

**41 ESSA RD. & 259 INNISFIL ST, BARRIE**



**41 ESSA Rd. & 259 Innisfil St,  
Barrie**

Rail Safety Assessment Report

September 17, 2021

160560021

Prepared for:

Tonlu Holdings Ltd.

Prepared by:

Stantec Consulting Ltd

## 41 ESSA RD. & 259 INNISFIL ST, BARRIE


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## Table of Contents

<b>1.0</b>	<b>INTRODUCTION .....</b>	<b>1</b>
<b>2.0</b>	<b>SITE LOCATION AND DESCRIPTION .....</b>	<b>1</b>
<b>3.0</b>	<b>BACKGROUND INFORMATION .....</b>	<b>2</b>
<b>4.0</b>	<b>RAIL OPERATIONS .....</b>	<b>3</b>
4.1	CURRENT CONFIGURATION .....	3
4.2	FUTURE CONFIGURATION .....	3
<b>5.0</b>	<b>DEVELOPMENT PROPOSAL .....</b>	<b>3</b>
<b>6.0</b>	<b>HAZARDS IDENTIFICATION .....</b>	<b>3</b>
<b>7.0</b>	<b>ASSESSMENT CRITERIA AND METHODOLOGY .....</b>	<b>4</b>
7.1	APPLICABLE GUIDELINES: .....	4
7.1.1	The Federation of Canadian Municipalities and Railway Association of Canada Guidelines for New Development in Proximity to Railway Operations (May 2013)- "FCM-RAC" .....	4
7.2	DERAILMENT IMPACT ANALYSIS:.....	5
<b>8.0</b>	<b>MITIGATION DESIGN ASSESSMENT .....</b>	<b>5</b>
8.1	BUILDING 1 .....	6
8.1.1	Setback .....	6
8.1.2	Safety Barriers .....	7
8.2	BUILDING 2 .....	7
8.2.1	Setback .....	7
8.2.2	Safety Barriers .....	8
8.3	BUILDING 3 .....	8
8.3.1	Setback .....	8
8.3.2	Safety Barriers .....	8
8.4	BUILDING 4 .....	9
8.4.1	Setback .....	9
8.4.2	Safety Barriers .....	9
<b>9.0</b>	<b>RISK ANALYSIS.....</b>	<b>9</b>
9.1	HISTORIC RAIL OCCURRENCES.....	9
9.2	CRASH IMPACT – FREQUENCY & SEVERITY MATRIX .....	10
<b>10.0</b>	<b>RECOMMENDED MITIGATION MEASURES.....</b>	<b>11</b>
<b>11.0</b>	<b>CONSIDERATIONS DURING CONSTRUCTION.....</b>	<b>11</b>
<b>12.0</b>	<b>CONCLUSION .....</b>	<b>12</b>

**LIST OF TABLES**

Table 1 - Combined Setback to Residential Units- Building 2.....	7
Table 2 - Frequency/Severity Matrix.....	10

**LIST OF FIGURES**

Figure 1 Site Location (Key Plan).....	2
Figure 2 Combining Horizontal and Vertical Setback [Source: FCM/ RAC] .....	6

**LIST OF APPENDICES**

<b>APPENDIX A</b>	<b>ARCHITECTURAL PLANS.....</b>	<b>I</b>
	<b>TOPOGRAPHICAL SURVEY PLAN.....</b>	<b>I</b>
<b>APPENDIX B</b>	<b>STRUCTURAL CALCULATION FOR 41 ESSA RD &amp; 259 INNISFIL ST. ....</b>	<b>II</b>

## **1.0 INTRODUCTION**

Stantec Consulting Ltd. has been retained by Tonlu Holdings Ltd. to complete a Rail Safety Assessment for the proposed development at 41 Essa Rd & 259 Innisfil St. in the City of Barrie.

The purpose of this study is to assess site specific hazards and mitigation measures relating to noise, vibration, trespass, and crash potential.

Architectural drawings completed by architectureunfolded dated Sep 9 2021, shows a mixed-use development consisting of four high-rise buildings which will provide for approximately 1276 residential units, low scale retail spaces and amenity areas. The proposed development will also incorporate 1313 parking spaces (to be situated within a 6-level above ground parking garage). A reduced copy of the Architectural Drawing set has been included in Appendix A.

## **2.0 SITE LOCATION AND DESCRIPTION**

The subject property land is located on the northeast side of Innisfil St. and Essa Rd in the City of Barrie adjacent to Barrie Collingwood Railway's (BCRY) Meaford Subdivision, approximate mile 0.3, as shown in Figure 1. This land, with an area of approximately 4.42 acres, is currently occupied with few low-rise commercial buildings. The triangular shaped site is bound by Innisfil St. to the west, Essa Rd to the east, and the BCRY on the north. The current site conditions will be demolished in the near future to give way to the proposed development which includes 4 high-rise towers as follows:

- Building 1: 29 Storey Tower
- Building 2: 35 Storey Tower
- Building 3: 37 Storey Tower
- Building 4: 20 Storey Tower

The Topographical Survey Plan for the site, included in Appendix A, indicates that the existing elevations of the proposed development range between 226.77 m and 229.92.



## BACKGROUND INFORMATION



Figure 1 Site Location (Key Plan)

## 3.0 BACKGROUND INFORMATION

The following documentation were consulted in the preparation of this Rail Safety Assessment:

- Federation of Canadian Municipalities (FCM) and Railway Association of Canada (RAC)'s Guidelines for New Development in Proximity to Railway Operations (May 2013)
- Canadian Railway Safety Act (last amended June 22 2017)
- AECOM's Submission Guidelines for Crash Walls (Revised July 29 2014)



## 4.0 RAIL OPERATIONS

The existing rail operations in close proximity to the north property limit of the proposed development site are part of Barrie Collingwood Railway's Meaford Subdivision, approximate mile 0.3. Stantec reached out to the Railway to determine the track classification. The Railway considers this subdivision as a Principal Branch Line.

