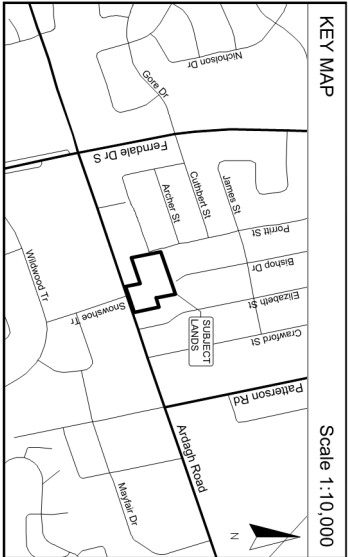
The Hedbern Development Corp. retained **JD Engineering** to prepare this traffic brief in support of the proposed development in the City of Barrie. The proposed Site Plan is shown in **Appendix A**. This chapter summarizes the conclusions and recommendations from the study.

The proposed development includes 58 residential townhouse units. Of the proposed 58 townhouse units, 27 units will have direct driveway access onto Bishop Drive and 31 units will utilize the Site Access.

1. The proposed development is expected to generate a total of 27 AM and 32 PM peak hour trips.
2. Weekday detailed turning movement counts were completed for the Ardagh Road / Snowshoe Trail intersection on Thursday, February 21st, 2019.

3. An estimate of the amount of traffic that would be generated by the Subject Site was prepared and assigned to the study area streets and intersections.
4. An intersection operation analysis was completed under total (2031) traffic volumes with the proposed development operational at the study area intersections. No geometric lane improvements or traffic signal improvements are recommended within the study area.
5. The Site Access will operate efficiently as a full-movement access, with two-way stop control for the northbound and southbound movements. A single northbound and two southbound lanes (one right turn lane and one through-left turn lane) at the Site Access driveway will provide the necessary capacity to convey the traffic volume generated by the proposed development.
6. The sight distance available for the proposed Site Access meets the minimum stopping sight distance requirements.
7. In summary, the proposed development will not cause any operational issues and will not add significant delay or congestion to the local roadway network.

Appendix A – Site Plan



LEGEND

SUBJECT LANDS (Area 1.63ha / 4.03ac) * SMALL LOT AREA EXCEEDS LOT COVERAGE (MAX.)

BLOCK/CLUSTER TOWNHOUSE +

DWELLING

BACK TO BACK TOWNHOUSE DWELLINGS

STREET TOWNHOUSE

DWELLINGS (Area: 0.91ha / 2.25ac)

BALCONIES

AMENITY AREA

ZONING TABLE - CONDO TOWNHOUSE (31 UNITS)

PROVISION	REQUIRED	PROVIDED
Lot Area	720 (min)	7,184.0m ² (1.78 ac)
Lot Frontage	21m (min)	102.2m
Setbacks		
Front Yard	7.0m (min)	3.0m
Interior Side Yard	1.8m (min)	1.8m
Exterior Side Yard	3.0m (min)	N/A
Rear Yard	7.0m (min)	4.5m
Landscaped Open Space	35% (min)	45.0%
Amenity Area	12m ² / unit (972.0m ²)	1,514.1m ²
Lot Coverage	35% (max)	35.0%
Gross Floor Area (max)	60% of lot area (max)	80.0%
Building Height	10.0m (max)	10.0m
Parking	1.5 / unit (47 spaces, with 1 A, 72 spaces (10 visitor, with 1 A, & 2 B SF spaces)	permitted
Tandem Parking	not permitted	permitted
Secondary Means of Access	7.0m	4.5m
Density	40.0 u/ha	43.1 u/ha
Landscaped Buffer Area	3.0m	3.0m

ZONING TABLE - STREET TOWNHOUSE (27 UNITS)

PROVISION	REQUIRED	PROVIDED
Lot Area (5.3.4.2a)	200.0m ² (min)	162.3m ²
Lot Frontage (5.3.4.2a)	6.0m (min)	6.0m
Setbacks		
Front Yard (5.3.4.2a)	4.5m (min)	6.0m
Interior Side Yard	1.8m (min)	1.8m
Exterior Side Yard (5.3.4.2a)	3.0m (min)	3.7m
Rear Yard	7.0m (min)	7.0m
Landscape Open Space	35% (min)	39.0%
Dwelling Unit Floor Area (min)	55.0m ² / dwelling unit + 10.0m ² / deck/terrace	>55.0m ² / dwelling unit + 10.0m ² / deck/terrace
Lot Coverage	45% (max)	49.0%
Gross Floor Area (max)	60%	88%
Building Height	10.0m (max)	10.0m

Source: Parcel fabric by County of Simcoe Interactive Mapping, 2019
Note: This drawing is for discussion purposes only.
The information shown is approximate and subject to change.