### 4.1 CURRENT CONFIGURATION

Listed below are the rail operations characteristics associated with this location:

- 1 Branch Line track (BCRY)
- Maximum train speed 10 mph (approx. 16 Km/h)
- 0.57 trains per day (as per the Railway)

### 4.2 FUTURE CONFIGURATION

A discussion regarding the future expansion plan took place between Stantec and the Railway.

Railway does not anticipate an expansion or changes to services through this area in the near future. Therefore, this report is based on current configuration and maximum train speed.

Figure 1.0 depicts the location of Barrie Collingwood Rail R.O.W. relative to the 41 Essa Rd & 259 Innisfil St. development site.

## 5.0 DEVELOPMENT PROPOSAL

The drawings provided, dated Sep 9, 2021, by architectureunfolded indicate four high-rise: 20, 29, 35 & 37 Storey, mixed used towers, are proposed. A reduced copy of the Architectural Drawing set has been included in Appendix A.

## 6.0 HAZARDS IDENTIFICATION

Several factors or concerns should be given consideration when new developments are erected within proximity to railway operations. These include crash safety, trespass, noise pollution, vibration, and stormwater management. These will form the list of criteria for the mitigation measures to be discussed in this report.

Amongst the above listed factors, the two (2) factors the general public most commonly associates with proximity to rail operations are noise pollution and vibration. It can be argued that noise pollution and



vibration are built-in characteristics or properties of rail operations. Generally, noise pollution is generated as a result of wheel – rail interactions, locomotive engines, and whistles. Whereas vibration is produced as the wheels roll on tracks. While it may not be entirely possible to eliminate the impact of these two (2) issues onto neighboring properties, they can be mitigated.

Trespassing and crashes are by far the two (2) most severe hazards since they can lead to the greatest physical impact and could result in serious injury including instant loss of life.

## 7.0 ASSESSMENT CRITERIA AND METHODOLOGY

The methodology to be employed in this report will be based on comparing or assessing the technical/ engineering criteria from the existing guidelines noted in Section 3.0 of this report, which will form the basis for the selection and discussion of the required mitigation measures for the proposed development site. The most relevant set(s) of criteria will then be selected and the feasibility for the implementation of the associated mitigation measures will also be assessed.

### 7.1 APPLICABLE GUIDELINES:

#### 7.1.1 The Federation of Canadian Municipalities and Railway Association of Canada Guidelines for New Development in Proximity to Railway Operations (May 2013)- “FCM-RAC”

- The FCM/RAC Guidelines recommend the use of “standard mitigation measures” which include a 15m setback for Branch lines with an earthen berm constructed to a height of 2.0m above grade for Branch lines with side slopes not steeper than 2.5 to 1
- Marginal reductions in the recommended setback of up to 5m may be achieved through a reciprocal increase in the height of the berm
- Where there are elevation differences between the railway and a subject development property, appropriate variations in the minimum setback should be determined
- No berm is required where the railway line is in a cut of equivalent depth of the standard berm height
- If applicable to the site conditions, in lieu of the recommended berm, a ditch or valley between the railway and the subject new development property that is generally equivalent to or greater than the inverse of the berm could be considered
- Notwithstanding the above, the FCM/RAC Guidelines were also written to offer alternatives where standard mitigation measures are not viable
- At a minimum all new residential developments in proximity to railway corridors must include a 1.83m high chain link fence along the entire property line to provide protection against trespassing



## 7.2 DERAILMENT IMPACT ANALYSIS:

In addition to the above, the following guidelines detail train derailment as they relate to protection requirements for impact.

- A separation distance of 15 m (minimum) taken horizontally from the rail corridor property line to face of building and a 2.0 m high earthen crash safety berm (sloped 2.5:1) is required (FCM/RAC)
- Minimal reductions in the recommended setback, of up to 5 metres can be achieved by a reciprocal increase in the height of the berm (FCM/RAC)
- Alternate berm designs (e.g. crash walls) may be considered for spatially constrained locations. The intent is to provide a level of energy absorption equivalent to that of a standard berm (FCM/RAC)

## 8.0 MITIGATION DESIGN ASSESSMENT

Stantec's assessment of the proposed conditions is based on a review of the latest Architectural Drawings which includes 4 buildings. These four buildings will be assessed individually. It has been assumed that structural elements for the residential tower are located in the parking podium levels at the limit of the tower envelope and that these will require protection.

FCM/RAC guidelines stipulate that the minimum building setback distance is measured from the mutual property line to the building face and that the mutual property line is always the reference point for building setbacks, regardless of area conditions. Therefore, all setback calculation in this report is measured from the Railway R.O.W.

FCM/RAC guidelines further measure the setback from the mutual property line and consider the combined horizontal and vertical separation in calculating the setback as shown in Figure 2 which was extracted from the FCM/RAC guidelines.