IPS INNOVATIVE PLANNING SOLUTIONS
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Date: August 21, 2019 Drawn By: AS
File: 14-529 Checked By: GB



CONCEPTUAL PLAN - 58 TOWNHOUSE UNITS

158, 162, 166 & 170 ARDAGH RD - CITY OF BARRIE

RESIDENTIAL CURRENT OP DESIGNATION R1(H95) & R3

CURRENT ZONING

SCHEDULE OF REVISIONS

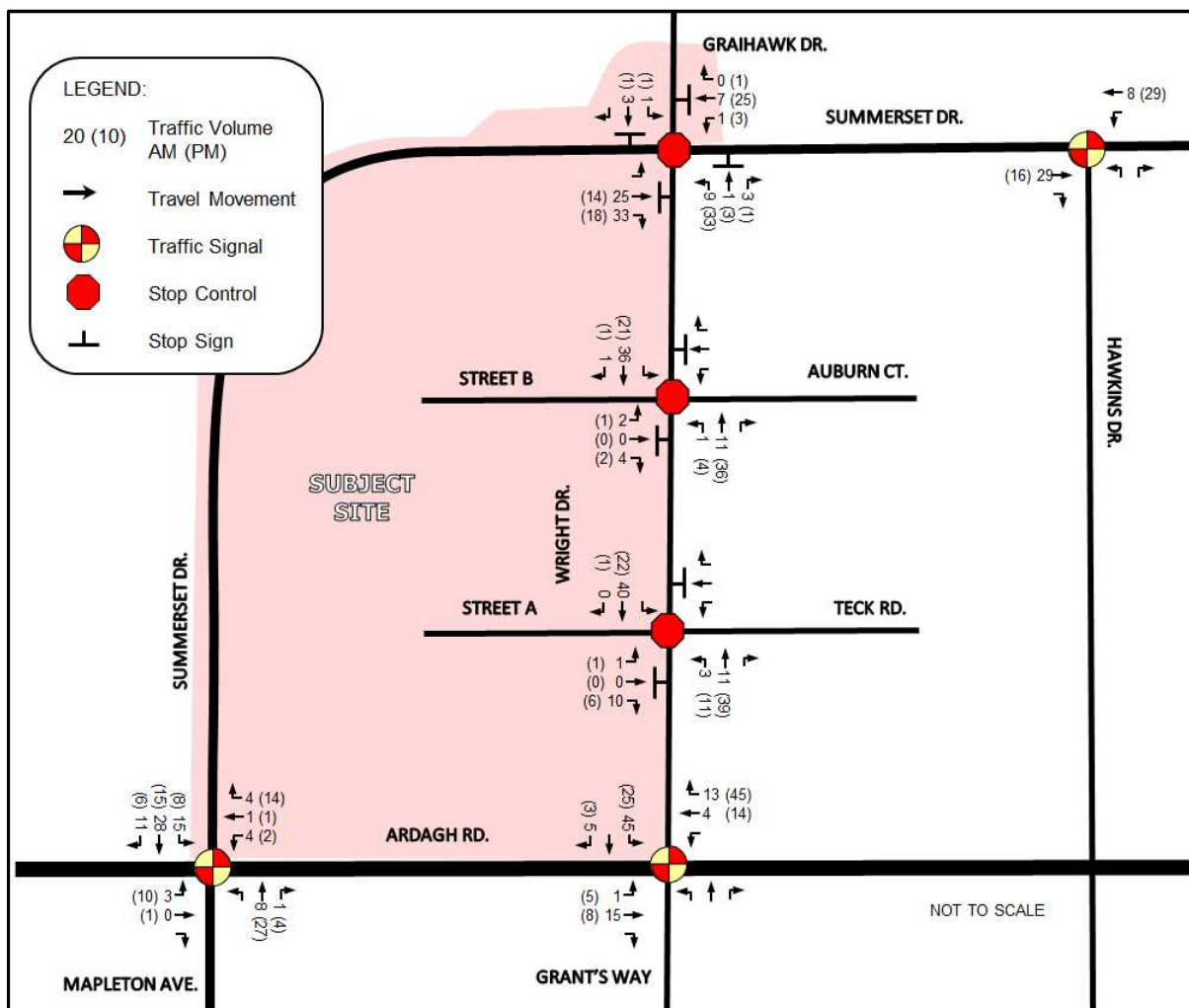
No.	Date	Description	By

Appendix B – Adjacent Development Excerpts

The proposed development includes 11 units with direct access onto Graihawk Drive. Consequently, we have assumed that 3.5% of the traffic generated by the proposed development will use Graihawk Drive. There are also five units with direct access onto Wright Drive and two units with direct access onto Summerset Drive, east of Wright Drive. For the purpose of our analysis, we have ignored the actual access location for these lots and grouped these lots in with the other lots that will use Street A, Street B and Summerset Drive (west of Wright Drive) to access the existing road network.

The assignment of the traffic generated by the proposed development is provided in **Figure 6**.

Figure 6 – Traffic Assignment for the Proposed Development



4.3 Total Horizon Year Traffic Volumes with the Proposed Development

For the total (2026) horizon year traffic volumes, the proposed development traffic was added to the background (2026) traffic volumes and the calculated redistribution of traffic to the Summerset Drive extension was applied. The resulting total (2026) horizon year total traffic volume for the AM and PM peak hour can be found in **Figure 7**.

Appendix C – Traffic Count Data

Ontario Traffic Inc.

Morning Peak Diagram

Specified Period

From: 7:00:00

To: 9:00:00

One Hour Peak

From: 7:30:00

To: 8:30:00

Municipality: Barrie

Site #: 1904400001

Intersection: Ardagh Rd & Snowshoe Trail

TFR File #: 16

Count date: 21-Feb-19

Weather conditions:

Person(s) who counted:

**** Non-Signalized Intersection ****

Major Road: Ardagh Rd runs W/E

East Leg Total: 789

East Entering: 283

East Peds: 0

Peds Cross: X

Heavys	Trucks	Cars	Totals
0	8	269	277

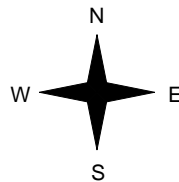


Ardagh Rd

Heavys	Trucks	Cars	Totals
0	9	461	470
0	0	4	4
0	9	465	



Snowshoe Trail



Cars	Trucks	Heavys	Totals
266	6	0	272
10	1	0	11
276	7	0	



Ardagh Rd

Cars	Trucks	Heavys	Totals
497	9	0	506

Peds Cross: X
West Peds: 0
West Entering: 474
West Leg Total: 751

Cars	14
Trucks	1
Heavys	0
Totals	15



Cars	3	36	39
Trucks	2	0	2
Heavys	0	0	0
Totals	5	36	

Peds Cross: X
South Peds: 1
South Entering: 41
South Leg Total: 56

Comments

Ontario Traffic Inc.

Afternoon Peak Diagram

Specified Period

From: 16:00:00

To: 18:00:00

One Hour Peak

From: 16:15:00

To: 17:15:00

Municipality: Barrie

Site #: 1904400001

Intersection: Ardagh Rd & Snowshoe Trail

TFR File #: 16

Count date: 21-Feb-19

Weather conditions:

Person(s) who counted:

**** Non-Signalized Intersection ****

Major Road: Ardagh Rd runs W/E

East Leg Total: 971

East Entering: 634

East Peds: 0

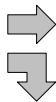
Peds Cross: X

Heavys	Trucks	Cars	Totals
0	5	592	597

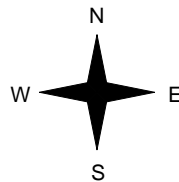


Ardagh Rd

Heavys	Trucks	Cars	Totals
0	3	320	323
0	0	7	7
0	3	327	



Snowshoe Trail



Cars	Trucks	Heavys	Totals
590	5	0	595
39	0	0	39
629	5	0	



Ardagh Rd

Cars	Trucks	Heavys	Totals
334	3	0	337

Peds Cross: X
West Peds: 1
West Entering: 330
West Leg Total: 927

Cars	46
Trucks	0
Heavys	0
Totals	46



Cars	2	14	16
Trucks	0	0	0
Heavys	0	0	0
Totals	2	14	

Peds Cross: X
South Peds: 6
South Entering: 16
South Leg Total: 62

Comments

Ontario Traffic Inc.