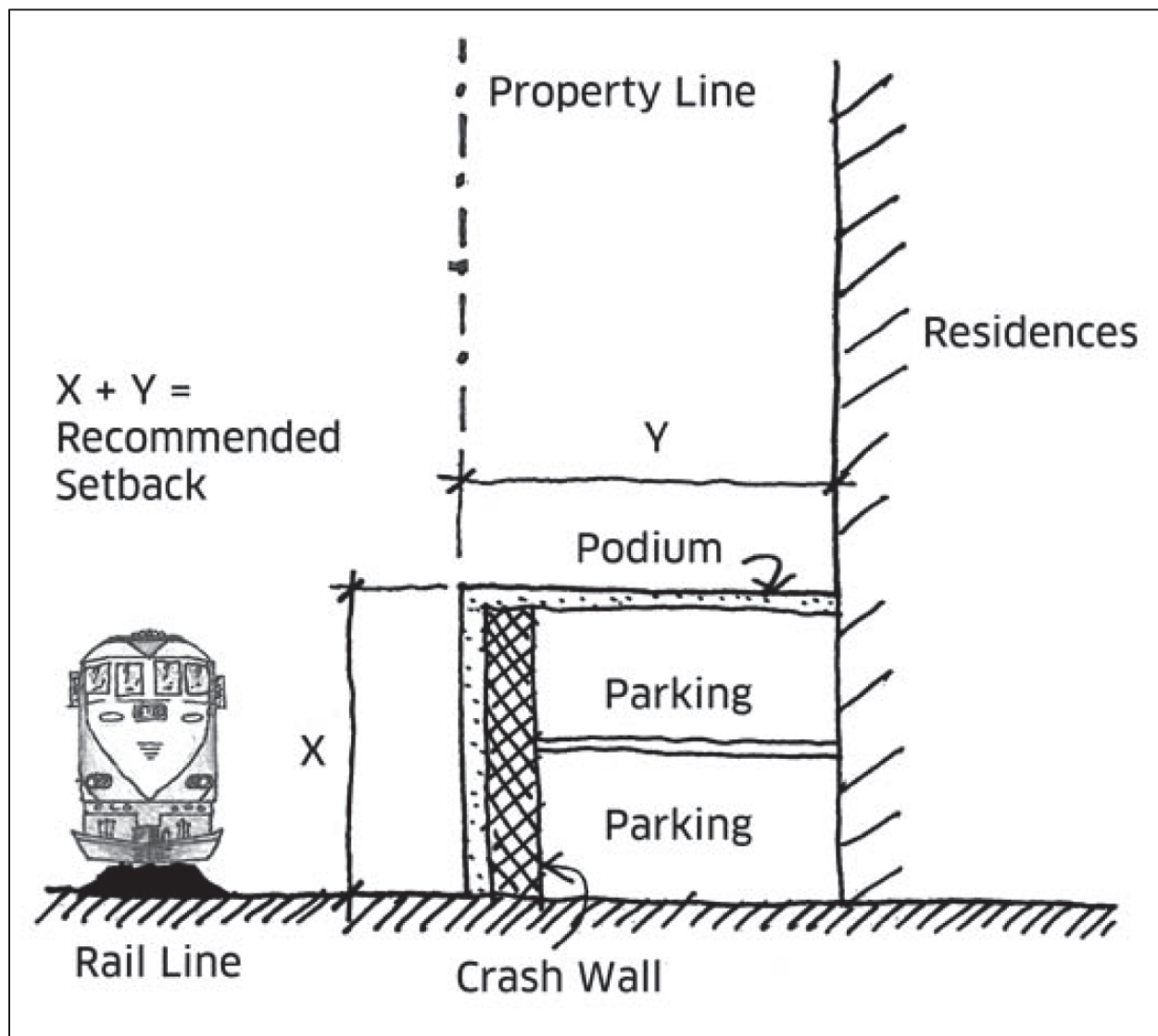


Figure 2 Combining Horizontal and Vertical Setback [Source: FCM/ RAC]

## 8.1 BUILDING 1

### 8.1.1 Setback

This proposed 29 Storey building has 6 levels of aboveground podium and incorporates small scale retail & residential units on the ground floor. The latest architectural drawings show that the distance to the nearest structures supporting the residential units from the railway R.O.W approximately 83m. This is well beyond the minimum 15m setback requirement by FCM/ RAC.





### 8.1.2 Safety Barriers

FCM/ RAC guidelines call for a safety barrier in the form of an earthen berm where full setbacks are provided. Safety barriers reduce the risks associated with railway incidents/ accidents by intercepting or deflecting derailed cars as well minimize the lateral spread or width in which the rail can travel. Viable and site-specific alternatives can be proposed such as crash berms, crash walls, reverse ditches etc. in lieu of earthen berm.

Given the low train speed in the subject area, structural calculations were completed by 'Jablonsky, Ast and Partners' to determine the furthest distance a train could potentially travel in the incident of a train derailment at a speed of 10mph. It was established that a derailed train has the potential to travel as far as 1.4m from track centreline, refer to Structural Report in Appendix B. On this basis, no crash wall or safety barrier is required for this building.

## 8.2 BUILDING 2

### 8.2.1 Setback

This proposed 35 Storey building has 6 levels of aboveground podium with residential units starting from the ground floor. The latest architectural drawings show that the distance to the nearest structures supporting the residential units from the railway R.O.W is approximately 15m.

**Table 1 - Combined Setback to Residential Units- Building 2**

Floor	Horizontal Setback to Residential Units (m)	Vertical Separation above Property Line	Combined Setback (m) (to Residential Units)
1	15	0.6	15.6
2	15	3.6	18.6
3	15	6.6	21.6
4	15	9.6	24.6
5	15	12.6	27.6
6	15	15.6	30.6
7	15	18.6	33.6

As noted in the table above, the lowest combined vertical and horizontal setback to the residential units is 15.6m which occurs on the ground floor. This meets the minimum 15m setback from a Branch Line as per FCM/ RAC.

The lower six levels contain low occupancy locker units, at a setback of approximately 12m from the railway R.O.W. There is no minimum setback requirement for low occupancy uses such as lockers and are allowed within the minimum required setback.



### **8.2.2 Safety Barriers**

FCM/ RAC guidelines call for a safety barrier in the form of an earthen berm where full setbacks are provided. Safety barriers reduce the risks associated with railway incidents/ accidents by intercepting or deflecting derailed cars as well minimize the lateral spread or width in which the rail can travel. Viable and site-specific alternatives can be proposed such as crash berms, crash walls, reverse ditches etc. in lieu of earthen berm.

Given the low train speed in the subject area, structural calculations were completed by 'Jablonsky, Ast and Partners' to determine the furthest distance a train could potentially travel in the incident of a train derailment at a speed of 10mph. It was established that a derailed train has the potential to travel as far as 1.4m from track centreline, refer to Structural Report in Appendix B. On this basis, no crash wall or safety barrier is required.

## **8.3 BUILDING 3**

### **8.3.1 Setback**

This proposed 37 Storey tower has 6 levels of aboveground parking podium with garbage room, lobby, bike storage on the ground floor and locker units from the second to 6<sup>th</sup> floor. The latest architectural drawings show that the distance to the nearest structures supporting the residential units from the railway R.O.W is approximately 15m with residential units starting from the 7<sup>th</sup> floor. The lowest combined vertical and horizontal setback to the residential units is 33m which occurs on the 7<sup>th</sup> floor. This exceeds the minimum required by the FCM/RAC guidelines.

As indicated above, the tower has six levels of aboveground parking podium with low occupancy usage. There is no minimum setback requirement for low occupancy uses, however, the setback requirements for the structural elements supporting the 37-storey residential tower are considered the same as the residential units (15m). The lowest setback to the structural element supporting the residential units is 15m, which meets the minimum 15m setback from a Branch Line as per FCM/ RAC.

### **8.3.2 Safety Barriers**

FCM/ RAC guidelines call for a safety barrier in the form of an earthen berm where full setbacks are provided. Safety barriers reduce the risks associated with railway incidents/ accidents by intercepting or deflecting derailed cars as well minimize the lateral spread or width in which the rail can travel. Viable and site-specific alternatives can be proposed such as crash berms, crash walls, reverse ditches etc. in lieu of earthen berm.

Given the low train speed in the subject area, structural calculations were completed by 'Jablonsky, Ast and Partners' to determine the furthest distance a train could potentially travel in the incident of a train derailment at a speed of 10mph. It was established that a derailed train has the potential to travel as far



Risk Analysis

as 1.4m from track centreline, refer to Structural Report in Appendix B. On this basis, no crash wall or safety barrier is required.

## **8.4 BUILDING 4**

### **8.4.1 Setback**

This proposed 20 Storey tower has 6 levels of aboveground parking podium with low scale retail and indoor amenities on the ground floor. The latest architectural drawings show that the distance to the nearest structures supporting the residential units from the railway R.O.W is approximately 12.3m with residential units starting from the 7<sup>th</sup> floor. The lowest combined vertical and horizontal setback to the residential units is 31.5m which occurs on the 7<sup>th</sup> floor. This exceeds the minimum required by the FCM/RAC guidelines.

The tower has six levels of aboveground parking podium with low scale retail and amenities on the ground floor. There is no minimum setback requirement for low occupancy uses such as parking podiums, however, the setback requirements for the structural elements supporting the 20-storey residential tower are considered the same as the residential units (15m). The lowest setback to the structural element supporting the residential units is 13.5m, which is less than the requirement. Due to this setback deficit of supporting the structural elements, a safety barrier in the form of crash wall might be required. However, the low train speed could negate this requirement, structural calculations were completed to determine whether further crash protection is required, described in the section below.

### **8.4.2 Safety Barriers**

Given the low train speed in the subject area, structural calculations were completed by 'Jablonsky, Ast and Partners' to determine the furthest distance a train could potentially travel in the incident of a train derailment at a speed of 10mph. It was established that a derailed train has the potential to travel as far as 1.4m from track centreline, refer to Structural Report in Appendix B. On this basis, no crash wall or safety barrier is required.

## **9.0 RISK ANALYSIS**

### **9.1 HISTORIC RAIL OCCURRENCES**

Dataset on Transportation Safety Board (TSB) records and publishes data from Rail Occurrence Database System (RODs) on reportable accidents and incidents since January 1983. According to this dataset there has not been a TSB reportable accident or injury reported within ½ km of the subject property on the Meaford Subdivision since 1983.



## Risk Analysis

## 9.2 CRASH IMPACT – FREQUENCY & SEVERITY MATRIX

A 5x5 standard matrix of frequency vs. severity has been constructed based on the following criteria to assess the potential impact of a crash upon completion of the proposed development. The frequency/severity matrix provided is intended to present only the anticipated level of impact on life safety in the unlikely event of a derailment.

The following definitions were used to access the frequency and severity for Table 3.

### Frequency:

- Minimal (1) - First aid or minor medical treatment
- Marginal (2) - Reportable injury/occupational illness, lost workday
- Serious (3) - Serious injury, multiple injuries
- Critical (4) - Permanent partial disability
- Catastrophic (5) - Death or permanent total disability

### Probability:

- Improbable (1) - So unlikely it can be assumed that an occurrence may not be experienced in a lifetime.
- Remote (2) - Unlikely, but possible to occur in the lifetime of a person or item.
- Occasional (3) - Likely to occur sometime in the life of a person or item.
- Probable (4) - Will occur several times in the life of a person or item.
- Frequent (5) - Likely to occur frequently in the life of a person or item

**Table 2 - Frequency/Severity Matrix**

Frequency	Frequent (5)	-	-	-	-	-
	Probable (4)	-	-	-	-	-
	Occasional (3)	-	-	-	-	-
	Remote (2)	2	-	-	-	-
	Improbable (1)	-	-	-	-	-
		Minimal (1)	Marginal (2)	Serious (3)	Critical (4)	Catastrophic (5)



Recommended Mitigation Measures

<b>Legend:</b> 01-03 Very Low; 04-06 Low; 07-11 Medium; 12-18 High; 19-25 Very High	<b>Severity</b>
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Considering the low train speed and light train traffic, it can be anticipated that the severity of impact in the event of a train derailment would be minimal within the development lands, however, the rating above accounts for the overall severity of the potential derailment which includes the impacts within the corridor (trains, track infrastructure, rail passengers/staff).

## 10.0 RECOMMENDED MITIGATION MEASURES

From this Rail Safety Assessment study, it is our opinion that the proposed development is compliant with FCM/ RAC guidelines for development adjacent to rail corridor.

### General Site

- Entire site meets the recommended setback as per FCM/ RAC Guidelines. No supplementary safety barrier (mitigation measure against crash protection) is required due to the low train speed as established in the structural report.
- As per FCM/ RAC guidelines, at a minimum all new residential developments in proximity to railway corridors must include a 1.83m high chain link fence along the entire property line to provide protection against trespassing. Therefore, a 1.83m high trespass mitigation fence should be installed along the Railway R.O.W. to create a barrier between the municipal ROW and the corridor providing added protection against trespassing onto the existing railway corridor by way of traversing across the new proposed roadway from the development property. Party accountable for this installation should be determined.
- Considerations for noise pollution should be assessed by a qualified noise consultant.
- Considerations for vibration during and post construction are to be assessed by a qualified vibration consultant.
- It is expected that Drainage and Stormwater Management programs for the proposed development conditions within the site will need to be based on appropriate standards. Drainage and Stormwater management matters are to be further investigated and addressed by the project's civil engineering consultant.