Total Count Diagram

Municipality: Barrie
Site #: 1904400001
Intersection: Ardagh Rd & Snowshoe Trail
TFR File #: 16
Count date: 21-Feb-19

Weather conditions:
Person(s) who counted:

**** Non-Signalized Intersection ****

Major Road: Ardagh Rd runs W/E

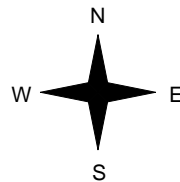
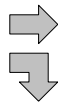
East Leg Total: 3950
 East Entering: 2099
 East Peds: 4
 Peds Cross: 8

Heavys	Trucks	Cars	Totals
0	31	1976	2007



Ardagh Rd

Heavys	Trucks	Cars	Totals
0	21	1693	1714
0	1	31	32
0	22	1724	



Snowshoe Trail

Cars	Trucks	Heavys	Totals
------	--------	--------	--------

1963	27	0	1990
108	1	0	109
2071	28	0	

Ardagh Rd

Cars	Trucks	Heavys	Totals
1830	21	0	1851

Peds Cross: 8
 West Peds: 2
 West Entering: 1746
 West Leg Total: 3753

Cars	139
Trucks	2
Heavys	0
Totals	141



Cars	13	137	150
Trucks	4	0	4
Heavys	0	0	0
Totals	17	137	

Peds Cross: 4
 South Peds: 17
 South Entering: 154
 South Leg Total: 295

Comments

Ontario Traffic Inc.

Traffic Count Summary

Intersection: Ardagh Rd & Snowshoe Trail

Count Date: 21-Feb-19

Municipality: Barrie

North Approach Totals						North/South Total Approaches	South Approach Totals					
Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds		Hour Ending	Includes Cars, Trucks, & Heavys				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0
8:00:00	0	0	0	0	0	41	8:00:00	8	0	33	41	0
9:00:00	0	0	0	0	0	42	9:00:00	3	0	39	42	1
16:00:00	0	0	0	0	0	0	16:00:00	0	0	0	0	0
17:00:00	0	0	0	0	0	18	17:00:00	0	0	18	18	4
18:00:00	0	0	0	0	0	22	18:00:00	4	0	18	22	7
19:00:00	0	0	0	0	0	31	19:00:00	2	0	29	31	5

Ontario Traffic Inc.

Count Date: 21-Feb-19	Site #: 1904400001
-----------------------	--------------------

[illegible]

Ontario Traffic Inc.

Count Date: 21-Feb-19 Site #: 1904400001

[illegible]

Ontario Traffic Inc.

Count Date: 21-Feb-19 Site #: 1904400001

[illegible]

[illegible]

Count Date: 21-Feb-19 **Site #:** 1904400001

[illegible]

Appendix D – Transportation Tomorrow Survey Excerpts



TTS Cross Tabulation

Cross Tabulation Query Form - Trip - 2016 v1.1

Filter Variables

Planning district of destl... X ▼

2006 GTA zone of hous... X ▼

(Optional) Table Attribute ▼

Group Attributes

Row Grouping

Column Grouping

Table Grouping

Grouping file: Choose File No file chosen

Filter Selection +

Start time of trip ▼ In ▼ 700-900

And ▼

Trip purpose of destination ▼ In ▼ w,r

And ▼

2006 GTA zone of household ▼ In ▼ 8522

Add Delete

Output

● Comma-delimited table ● Column format

Expansion Factor Or

Click to Select: Load

Load

Execute Query

Select All

Save As

Tue Mar 05 2019 00:43:48 GMT-0500 (Eastern Standard Time) - Run Time: 323ms

Cross Tabulation Query Form - Trip - 2016 v1.1

Row: Planning district of destination - pd_dest
Column: 2006 GTA zone of household - gta06_hhid

Filters:

(Start time of trip - start_time In 700-900

and

Trip purpose of destination - purp_dest In w,r

and

2006 GTA zone of household - gta06_hhid In 8522)

Trip 2016

Table:

,8522
PD 4 of Toronto,31
PD 8 of Toronto,29
PD 9 of Toronto,20
PD 11 of Toronto,76
Newmarket,126
Aurora,84
Markham,50
Vaughan,91
Brampton,8
Mississauga,34
Barrie,2133
Innisfil,239
Bradford-West Gwillimbury,45
New Tecumseth,70
Adjala-Tosorontio,37
Essa,31
Clearview,32
Springwater,108
Haliburton,20
Grey,7
Collingwood,35
Penetanguishene,9
Orillia,199



TTS Cross Tabulation

Cross Tabulation Query Form - Trip - 2016 v1.1

Filter Variables

2006 GTA zone of cest...
2006 GTA zone of hous...
(Optional) Table Attribute

Group Attributes

Row Grouping
Column Grouping
Table Grouping

Grouping file: Choose file
No file chosen

Filter Selection +

Start time of trip
In
700-900

And

Trip purpose of destination
In
w.r

And

2006 GTA zone of household
In
8522

And

Planning district of destination
In
81

Output

●

Comma-delimited table

●

Column format

Expansion Factor On

Click to Selected Load

Load

Execute Query

Select All

Save As

Tue Mar 05 2019 00:58:26 GMT-0500 (Eastern Standard Time) - Run Time: 2568ms

Cross Tabulation Query Form - Trip - 2016 v1.1




















Row: 2006 GTA zone of destination - gta06_dest
Column: 2006 b1A zone of household - gta06_hhld




