## 11.0 CONSIDERATIONS DURING CONSTRUCTION

The following matters should be given consideration during construction:



## CONCLUSION

- Crane swing agreement
- Impact of railway loading onto shoring
- Trespass
- Railway personnel for flagging
- Vibration to and from the railway corridor during excavation and construction
- Erosion and Sediment Control

## 12.0 CONCLUSION

Based on the discussion in this report, the proposed measures put forth can be safely implemented on-site to adequately address crash, trespass, noise pollution, vibration, and drainage considerations.

We trust that the above report meets your requirements. Please contact us should you have any questions.

Regards,

**Stantec Consulting Ltd.**



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A handwritten signature in black ink, likely belonging to Jeff Kingston.

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## Appendix A ARCHITECTURAL PLANS

### TOPOGRAPHICAL SURVEY PLAN









notes:

revisions:

1. 2BA submission  
submissions:

architectural team:

project:  
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ground floor  
plan

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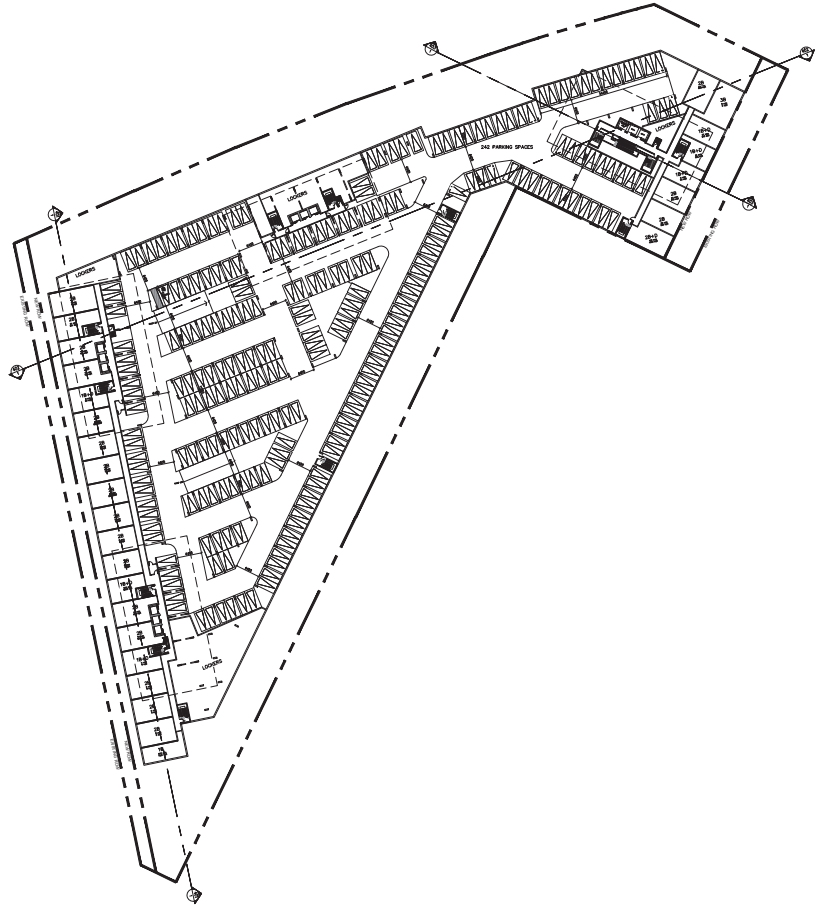
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notes:

revisions:

1. IBA submission  
submissions: 07-09-21  
date

architectural team:  
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frab, dachauer

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Architect:  
mimo, aischel  
frab, dachauer  
Architect:  
mimo, aischel  
frab, dachauer  
Architect:  
mimo, aischel  
frab, dachauer

project:  
239 Ivinskii Street  
Moscow

6th floor  
plan

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mimo - id

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project:  
drawn by:  
drawing number:

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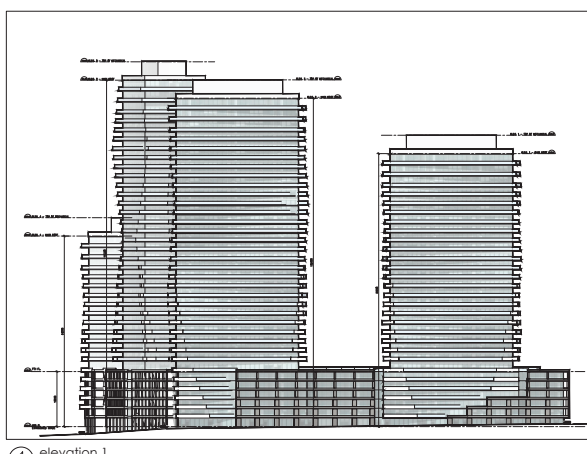
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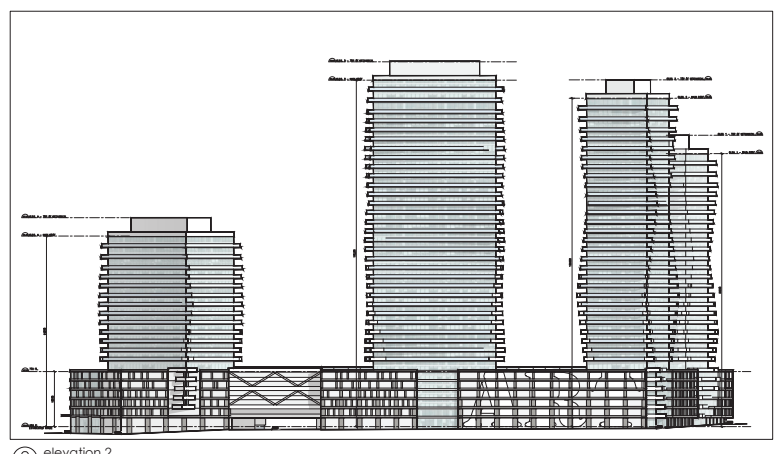
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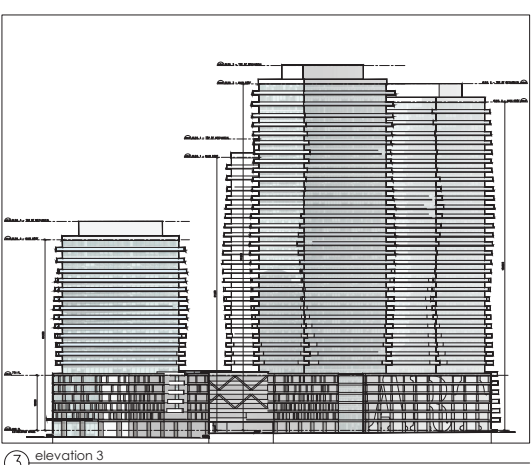
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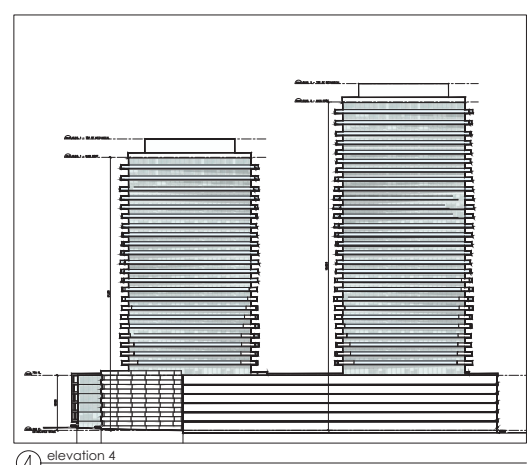
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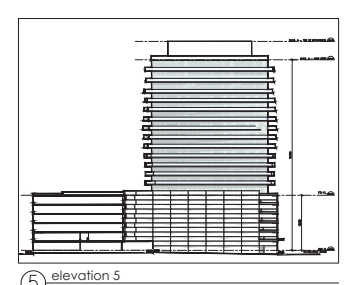
② elevation 2



③ elevation 3



④ elevation 4



⑤ elevation 5

1 BA submission  
submissions: 07-09-21  
date

architectural team :  
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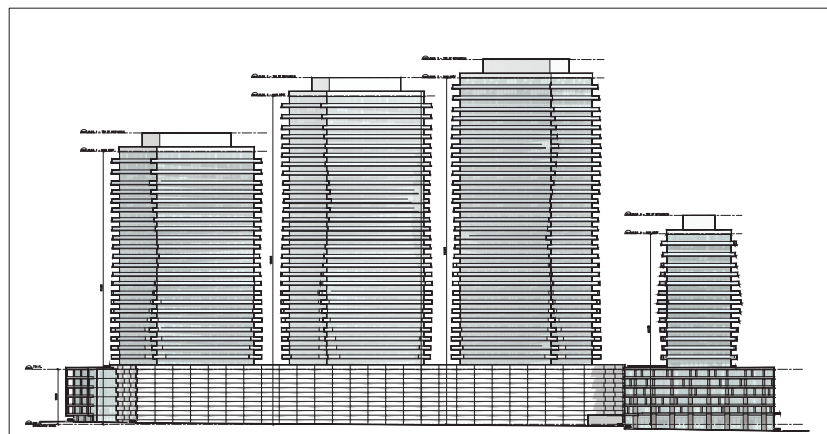
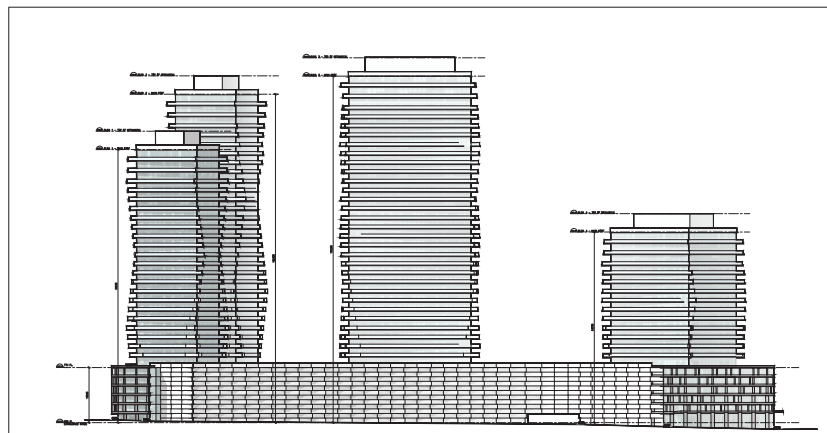
project:  
229 Inverell Street  
Sydney

elevation

2020  
1:500  
21-04  
mz - kd

scale:  
year:  
project:  
drawn by:  
drawing number:





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The design of these plans and specifications grants generally a license to use the drawing for the purpose of the design in a print or a pure online presentation of the design. The architect hereby grants the user a license to use the drawing for the purpose of the design in a print or a pure online presentation of the design.

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1 ZBA submission  
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**structure**  
**Stationary AZ and Pathway International**  
 401 Sheppard Avenue East, Suite 100, Scarborough, Ontario  
 M1S 1T5  
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 Fax: 416-291-1112  
 E-mail: [info@ipsconsulting.com](mailto:info@ipsconsulting.com)  
 Web: [www.ipsconsulting.com](http://www.ipsconsulting.com)