Filters:
(start_time of trip - start_time In 700-900
and
trip_purpose_of_destination - purp_dest In w,r
and
2006_gta_zone_of_household - gta06_hhld In 8522
and
Planning_district_of_destination - pd_dest In 81,)

Trip 2016
Table:

	, 8522
8501,296	
8502,17	
8504,96	
8508,18	
8509,156	
8510,139	
8513,18	
8514,55	
8518,42	
8520,7	
8521,267	
8522,163	
8523,37	
8524,404	
8526,61	
8527,270	
0520,00	

Appendix E – Synchro Analysis Output – Total Traffic Volumes

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1	663	5	14	364	2	6	0	46	8	0	3
Future Volume (Veh/h)	1	663	5	14	364	2	6	0	46	8	0	3
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	1	705	5	15	387	2	6	0	49	9	0	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												2
Median type		TW	LT		TW	LT						
Median storage (veh)		2			2							
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	389			710			1128	1128	708	1174	1130	388
vC1, stage 1 conf vol							710	710		418	418	
vC2, stage 2 conf vol							418	419		756	712	
vCu, unblocked vol	389			710			1128	1128	708	1174	1130	388
tC, single (s)	4.1			4.2			7.5	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)							6.5	5.5		6.1	5.5	
tF (s)	2.2			2.3			3.9	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			98	100	89	97	100	100
cM capacity (veh/h)	1170			858			324	382	438	311	373	660
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	1	710	15	389	55	12						
Volume Left	1	0	15	0	6	9						
Volume Right	0	5	0	2	49	3						
cSH	1170	1700	858	1700	422	415						
Volume to Capacity	0.00	0.42	0.02	0.23	0.13	0.03						
Queue Length 95th (m)	0.0	0.0	0.4	0.0	3.6	0.7						
Control Delay (s)	8.1	0.0	9.3	0.0	14.8	15.3						
Lane LOS	A		A		B	C						
Approach Delay (s)	0.0		0.3		14.8	15.3						
Approach LOS					B	C						
Intersection Summary												
Average Delay			1.0									
Intersection Capacity Utilization			50.5%		ICU Level of Service		A					
Analysis Period (min)			15									

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	3	447	9	49	820	8	3	0	18	8	0	3
Future Volume (Veh/h)	3	447	9	49	820	8	3	0	18	8	0	3
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	3	481	10	53	882	9	3	0	19	9	0	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												2
Median type		TWLT			TWLT							
Median storage veh		2			2							
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	891			491			1482	1489	486	1498	1490	886
vC1, stage 1 conf vol							492	492		992	992	
vC2, stage 2 conf vol							990	997		506	497	
vCu, unblocked vol	891			491			1482	1489	486	1498	1490	886
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)							6.1	5.5		6.1	5.5	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			95			99	100	97	96	100	99
cM capacity (veh/h)	761			1083			255	275	585	249	273	343
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	3	491	53	891	22	12						
Volume Left	3	0	53	0	3	9						
Volume Right	0	10	0	9	19	3						
cSH	761	1700	1083	1700	498	332						
Volume to Capacity	0.00	0.29	0.05	0.52	0.04	0.04						
Queue Length 95th (m)	0.1	0.0	1.2	0.0	1.1	0.9						
Control Delay (s)	9.8	0.0	8.5	0.0	12.6	18.9						
Lane LOS	A		A		B	C						
Approach Delay (s)	0.1		0.5		12.6	18.9						
Approach LOS					B	C						
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utilization			60.3%		ICU Level of Service		B					
Analysis Period (min)			15									

Appendix F – OTM Signal Justification Sheets

Justification No. 7 - 2031 Total Traffic (Critical Case)

Ardagh Rd / Snowshoe Trail & Site Access

Justification	Description		Compliance			Signal Warrant	Underground Provisions Warrant
			Sectional		Entire %		
			Rest. Flow	Numerical			
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	720	620	86%	12%	NO	NO
	B. Vehicle volume, along minor streets (average hour)	170	24	14%		NO	NO
2. Delay to cross traffic	A. Vehicle volume, major street (average hour)	720	590	82%	7%	NO	NO
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	75	6	8%		NO	NO