project:  
259 Innisfil Street

elevation

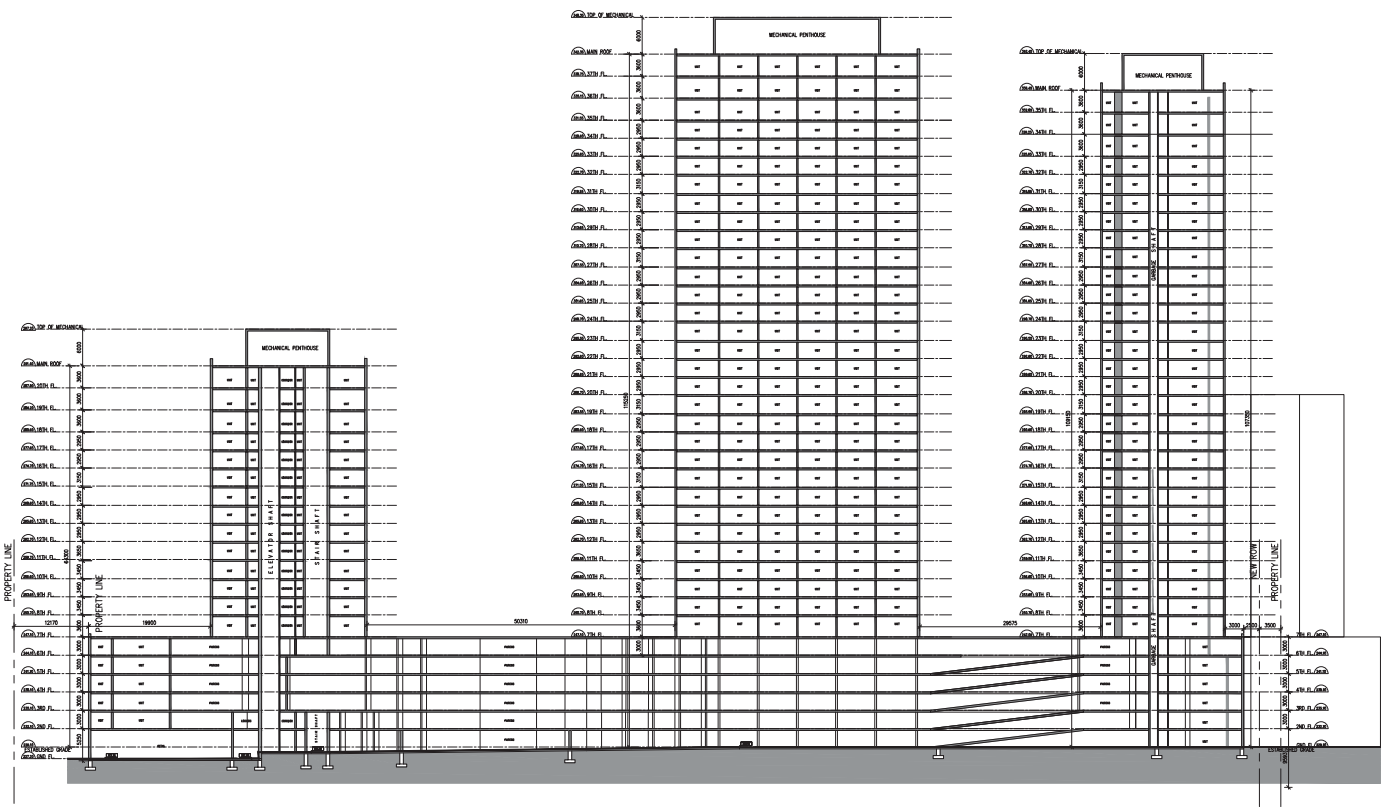
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notes:

revisions:

1. IBA submission submissions: 07-09-21 date

architectural team:

project:

259 Inverness Street

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date

A503



Street View Looking West



View Looking South East



Aerial View Looking North

Notes:  
1. The architectural rendering is a conceptual illustration of the proposed development and is not intended to represent a final design or construction. The rendering is for informational purposes only and should not be used for any other purpose.  
2. The rendering is based on the information provided by the client and is subject to change without notice. The rendering is not a guarantee of any kind and should not be relied upon for any purpose.  
3. The rendering is the property of the architect and should not be reproduced or distributed without the architect's written consent.  
4. The rendering is not a contract and should not be used as such. The rendering is a representation of the architect's design and is not a guarantee of any kind.  
5. The rendering is not a substitute for a professional engineering or architectural opinion. The rendering is a representation of the architect's design and is not a guarantee of any kind.  
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notes:

revisions:

1. IBA submission  
submissions:

architectural team:  
MBA, Inc.  
MBA, Inc.

designing:  
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project:  
229 Irving Street

renderings

2020

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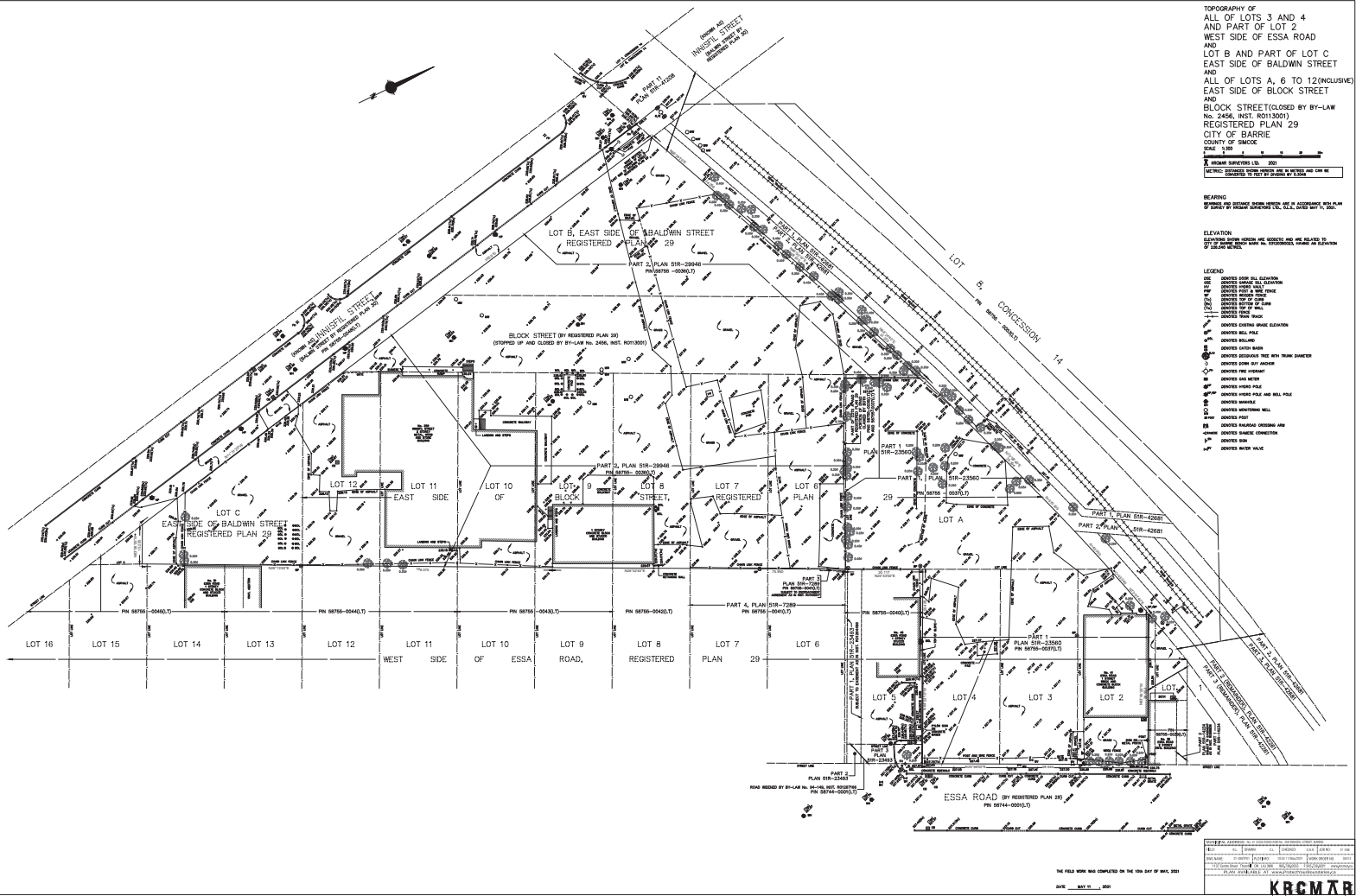
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## **Appendix B    STRUCTURAL CALCULATION FOR 41 ESSA RD & 259 INNISFIL ST.**





**JABLONSKY, AST AND PARTNERS**  
Consulting Engineers

400 - 3 Concorde Gate  
Toronto, ON M3C 3N7  
Telephone (416) 447-7405  
Fax (416) 447-2771  
www.astint.on.ca  
Email jap@astint.on.ca

August 11, 2021

Tonlu Holdings Limited  
111 Strada Drive  
Vaughan, ON L4L 5V9

Attn: Ms. Isabel Bercasio

Re: 259 Innisfil St., Barrie, ON  
Structural Train Derailment Review  
Our Project No. 21113

Dear Ms. Bercasio,

As per your recent request, we are providing the following design brief regarding the structural impact of a train derailment, on the site of 259 Innisfil St., adjacent to the Barrie line. The proposed development consists of an above grade, reinforced cast-in-place concrete structure with a 6-storey podium and 4 towers which are proposed to be 29, 35, 37 and 20 stories. The development is located adjacent to the Barrie Rail Line.

The approach typically taken to design a crash barrier is to equate the momentum of the moving body to the work done or energy absorbed in reducing the momentum of that moving body to zero. Current guidelines produced by AECOM (last amended in 2014) provide information with respect to train mass, direction and impact and train velocity. As well, recommendations are made with respect to how much energy is absorbed by the plastic deformation of the locomotives and rail cars. This site is unique in that the rail speeds are quite low and so the potential impact of a derailment is significantly reduced. Based on the track speeds of this line, we have chosen to use Method Two in which the following four cases are to be considered:

1. Freight Train Glancing Blow: nine cars weighing 143 tons (129,700 kg) each, impacting the wall at an angle,  $\theta_G$ . The angle of impact will be a function of track curvature, and for tangent track may be taken as 3.5 degrees.
2. Freight Train Single Car Impact: a single car weighing 143 tons (129,700kg) impacting the wall as it undergoes rotation about its center. The angle of rotation at impact is  $\theta_F$ .
3. Passenger Train Glancing Blow: eight cars weighing 74 tons (67,120kg) each impacting the wall at an angle,  $\theta_G$ . The angle of impact will be a function of track curvature and for a tangent track may be taken as 3.5 degrees.
4. Passenger Train Single Car Impact: a single car weighing 74 tons (67,120kg) impacting the wall as it undergoes rotation about its center. The angle of rotation at impact is  $\theta_F$ .



Based on information provided to us, the track design speed for freight trains is 10 mph (4.47m/s) and there is no passenger traffic. Based on these speeds, we can report the following (see enclosed diagram and calculations):

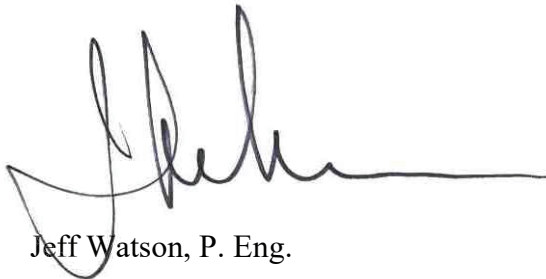
1. Based on the deceleration parameters included in the guidelines, beyond 1.4m from the centerline of track, the train will have reached zero velocity and therefore would impose no load.
2. Based on the guidelines, this load case does not need to be considered where the distance from the centerline of track is greater than 8.5m.
3. As there is no passenger traffic, this load case does not apply.
4. As there is no passenger traffic, this load case does not apply.

As the subject site is greater 8.5m from the centerline of track, any crash barrier required would not experience any loading associated with a potential derailment and would be of no benefit structurally.

We trust the foregoing will be sufficient, however, should you have any further questions, please do not hesitate to contact our office.

Yours very truly,

JABLONSKY, AST AND PARTNERS  
CONSULTING ENGINEERS



Jeff Watson, P. Eng.



Enclosure

cc: Gordon Tattle, JAP



